

PROJECT STRUCTURAL NOTES (KLAMATH FALLS, OREGON)

GENERAL INFORMATION:

1. STRUCTURAL DRAWINGS ARE A PORTION OF THE CONTRACT DOCUMENTS AND ARE INTENDED TO BE USED WITH ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING THE REQUIREMENTS FROM THESE DRAWINGS INTO THEIR SHOP DRAWINGS AND WORK. THESE GENERAL NOTES SUPPLEMENT THE PROJECT SPECIFICATIONS. REFER TO THE PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS. NOTES AND DETAILS OF CONSTRUCTION NOT FULLY SHOWN SHALL TAKE PRECEDENCE OVER THE GENERAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE GIVEN, CONSTRUCTION SHALL BE AS SHOWN FOR SIMILAR WORK.
2. THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES.
3. ALL EXISTING CONDITIONS, DIMENSIONS AND ELEVATIONS SHALL BE FIELD VERIFIED. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT OF ANY SIGNIFICANT DISCREPANCIES FROM CONDITIONS SHOWN ON THE DRAWINGS.
4. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION MEANS AND METHODS. RESPONSIBILITY SHALL INCLUDE BUT NOT LIMITED TO DEMOLITION AND CONSTRUCTION MEANS AND METHODS, TECHNIQUES, SEQUENCING, AND SAFETY REQUIRED TO COMPLETE CONSTRUCTION.
5. UNLESS OTHERWISE NOTED, MATERIAL AND DESIGN SPECIFICATIONS CITED HEREIN SHALL BE THOSE CONFORMING WITH THE VERSION OF THE APPLICABLE SPECIFICATIONS OR CODE MOST RECENTLY ADOPTED BY THE PERMITTING AUTHORITY. THESE STRUCTURAL NOTES ARE TO BE USED AS A SUPPLEMENT TO THE SPECIFICATIONS.
6. THIS STRUCTURE AND ALL OF ITS PARTS MUST BE ADEQUATELY BRACED AGAINST WIND, LATERAL EARTH AND SEISMIC FORCES UNTIL THE PERMANENT LATERAL-FORCE RESISTING SYSTEMS HAVE BEEN CONSTRUCTED AND ALL ATTACHMENTS AND CONNECTIONS NECESSARY FOR THE STABILITY OF THE STRUCTURE AND ITS PARTS HAVE BEEN MADE.
7. ALL FEATURES OF CONSTRUCTION NOT FULLY SHOWN SHALL BE OF THE SAME TYPE AND CHARACTER AS SHOWN FOR SIMILAR CONDITIONS, SUBJECT TO REVIEW BY THE ARCHITECT AND STRUCTURAL ENGINEER OF RECORD.
8. ALL PRODUCTS AND MATERIALS USED BY THE CONTRACTOR SHALL BE APPLIED, PLACED, ERECTED OR INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS.
9. ALL KEYNOTES INDICATE NEW ITEMS TYPICALLY UNLESS NOTED OTHERWISE.

CODE REQUIREMENT:

1. CONFORM TO THE 2022 OREGON STRUCTURAL SPECIALTY CODE, BASED ON THE 2021 INTERNATIONAL BUILDING CODE (IBC). NOTE: THIS APPLIES TO ALL REFERENCES TO OSSC.

DESIGN CRITERIA:

1. DESIGN IS BASED ON THE STRENGTH AND DEFLECTION CRITERIA OF THE OSSC. IN ADDITION TO THE DEAD LOADS, THE FOLLOWING LOADING AND ALLOWABLE LOAD IS USED FOR DESIGN:

A. LIVE LOADS:

SLAB ON GRADE250 PSF

ROOF20 PSF

OFFICE SPACE50 PSF

CLASSROOM40 PSF

B. GROUND SNOW LOAD:

EXPOSURE FACTOR1.0

SNOW IMPORTANCE FACTOR1.1

THERMAL FACTOR1.0

FLAT ROOF SNOW LOAD22-27 PSF

C. WIND LOAD:

BASIC WIND SPEED (3-SECOND GUST)104 MPH

WIND EXPOSUREC

WIND IMPORTANCE FACTOR1.0

BUILDING CATEGORYIII

INTERNAL PRESSURE COEFFICIENT0.18

TOPOGRAPHIC FACTOR1.13

D. EARTHQUAKE DESIGN DATA:

RISK CATEGORYIII

BSE-1N

S<sub>s</sub>0.898g

S<sub>1</sub>0.349g

S<sub>ps</sub>0.683g

S<sub>o1</sub>0.454

D

SITE CLASSD

SEISMIC DESIGN CATEGORYD

SEISMIC IMPORTANCE FACTOR1.25

ANALYSIS PROCEDUREEQUIVALENT LATERAL FORCE

BASIC SEISMIC-FORCE RESISTING SYSTEM:

LIGHT-FRAME WALLS SHEATHED WITH WOOD STRUCTURAL PANELS

RESPONSE MODIFICATION FACTORR = 6.5

SEISMIC RESPONSE COEFFICIENTC<sub>s</sub> = 0.131

STRUCTURAL OBSERVATION:

THE STRUCTURAL ENGINEER OF RECORD (SEOR) WILL PERFORM STRUCTURAL OBSERVATION BASED ON THE REQUIREMENTS OF THE OSSC AT THE STAGES OF CONSTRUCTION LISTED BELOW. CONTRACTOR SHALL PROVIDE SUFFICIENT ADVANCED NOTICE AND ACCESS FOR THE SER TO PERFORM THESE OBSERVATIONS.

STRUCTURAL OBSERVATION	
ITEM	COMMENTS
PRIOR TO FIRST CONCRETE POUR	AFTER REBAR PLACEMENT
DURING INITIAL STEEL ERECTION	--
PRIOR TO COVERING SHEAR WALLS SHEATHING AND FRAMING	--
AS REQUIRED TO ADDRESS STRUCTURAL ISSUES	--

A FIELD REPORT WILL BE SUBMITTED TO THE BUILDING DEPARTMENT FOLLOWING EACH SITE VISIT.

SPECIAL INSPECTIONS AND TESTING:

1. SPECIAL INSPECTION WILL BE PROVIDED BY THE OWNER BASED ON THE REQUIREMENTS OF THE OSSC AS SUMMARIZED IN THE SPECIAL INSPECTION AND TESTING PROGRAM ON SHEETS S003-S004. CONTRACTOR SHALL PROVIDE SUFFICIENT NOTICE AND ACCESS FOR THE SPECIAL INSPECTOR TO PERFORM THESE INSPECTIONS.

SUBMITTALS:

1. SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT PRIOR TO FABRICATION AND CONSTRUCTION REGARDING ALL STRUCTURAL ITEMS, INCLUDING THE FOLLOWING:

A. CONCRETE MIX DESIGNS, CONCRETE AND MASONRY REINFORCEMENT (INCLUDING MILL TEST REPORTS), STRUCTURAL STEEL (INCLUDING MILL TEST REPORTS)

B. ANY CHANGES TO THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT AND ARE SUBJECT TO REVIEW AND ACCEPTANCE OF THE STRUCTURAL ENGINEER OF RECORD.

C. DESIGN DRAWINGS, SHOP DRAWINGS, AND CALCULATIONS FOR THE DESIGN AND FABRICATION OF ITEMS THAT ARE DESIGNED BY OTHERS, INCLUDING: GLUE-LAMINATED MEMBERS, PRE-MANUFACTURED WOOD JOISTS, PRE-MANUFACTURED WOOD ROOF TRUSS SYSTEMS, AND ATTACHMENTS (INCLUDING SUPPORT, BRACING AND ANCHORAGE), WINDOW WALL, CURTAIN WALLS, AND ALL OTHER GLAZING SYSTEMS SHALL BEAR THE SEAL AND SIGNATURE OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF OREGON, AND SHALL BE INCLUDED FOR CONNECTIONS TO THE STRUCTURE, CONSIDERING LOCALIZED EFFECTS ON STRUCTURAL ELEMENTS INDUCED BY THE CONNECTION LOADS. DESIGN SHALL BE BASED ON THE REQUIREMENTS OF THE OSSC WITH THE FOLLOWING:

• EARTHQUAKE AND WIND LOADS AS NOTED IN DESIGN CRITERIA

D. THE CONTRACTOR SHALL COORDINATE SEISMIC RESTRAINTS OF MECHANICAL, PLUMBING, AND ELECTRICAL EQUIPMENT, MACHINERY, AND ASSOCIATED PIPING WITH THE STRUCTURE. ANY CONNECTIONS TO THE STRUCTURE SHALL CONFORM TO OSSC AND SHALL BE SUBMITTED TO THE ARCHITECT PRIOR TO FABRICATION.

DIVISION 03 - CONCRETE

CONCRETE:

1. CONCRETE WORK SHALL CONFORM TO CHAPTER 19 OF THE OSSC. CONCRETE STRENGTHS SHALL BE VERIFIED BY STANDARD 28 DAY CYLINDER TESTS PER ASTM C39, AND SHALL BE AS FOLLOWS:

ABSOLUTE WATER-CEMENT RATIO BY WEIGHT				
f'c (PSI)	NON AIR-ENTRAINED	AIR-ENTRAINED	USE	
3,000	N/A	0.50	MISC. CONCRETE, CURBS, SIDEWALKS, ETC.	
4,000	0.50	N/A	INTERIOR SLABS ON GRADE	
4,000	0.45	N/A	BASEMENT WALLS AND FOOTINGS	
4,500	N/A	0.45	EXTERIOR SLABS ON GRADE, WALLS, COLUMNS, AND BEAMS	

2. VERIFY WATER/CEMENT RATIO WITH FLOOR COVERING MANUFACTURER FOR CONCRETE FLOORS WITH MOISTURE SENSITIVE FLOOR COVERINGS, AND VERIFY COORDINATE WITH PROJECT SPECIFICATIONS.
3. MINIMUM CEMENT CONTENT PER CUBIC YARD SHALL BE AS FOLLOWS:

• f'c=4,000 psi: 550 lbs.
4. FLY ASH CONFORMING TO ASTM C618 (INCLUDING TABLE 2A) TYPE F, MAY BE USED TO REPLACE UP TO 20% OF THE CEMENT CONTENT, PROVIDED THAT THE MIX STRENGTH IS SUBSTANTIATED BY TEST DATA.
5. THE CONTRACTOR SHALL SUBMIT CONCRETE MIX DESIGNS, ALONG WITH TEST DATA COMPLIANT WITH OSSC SECTION 1905, A MINIMUM OF TWO WEEKS PRIOR TO PLACING CONCRETE. NO WATER MAY BE ADDED TO CONCRETE IN THE FIELD UNLESS SPECIFICALLY APPROVED IN WRITING BY THE CONCRETE SUPPLIER IN CONJUNCTION WITH THE CONCRETE MIX DESIGN.
6. A WATER-REDUCING ADMIXTURE CONFORMING TO ASTM C494, USED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS, SHALL BE INCORPORATED IN CONCRETE DESIGN MIXES. A HIGH-RANGE WATER-REDUCING (HRWR) ADMIXTURE CONFORMING TO ASTM C494, TYPE F OR G, MAY BE USED IN CONCRETE MIXES PROVIDING THAT THE SLUMP DOES NOT EXCEED 8". AN AIR-ENTRAINING AGENT CONFORMING TO ASTM C260 SHALL BE USED IN CONCRETE MIXES FOR EXTERIOR HORIZONTAL SURFACES EXPOSED TO WEATHER. THE AMOUNT OF ENTRAINED AIR SHALL BE 5% +/- 1% BY VOLUME.

CONCRETE CAST IN PLACE:

1. CONCRETE SHALL HAVE A MAXIMUM SLUMP OF 4" WITHOUT THE USE OF ADMIXTURES AS NOTED.
2. A MINIMUM OF THREE (3) CONCRETE TEST CYLINDERS SHALL BE PROVIDED FOR EACH ONE HUNDRED (100) CU. YARDS, OR EACH DAY OF POUR, FOR EACH CONCRETE STRENGTH. CYLINDERS SHALL BE TESTED AS FOLLOWS:

A. ONE (1) AT SEVEN (7) DAYS, AND

B. TWO (2) AT TWENTY-EIGHT (28) DAYS
3. CONCRETE CYLINDER SAMPLING AND TESTING SHALL CONFORM WITH ASTM SPECIFICATIONS. ACCEPTANCE OF CONCRETE SHALL BE GOVERNED BY THE PROVISIONS OF ACI 318-19 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE". TWO (2) SETS OF MIX DESIGNS, WITH COMPLETE STATISTICAL BACKUP, SHALL BE SUBMITTED FOR REVIEW.
4. CONCRETE MATERIALS, FORM WORK, MIXING, PLACING AND CURING SHALL CONFORM WITH THE SPECIFICATIONS CONTAINED IN THE ACI "MANUAL OF CONCRETE PRACTICE".
5. AT AREAS OF DEPRESSIONS FOR SLABS AND BEAMS, PROVIDE MINIMUM THICKNESS OF DEPTH AS FOR ADJACENT AREAS, UNLESS NOTED OTHERWISE.
6. CONCRETE SLABS SHALL BE INSTALLED WITH CONSTRUCTION JOINTS NOT SPACED FARTHER THAN 12'-0" APART AND SHALL BE DIVIDED INTO APPROXIMATELY SQUARE PANELS. PANEL DIMENSION RATIOS SHALL NOT EXCEED 1.5:1.
7. ALL SAW CUT CONTROL JOINTS SHALL BE CUT WITHIN 4 TO 12 HOURS AFTER CONCRETE PLACEMENT. SAW CUT SHALL BE 1.5" DEEP.
8. CONCRETE SHALL NOT BE PLACED ON FROZEN GROUND.
9. BOND NEW CONCRETE TO EXISTING CONCRETE WITH "WELD-CRETE", AS MANUFACTURED BY LARSON PRODUCTS CORPORATION, OR APPROVED. AS A MINIMUM, EXISTING CONCRETE SURFACES SHALL BE STRENGTHENED BY CHIPPING TO A MINIMUM 1/4" AMPLITUDE TO EXPOSE COARSE AGGREGATE. PREPARATION AND APPLICATION IS TO BE IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
10. ALL EXPOSED CORNERS SHALL HAVE 3/4" CHAMFER, UNLESS NOTES OTHERWISE.

CONCRETE REINFORCING STEEL:

1. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60. FOR DEFORMED BARS AND ASTM A185 FOR SMOOTH WELDED WIRE FABRIC (WWF), UNLESS OTHERWISE NOTED. REINFORCING STEEL TO BE WELDED SHALL CONFORM TO ASTM A706. REINFORCING STEEL SHALL BE SECURELY TIED IN PLACE WITH #16 ANNEALED IRON WIRE.
2. BARS IN SLABS SHALL BE SUPPORTED ON WELL CURED CONCRETE BLOCKS OR APPROVED METAL CHAIRS, AS SPECIFIED BY THE CRSI MANUAL OF STRANDED PRACTICE, MSP-1. REINFORCING STEEL SHALL BE DETAINED IN ACCORDANCE WITH THE "ACI MANUAL OF STANDARD PRACTICE, MSP-1 REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE WITH THE "ACI MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES", ACI 315. LAP ALL REINFORCING BARS PER THE TYPICAL LAP SPLICE LENGTH SCHEDULE, EXCEPT AS NOTED. MECHANICAL SPLICES NOTED ON THE PLANS SHALL BE DAYTON BAR-GRIP SPLICES OR APPROVED WITH A CURRENT ICC APPROVAL REPORT.

TYPICAL LAP SPLICE LENGTH SCHEDULE							
BAR SIZE	3,000 psi		4,000 psi		5,000 psi		6,000 psi
	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1 CASE 2
#3	22	32	19	28	17	25	16 23
#4	29	43	25	37	22	33	20 31
#5	36	54	31	47	28	42	25 38
#6	43	64	37	56	33	50	31 46
#7	63	94	54	81	49	73	44 66

- NOTES:
- A. DIMENSIONS ARE IN INCHES.
- B. CASES 1 AND 2 ARE DEFINED AS FOLLOWS: (db = BAR DIAMETER)

a. BEAMS OR COLUMNS:

• CASE 1: COVER ≥ db AND c-c SPACING ≥ 2db

• CASE 2: COVER < db OR c-c SPACING < 2db

b. ALL OTHERS:

• CASE 1: COVER ≥ db AND c-c SPACING ≥ 3db

• CASE 2: COVER < db OR c-c SPACING < 3db
- C. FOR TOP BARS, MULTIPLY LAP LENGTH ABOVE BY 1.3. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12" OF CONCRETE CAST BELOW THE BARS.
3. REINFORCEMENT SHALL BE SECURED IN FORMS WITH TIES AND ANCHORAGE TO PREVENT DISPLACEMENT. ALL TIE WIRE SHALL BE MIN. #16 ANNEALED STEEL.
4. ALL REINFORCING STEEL SHALL BE EMBEDDED 100% ALONG ALL PERIMETER EDGES AND 50% FIELD.
5. REINFORCING (MINIMUM UNLESS NOTED OTHERWISE ON PLANS)

A. PLACE TWO (2) NO. 4 CONTINUOUS AT BOTTOM, TOP AND AT DISCONTINUOUS ENDS OF ALL FOUNDATIONS.

B. PLACE BARS AT CORNERS AND INTERSECTIONS FOR WALLS AND FOUNDATIONS EQUAL IN SIZE AND NUMBER TO HORIZONTAL REINFORCING WITH LEGS THAT SATISFY THE REQUIRED LAP SPLICE LENGTH PER SCHEDULE ABOVE.

C. PLACE TWO (2) NO. 4x OPENING DIMENSIONS PLUS 4'-0" EACH SIDE OF ALL OPENINGS AND TWO (2) NO. 4x 4'-0" DIAGONAL BARS AT EACH CORNER OF ALL SLAB OPENINGS GREATER THAN 1'-6" IN DIMENSION.
6. ALL WELDED WIRE FABRIC SHALL CONFORM WITH ASTM A 185. ALL WIRE FABRIC SHALL BE SUPPLIED, LAID IN FLAT SHEETS AND CHAIRED TO PROPER POSITION IN SLABS. LAP ONE (1) FULL MESH PLUS 2" ON SIDES AND ENDS.
7. ALL REINFORCING STEEL SHALL BE DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH ACI DETAILING MANUAL 315.

A. ALL REINFORCING STEEL SHALL BE ACCURATELY AND SECURELY PLACED.

B. REINFORCING SHALL NOT BE BENT OR DISPLACED FOR THE CONVENIENCE OF OTHER TRADES, UNLESS APPROVED BY THE STRUCTURAL ENGINEER.

C. SPLAY REINFORCING STEEL AROUND OPENINGS WITH 1" IN 10" SPLAY, UNLESS NOTED OTHERWISE.

D. MINIMUM COVER FROM CONCRETE SURFACES TO REINFORCING STEEL SHALL BE:

• 3" TO BOTTOM OF FOOTING

• 2" TO EARTH FACE OF WALL

• 3/4" TO INSIDE FACE OF WALL

• 1 1/2" TO MAIN STEEL BEAMS AND COLUMNS

• 3/4" SLAB TO TOP AND BOTTOM SURFACES, CENTER OF SLAB ON GRADE
8. REINFORCEMENT BARS SHALL NOT BE TACK WELDED, WELDED, HEATED OR CUT, UNLESS INDICATED ON THE CONTRACT DOCUMENTS OR APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.
9. REINFORCEMENT COUPLERS SHALL BE LENTON, FOX-HOWLETT OR APPROVED, CAPABLE OF DEVELOPING ONE HUNDRED TWENTY-FIVE PERCENT (125%) OF THE SPECIFIED YIELD STRENGTH OF THE REINFORCEMENT.

CONCRETE ACCESSORIES:

1. EXPANSION BOLTS SHALL BE HILTI KWIK TZ, SIMPSON STRONG BOLT, DEWALT POWER-STUD+SD2, OR APPROVED WITH EQUIVALENT ICC ALLOWABLE TENSION AND SHEAR VALUES. EXPANSION BOLTS SHALL BE INSTALLED IN STRICT CONFORMANCE WITH MANUFACTURER'S RECOMMENDATIONS. DO NOT CUT REINFORCING IN NEW OR EXISTING CONCRETE DURING INSTALLATION.
2. EPOXY ADHESIVE SHALL BE HILTI HIT-RE 500 V3, SIMPSON SET-36, DEWALT PURE110+ EPOXY, DEWALT AC208+ ACRYLIC, OR APPROVED WITH EQUIVALENT ICC ALLOWABLE TENSION AND SHEAR VALUES. EPOXY ANCHORS SHALL BE INSTALLED IN STRICT CONFORMANCE WITH MANUFACTURER'S RECOMMENDATIONS. DO NOT CUT REINFORCING IN NEW OR EXISTING CONCRETE DURING INSTALLATION.
3. PERMANENTLY EXPOSED EMBEDDED PLATES AND ANGLES SHALL BE HOT-DIPPED, GALVANIZED AFTER FABRICATION, UNLESS OTHERWISE NOTED. NO LOADS OR WELDS SHALL BE PLACED ON EMBEDDED PLATES OR ANGLES FOR A MINIMUM OF 7 DAYS AFTER CASTING.
4. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS SHALL BE DONE BY A CERTIFIED ADHESIVE ANCHOR INSTALLER (AAI) AS CERTIFIED THROUGH ACI/CRSI, OR AN APPROVED ALTERNATE WHEN SUBMITTED AND APPROVED BY THE EOR (ACI 318-19 D.9.2.2)(ACI 318-19 17.8.2.2). PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO COMMENCEMENT OF INSTALLATION.
5. ADHESIVE ANCHORS MUST BE INSTALLED IN CONCRETE AGED A MINIMUM OF 21 DAYS (ACI 318-19 17.2.2).

FOUNDATIONS:

1. FOUNDATION SIZES ARE BASED ON TOTAL LOAD BEARING PRESSURE OF 2,000 psf FOR BEARING ON COMPACTED NATIVE SOILS OR ENGINEERED FILL PER GEOTECHNICAL REPORT.
2. ALL FOUNDATION ELEMENTS SHALL BE PLACED OVER FIRM, UNDISTURBED NATIVE SOIL OR ON APPROVED STRUCTURAL FILL. THE EXISTING SITE SHALL BE CLEARED AND GRUBBED OF ALL ORGANIC AND/OR EXPANSIVE MATERIAL PRIOR TO STRUCTURAL FILL IMPORT. REFER TO THE PROJECT GEOTECHNICAL REPORT FOR SUBGRADE REQUIREMENTS.
3. ALL PERIMETER FOUNDATION ELEMENTS SHALL BE PLACED A MINIMUM OF 24" BELOW LOWEST ADJACENT EXTERIOR GRADE.
4. THE GENERAL CONTRACTOR SHALL COORDINATE ALL SLAB PENETRATIONS WITH PLUMBING, ELECTRICAL AND MECHANICAL REQUIREMENTS.
5. THE GENERAL CONTRACTOR SHALL REVIEW THE PROJECT GEOTECHNICAL REPORT FOR ADDITIONAL RECOMMENDATIONS FOR STRUCTURAL FILL, PLACEMENT OF STRUCTURAL FILL, AND COMPACTION REQUIREMENTS.
6. BOTTOM OF FOOTINGS SHALL BE STEPPED AT 2'-0" HORIZONTAL TO 1'-0" VERTICAL STEPS OR SHALL BE SLOPED NOT TO EXCEED 1 VERTICAL TO 4 HORIZONTAL WHEN PRIOR APPROVAL IS RECEIVED BY ENGINEER.

NON-SHRINK GROUT:

1. GROUT SHALL BE NON-SHRINKABLE GROUT CONFORMING WITH ASTM C 1107 AND C.R.D. - 621, CORPS OF ENGINEERS "SPECIFICATIONS FOR NON-SHRINK GROUT". GROUT SHALL HAVE A SPECIFIED COMPRESSIVE STRENGTH AT TWENTY-EIGHT (28) DAYS OF 5000 psi. PRE-GROUTING OF BASE PLATES WILL NOT BE PERMITTED.

DIVISION 04 - MASONRY

CONCRETE MASONRY:

1. CONCRETE MASONRY UNITS SHALL COMPLY WITH ASTM C90, SAMPLED AND TESTED IN ACCORDANCE WITH ASTM C140 WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2,800 psi. LINEAR SHRINKAGE FOR UNITS SHALL NOT EXCEED 0.065%. ASSEMBLIES SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF f'm = 2,000 psi AS VERIFIED BY PRISM TESTS BEFORE AND DURING CONSTRUCTION. CONCRETE MASONRY WALLS SHALL BE REINFORCED AS SHOWN ON THE PLANS AND DETAILS AND, IF NOT SHOWN, SHALL BE AS NOTED UNDER "MASONRY REINFORCING STEEL".

MORTAR:

1. MORTAR SHALL BE TYPE S, WITH A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 1,800 psi, AND SHALL CONFORM TO OSSC SECTION 2103.

MASONRY GROUT:

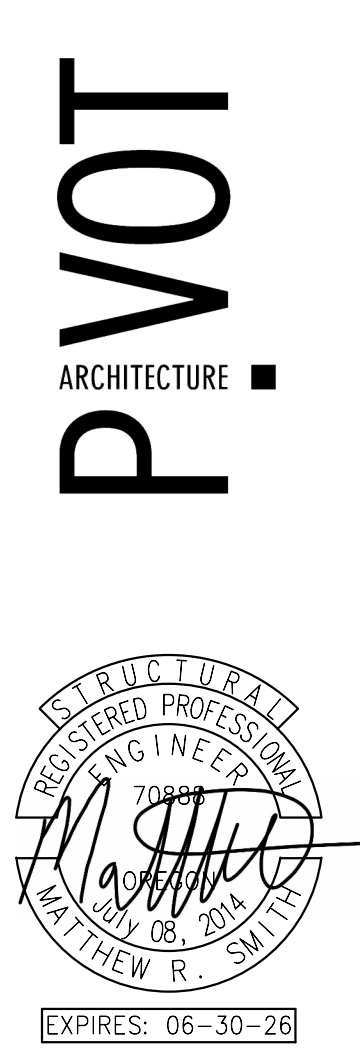
1. GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2,000 psi AT 28 DAYS AND SHALL CONFORM TO OSSC SECTION 2103. GROUT SHALL CONSIST OF A MIXTURE OF CEMENTITIOUS MATERIALS AND AGGREGATE TO WHICH SUFFICIENT WATER HAS BEEN ADDED TO CAUSE THE MIXTURE TO FLOW WITHOUT SEGREGATION OF THE CONSTITUENTS. ALL CELLS CONTAINING VERTICAL BARS AND ALL BOND BEAMS SHALL BE FILLED WITH GROUT. FULLY GROUT WALLS WHERE INDICATED.

MASONRY REINFORCING STEEL:

1. REINFORCING SHALL CONFORM TO OSSC SECTION 2103. DEFORMED BARS SHALL BE ASTM A615 GRADE 60, AND SHALL BE SECURELY PLACED IN ACCORDANCE WITH TMS 602 SECTION 3.4. UNLESS OTHERWISE NOTED ON THE PLANS, THE MINIMUM WALL REINFORCEMENT SHALL BE AS FOLLOWS:

TYPICAL MASONRY WALL REINFORCING		
WALL THICKNESS	VERTICAL BARS	HORIZONTAL BARS
		STACKBOND
8"	#7 @ 16" o.c.	(2) #5 @ 24" o.c.

2. BOND BEAMS WITH TWO #6 BARS HORIZONTALLY SHALL BE PROVIDED AT ALL FLOOR AND ROOF LINES AND AT THE TOP OF THE WALLS. STEP BOND BEAMS AS REQUIRED TO MATCH ROOF SLOPES. PROVIDE A BOND BEAM WITH TWO #6 BARS HORIZONTALLY ABOVE AND BELOW ALL OPENINGS, AND EXTEND THESE BARS 2'-0" PAST THE OPENING AT EACH SIDE. PROVIDE ONE BAR, MATCHING VERTICAL BAR SIZE, FOR THE FULL HEIGHT OF THE WALL AT EACH SIDE OF OPENINGS, WALL ENDS, AND INTERSECTIONS. DOWELS TO MASONRY WALLS SHALL BE EMBEDDED A MINIMUM OF 1'-6" OR HOOKED INTO THE SUPPORTING STRUCTURE AND BE OF THE SAME SIZE AND SPACING AS WALL INTERSECTIONS. LAP ALL BARS AT SPLICES 48 DIAMETERS, WITH A MINIMUM LAP OF 18", EXCEPT AS NOTED.



BID & PERMIT SET  
KLAMATH COMMUNITY COLLEGE  
EARLY LEARNING CENTER

PROJECT #: K-6381-24  
KLAMATH COMMUNITY COLLEGE  
7390 S. 6TH AVE.  
KLAMATH FALLS, OR 97603

SHEET TITLE:

STRUCTURAL  
GENERAL NOTES

REVISIONS:

#	DESCRP.	DATE
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ISSUE DATE: 08/01/2025

S001

DIVISION 05 - METALS

STRUCTURAL STEEL AND MISCELLANEOUS IRON:

STRUCTURAL STEEL SHALL BE:

STRUCTURAL STEEL	
ASTM A992, GRADE 50	WIDE FLANGE SHAPES
ASTM A36	CHANNELS, PLATES, AND ANGLES, U.N.O.
ASTM A500, GRADE B (Fy = 46 KSI)	HOLLOW STRUCTURAL SECTIONS (TUBES)
ASTM A53, GRADE B (Fy = 35 KSI)	PIPES

- DESIGN, FABRICATION, AND ERECTION SHALL BE IN ACCORDANCE WITH THE "AISC SPECIFICATION FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" WITH "COMMENTARY" AND THE "CODE OF STANDARD PRACTICE", WITH EXCEPTIONS NOTED IN SPECIFICATIONS.
- DRAWINGS ARE DIMENSIONED FOR LAYOUT AND NOT DIMENSIONED PER AISC STANDARDS. IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO COORDINATE BETWEEN ALL DRAWINGS AND DEVELOP SHOP DRAWINGS WITH DETAIL AND DIMENSIONING PER AISC.
- ALL FABRICATION, ERECTION, IDENTIFICATION, AND PAINTING SHALL CONFORM TO AISC SPECIFICATIONS.
- ALL STEEL EXPOSED TO WEATHER, SOIL, MOISTURE, OR AS DENOTED ON PLANS SHALL BE HOT DIP GALVANIZED PER ASTM A-123, OR OTHER APPROVED PROTECTIVE COATING.
- ALL WELDING SHALL CONFORM TO AWS (LATES EDITION) SPECIFICATIONS.
  - ALL WELDERS TO BE QUALIFIED UNDER AWS SPECIFICATIONS WITHIN THE PAST TWO YEARS FOR THE TYPE OF WELDING PERFORMED.
  - ALL WELDS SHALL BE PERFORMED USING PRE-QUALIFIED WELDING PROCEDURES.
  - WELDS FILLER METAL SHALL BE AWS A5.1 OR A5.5 E70XX ELECTRODES OR AWS A5.18 ER70S-X OR A5.2 E7XT-X.
  - AFTER FABRICATION, BUT BEFORE INSTALLATION, REMOVE RUST, SCALE, GREASE, AND OIL BY WIRE BRUSHING AND CHEMICAL TREATMENT.
  - WELDING OF REINFORCING STEEL SHALL BE AS SPECIFIED IN THESE STRUCTURAL NOTES UNDER "CONCRETE REINFORCING STEEL."
- ALL MEMBERS SHALL BE CONNECTED WITH SEMI-FINISHED MACHINE BOLTS, UNLESS NOTED OTHERWISE ON PLANS. MACHINE BOLTS SHALL CONFORM TO ASTM A 307, GRADE A.
- STRUCTURAL STEEL AND MISCELLANEOUS IRON:
  - EXPANSION ANCHORS SHALL BE I.C.B.O. APPROVED (ZINC PLATED IN ACCORDANCE WITH ASTM B633, HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A 153, A.I.S.I. 304 STAINLESS STEEL) AND CONFORM WITH FS FF-S-325, GROUP II, TYPE 4, CLASS 1. ACCEPTABLE ANCHORS ARE HILTI "KWIK-BOLT 1Z", SIMPSON STRONG BOLT, OR DEWALT POWER STUD+. ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
  - SLEEVE ANCHORS SHALL BE I.C.B.O. APPROVED (ZINC PLATED IN ACCORDANCE WITH ASTM B 633, A.I.S.I. 304 STAINLESS STEEL) AND CONFORM WITH FS FF-S-325, GROUP II, TYPE 3, CLASS 3. AN ACCEPTABLE ANCHOR IS THE HILTI "SLEEVE" ANCHOR, AS MANUFACTURED BY THE HILTI FASTENING SYSTEMS, INC. ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
  - FLUSH SHELL ANCHORS SHALL BE I.C.B.O. APPROVED (ZINC PLATED IN ACCORDANCE WITH ASTM B 633, A.I.S.I. 303 STAINLESS STEEL) AND CONFORM WITH FS FF-S-325, GROUP VIII, TYPE 1. AN ACCEPTABLE ANCHOR IS THE HILTI "HD" ANCHOR, AS MANUFACTURED BY HILTI FASTENING SYSTEMS, INC. ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
  - ADHESIVE ANCHORS SHALL BE I.C.B.O. APPROVED AND SHALL CONSIST OF ALL-THREAD ANCHOR ROD, NUT, WASHER AND EPOXY INJECTION GEL SYSTEM. ANCHOR RODS SHALL BE MANUFACTURED FROM:
    - A-36 MATERIAL (ZINC PLATED IN ACCORDANCE WITH ASTM B 633, HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A 153).
    - ASTM A 193, GRADE B-7 MATERIAL (ZINC PLATED IN ACCORDANCE WITH ASTM B 633, HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A 153).
    - A.I.S.I. 304 OR 316 STAINLESS STEEL, IN ACCORDANCE WITH ASTM F 593. ANCHOR RODS SHALL HAVE ROLLED THREADS. NUTS SHALL CONFORM WITH ASTM A 194. ACCEPTABLE ADHESIVE INJECTION GEL SYSTEMS ARE THE HILTI HIT-RE 500 V3, SIMPSON SET XP OR DEWALT 1000+. ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
- ANCHOR BOLT SHALL CONFORM WITH ASTM A 307, GRADE A, AND SHALL BE PROVIDED WITH STANDARD WASHERS AND NUTS. GALVANIZE EXTERIOR BOLTS. GALVANIZING SHALL BE IN ACCORDANCE WITH ASTM A 153, CLASS C. NUTS SHALL BE OVER-TAPPED TO CLASS 2A FIT BEFORE GALVANIZING, IN ACCORDANCE WITH ASTM A 563.
- BOLT HEADS OR NUTS BEARING ON SLOPING FLANGES SHALL BE EQUIPPED WITH BEVELED WASHERS.
- ERECTION AIDS (SUCH AS BOLTS, CLIPS, SHIMS, SEATS OR ANY OTHERS REQUIRED TO FACILITATE CONSTRUCTION) ARE THE RESPONSIBILITY OF THE CONTRACTOR TO DESIGN AND PROVIDE.
- ALL BRACING SHALL HAVE TWO (2) BOLT CONNECTIONS, UNLESS NOTED OTHERWISE. ALL CROSS BRACING SHALL BE BOLTED AT INTERSECTIONS WITH TWO (2) BOLT MINIMUM FOR ST AND ONE (1) BOLT FOR ANGLES. PROVIDE FILLER PLATE BETWEEN CROSS BRACES, AS REQUIRED.
- ALL FIELD WELDS TO GALVANIZED STEEL AND AREAS DAMAGED BY WELDING, FLAME CUTTING OR HANDLING, SHALL BE REPAIRED WITH AN ORGANIC COLD GALVANIZING COMPOUND HAVING A MINIMUM OF NINETY-FOUR PERCENT (94%) ZINC DUST IN THE DRY FILM. APPLY IN MULTIPLE COATS, UNTIL AN 8 MIL THICKNESS HAS BEEN ACHIEVED. SURFACES TO RECEIVE ZINC-RICH PAINT SHALL BE CLEAN, DRY AND FREE OF OIL, GREASE, SALT AND CORROSION PRODUCTS.
- ALL EMBEDDED STEEL SHALL BE FABRICATED FROM MATERIAL CONFORMING WITH THE REQUIREMENTS OF ASTM A 36. HOT-DIP GALVANIZE IN ACCORDANCE WITH ASTM A 123, UNLESS NOTED OTHERWISE.

DIVISION 06 - WOOD, PLASTICS AND COMPOSITES

FRAMING LUMBER:

- ALL FRAMING LUMBER SHALL BE DOUGLAS FIR-LARCH AND SHALL BE GRADED UNDER THE MOST RECENTLY ADOPTED RULES OF THE WEST COAST LUMBER INSPECTION BUREAU (WCLIB).
- ALL BEAMS AND JOISTS SHALL BE NO. 2 MINIMUM, UNLESS INDICATED OTHERWISE ON THE PLANS.
- ALL STUDS AND BLOCKING SHALL BE NO. 2.
- ALL LUMBER IN CONTACT WITH CONCRETE OR EXPOSED SHALL BE PRESSURE TREATED IN ACCORDANCE WITH AWPFA STANDARD C-2 AND SHALL BEAR THE AWPFA QUALITY MARK.
- DOUBLE ALL JOISTS UNDER WALL PARTITIONS, AND PROVIDE BLOCKING BETWEEN JOISTS WHERE BEARING WALLS ARE PERPENDICULAR TO JOISTS.
- ALL GLULAM BEAMS TO BE 24F-V4 TYPICAL. 24F-V8 FOR CANTILEVERED OR CONTINUOUS SPAN.
- ALL LVL LUMBER TO BE MICROLAM LVL OR APPROVED EQUAL.
- ALL PSL LUMBER TO BE PARALLAM PSL OR APPROVED EQUAL.
- ALL LSL LUMBER TO BE TIMBERSTRAND LSL OR APPROVED EQUAL.
- GLULAM COLUMNS TO BE BOISE CASCADE COMBINATION 3 COLUMN GRADE OR APPROVED EQUAL.

PLYWOOD SHEATHING:

- ALL PLYWOOD SHALL BE C-D GRADE WITH EXTERIOR GLUE MANUFACTURED IN ACCORDANCE WITH THE UNITED STATES PRODUCT STANDARDS PS 1-83/ANSI A199.1 "FOR CONSTRUCTION AND INDUSTRIAL PLYWOOD" AND SHALL CONFORM TO OSSC SECTION 2303 AND SHALL BEAR THE APA TRADEMARK OF THE APA.
- PLYWOOD SHALL BE LAID WITH END JOINTS STAGGERED.
- BLOCK ALL SHEAR WALL SHEATHING WITH 2x BLOCKING AT ALL EDGES.
- ROOF SHEATHING TO BE UNBLOCKED 5/8" C-D 40/20 PLY, UNLESS NOTED OTHERWISE ON PLANS.
- FLOOR SHEATHING TO BE UNBLOCKED 1 1/8" 2-4-1 T&G C-D 32/16 PLY, UNLESS NOTED OTHERWISE.
- EXTERIOR WALLS TO BE 1/2" EXPOSURE I, C-D PLY, OR OSB SHEATHING U.N.O. SEE PLANS FOR SHEAR WALL TYPE AND CORRESPONDENCE SHEAR WALL SCHEDULE FOR REQUIREMENTS.
- OSB MAY BE SUBSTITUTED FOR PLYWOOD WITH SAME SPAN RATING.

NAILING AND FASTENERS:

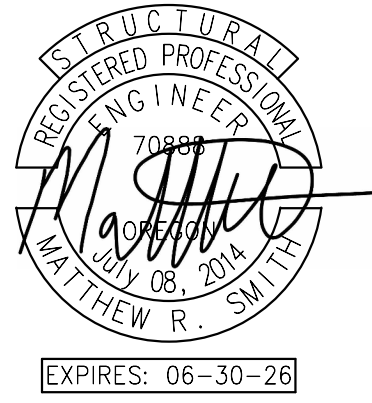
- NAILING INDICATED ON PLANS AND DETAILS ARE "COMMON" NAILS. MINIMUM FRAMING NAILING SHALL CONFORM TO OSSC TABLE 2304.10.2. SEE DETAILS FOR ADDITIONAL TYPICAL NAILING REQUIREMENTS. SUBSTITUTION OF NAILS OTHER THAN "COMMON" IS NOT PERMITTED WITHOUT PRIOR APPROVAL.
- POWER DRIVEN NAILS OTHER THAN "COMMON" NAILS MAY BE USED IF DATA IS SUBMITTED AND APPROVED PRIOR TO USE.
- APPLY 1/4" DIAMETER CONTINUOUS BEAD OF GLUE TO TOPS OF WOOD FRAMED FLOOR JOISTS, BLOCKING, AND PLATES IMMEDIATELY PRIOR TO FLOOR SHEATHING.
- ALL BOLTS AND CONNECTIONS SHALL BE MADE WITH MACHINE BOLTS (M.B.) CONFORMING TO ASTM A307. ALL BOLTS AND LAGS SHALL BE INSTALLED WITH STANDARD WASHERS, UNLESS NOTED.
- JOIST HANGERS, HOLD-DOWNS AND OTHER FRAMING ACCESSORIES ARE REFERRED TO ON PLANS BY PARTICULAR TYPE AS MANUFACTURED BY SIMPSON COMPANY, SAN LEANDRO, CA. ALL HARDWARE IS TO BE FASTENED PER MANUFACTURER'S SPECIFICATIONS, U.N.O.
- ALL PLATES AND LEDGERS SHALL BE ANCHORED WITH A MINIMUM OF THREE FASTENERS PER PIECE.
- PRE-DRILL HOLES FOR LAG BOLTS. SOAP THREADS OF LAGS IMMEDIATELY PRIOR TO INSTALLATION.
- EPOXY ANCHOR BOLTS AND ADHESIVE INDICATED ON DRAWINGS MAY BE SUBSTITUTED UPON CONTRACTORS REQUEST WITH E.O.R. APPROVED EQUAL. DEPTH OF EMBEDMENT SHALL BE AS PER MANUFACTURER SPECIFICATIONS, UNLESS NOTED OTHERWISE. INSTALL ALL EPOXY FASTENERS IN STRICT ACCORDANCE WITH THE MANUFACTURER'S REQUIREMENTS.
- PLYWOOD NAILING SHALL BE AS NOTED ON PLANS.
- SILL AT WALLS SHALL BE BOLTED TO CONCRETE WITH 5/8" DIAMETER x 10" LONG ANCHOR BOLTS AT 4'-0" o.c. MAXIMUM AND WITHIN 1'-0" OF SILL PLATE ENDS, CORNERS OR SPLICES, UNLESS DETAILED OTHERWISE. STEEL PLATE WASHERS SHALL BE INSTALLED AT ALL ANCHOR BOLTS IN SHEAR WALLS, WITH MINIMUM DIMENSIONS OF 3" x 3" x 3/16" THICK, AND SHALL BE INSTALLED WITHIN 1/2" OF SHEATHING FACE. SIMPSON BPS BEARING PLATE OR APPROVED EQUAL MAY BE USED.

PRE-FABRICATED FLOOR AND ROOF JOISTS:

- ALL ROOF AND FLOOR MEMBERS SHALL BE MANUFACTURED BY BOISE CASCADE OR APPROVED EQUAL.
- ALL JOISTS SHALL CONFORM WITH ALL APPLICABLE PROVISIONS OF THE OSSC.
- JOISTS SHALL NOT EXCEED A LIVE LOAD DEFLECTION OF L/240 AT ROOF AND L/480 AT FLOORS, OR THE WORKING STRESSES AS SHOWN IN THE OSSC.
- JOIST MANUFACTURER SHALL FURNISH COMPLETE ENGINEERING SHOP DRAWINGS WITH REGISTERED PROFESSIONAL ENGINEER'S SEAL.
- JOISTS
  - MANUFACTURER SHALL FURNISH ALL BRIDGING AND BRACING, AS REQUIRED TO PROVIDE A COMPLETE FLOOR OR ROOF SYSTEM. LAMINATED VENEER LUMBER, NOTED ON THE PLANS AS "MICRO-LAM" OR "LVL", SHALL BE PROVIDED BY THE JOIST MANUFACTURER. IT SHALL BE 1 3/4" WIDE AND MATCH THE FLOOR JOIST DEPTH.
- LUMBER, NOTED ON THE PLANS AS "RIM BOARD", SHALL BE TIMBERSTRAND RIM BOARD MATERIAL, AS MANUFACTURED BY TRUS JOIST MACMILLAN CORPORATION.
- CAMBERS
  - ROOF MEMBERS
    - 1.25 DEAD LOAD
  - FLOOR MEMBERS
    - 1.25 DEAD LOAD
- DOUBLE ALL JOISTS UNDER MECHANICAL UNITS, UNLESS NOTED OTHERWISE.
- LOADS HUNG FROM JOISTS SHALL NOT EXCEED THIRTY (30) POUNDS AT ANY ONE (1) POINT, NOR SHALL TOTAL LOADS IN POUNDS ON ANY ONE (1) JOIST EXCEED SIX (6) TIMES THE SPAN IN FEET, UNLESS DETAILED OTHERWISE ON THE DRAWINGS. ATTACHMENTS OF LOADS EXCEEDING ONE HUNDRED (100) POUNDS SHALL BE APPROVED PRIOR TO INSTALLATION.
- DO NOT NOTCH OR DRILL STRUCTURAL MEMBERS, EXCEPT AS ALLOWED BY OSSC OR AS APPROVED BY THE STRUCTURAL ENGINEER OR RECORD PRIOR TO INSTALLATION.

LIGHT-METAL PLATE CONNECTED WOOD TRUSSES:

- TRUSSES SHALL COMPLY WITH ALL PROVISIONS OF THE DESIGN SPECIFICATIONS FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES OF THE TRUSS PLATED INSTITUTE, EXCEPT AS NOTED OTHERWISE.
- ALL TRUSS AND COMPRESSIVE MEMBER STAY BRACING AND CONNECTIONS SHALL BE MANUFACTURED BY THE TRUSS MANUFACTURER.
- MEMBERS SHALL NOT EXCEED A LIVE LOAD DEFLECTION OF L/360 FOR ROOFS OR L/600 FOR FLOORS, OR WORKING STRESSES FOR THE APPROPRIATE MATERIAL UNDER A TOTAL LOAD OF: 44 PSF FOR ROOF MEMBERS, U.N.O.
- SHOP DRAWINGS SHALL CLEARLY SHOW ERECTION PLAN, ALL STAY BRACING FOR TRUSS COMPRESSIVE MEMBERS AND REQUIRED CONNECTIONS. TRUSS DESIGNS SHALL INCLUDE ALL I.C.B.O. APPROVAL INFORMATION.
- ALL CONNECTION PLATES SHALL DEVELOP THE FULL STRESS IN A MEMBER WITH A MINIMUM TRANSFER AT ANY MEMBER OF TWO THOUSAND (2,000) LBS.
- THE TRUSS MANUFACTURER SHALL SUBMIT CERTIFICATES FROM AND INDEPENDENT INSPECTION COMPANY ASSERTING THAT ALL TRUSSES DELIVERED TO PROJECT SITE CONFORM WITH APPROVED SHOP DRAWINGS. COST OF ALL INSPECTION CERTIFICATES SHALL BE BORNE BY THE TRUSS MANUFACTURER AND SHALL BE SUBMITTED PRIOR TO THE START OF ERECTION.
- CONNECT ALL TRUSSES TO SUPPORTING MEMBERS WITH ONE (1) SIMPSON H1 ANCHOR AND TWO (2) 3"x0.148"øx0.312"ø TOE NAILS, UNLESS NOTES OTHERWISE.
- DOUBLE TRUSSES UNDER MECHANICAL UNITS, UNLESS NOTED OTHERWISE.



BID & PERMIT SET  
KLAMATH COMMUNITY COLLEGE  
EARLY LEARNING CENTER

PROJECT # : K-6381-24  
KLAMATH COMMUNITY COLLEGE  
7380 S. 6TH AVE.  
KLAMATH FALLS, OR 97603

SHEET TITLE:  
STRUCTURAL  
GENERAL NOTES

REVISIONS:  
#      DESCRP.      DATE

ISSUE DATE: 08/01/2025

GENERAL - SPECIAL INSPECTIONS					
SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	FREQUENCY (NOTE 6)		REMARKS
			CONTINUOUS	PERIODIC	
FABRICATORS	1705.11 1704.2.5				SPECIAL INSPECTION IS REQUIRED FOR STRUCTURAL LOAD-BEARING MEMBERS AND ASSEMBLIES FABRICATED ON THE PREMISES OF A FABRICATOR'S SHOP. SPECIAL INSPECTIONS SHALL BE PERFORMED DURING FABRICATION. PERFORMING SPECIAL INSPECTIONS IS NOT REQUIRED, WHERE FABRICATOR HAS BEEN APPROVED AS AN APPROVED FABRICATOR, PER SECTION 1704.2.5.1.
DEFERRED SUBMITTALS				X	SPECIAL INSPECTION REQUIREMENTS FOR DEFERRED SUBMITTAL ITEMS, INCLUDING REQUIREMENTS FOR DESIGNATED SEISMIC SYSTEMS IN ACCORDANCE WITH OSSC SECTION 1705.13.4 IF APPLICABLE, TO BE SPECIFIED BY THE SYSTEM ENGINEER AND INCLUDED WITH DEFERRED SUBMITAL DOCUMENTS.
SUBMITTALS TO THE BUILDING OFFICIAL	1704.5			X	CERTIFICATES OF COMPLIANCE, REPORTS OF PRE-CONSTRUCTION TESTS, OR REPORTS OF MATERIAL PROPERTIES SHALL BE SUBMITTED TO THE BUILDING OFFICIAL.
POST INSTALLED MECHANICAL ANCHORS AND ADHESIVE ANCHORS (EXCLUDING CONDITIONS NOTED ABOVE) IN HARDENED CONCRETE AND COMPLETED MASONRY	1705.3	ACI 318: 17.2.5		X	
WIND RESISTING COMPONENTS - SPECIAL INSPECTIONS					
ROOF COVERING, ROOF DECK AND ROOF FRAMING CONNECTIONS	1705.12.3			X	
EXTERIOR WALL COVERING AND WALL CONNECTIONS TO ROOF AND FLOOR DIAPHRAGMS AND FRAMING	1705.12.3			X	

SOILS/GEOTECHNICAL - SPECIAL INSPECTIONS					
SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	FREQUENCY (NOTE 6)		REMARKS
			CONTINUOUS	PERIODIC	
SOILS					
VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY	1705.6	GEOTECHNICAL REPORT		X	BY THE GEOTECHNICAL ENGINEER OR QUALIFIED SPECIAL INSPECTOR
VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL				X	
PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS				X	
DURING FILL PLACEMENT, VERIFY USE OF PROPER MATERIALS AND PROCEDURES IN ACCORDANCE WITH THE PROVISIONS OF THE APPROVED GEOTECHNICAL REPORT. VERIFY DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.			X		
PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY				X	

SOILS/GEOTECHNICAL - TESTING					
SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	FREQUENCY (NOTE 6)		REMARKS
			CONTINUOUS	PERIODIC	
FILL IN-PLACE DENSITY OR PREPARED SUBGRADE DENSITY	1705.6	VARIES; GEOTECHNICAL REPORT OR MINIMUM PER OSSC 1803.5.5, 1803.5.8, WHICHEVER IS GREATER		X	BY THE GEOTECHNICAL ENGINEER OR QUALIFIED SPECIAL INSPECTOR
MATERIAL VERIFICATION		VARIES; CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS		X	BY THE GEOTECHNICAL ENGINEER OR QUALIFIED SPECIAL INSPECTOR
TEST ELEMENTS	1705.6 1705.7		REFERENCE SPECIFICATIONS FOR PERFORMANCE VERIFICATION AND PROOF LOAD TESTING REQUIREMENTS		BY THE GEOTECHNICAL ENGINEER

CONCRETE - SPECIAL INSPECTIONS					
SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	FREQUENCY (NOTE 6)		REMARKS
			CONTINUOUS	PERIODIC	
GENERAL	1705.3 1901.6	ACI 318: 26.13			SPECIAL INSPECTIONS OF CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1705.3 OF THE IBC AND SECTION 26.13 OF ACI 318.
REINFORCING STEEL	1901.5	ACI 318: CH. 20, 25.2, 25.3, 26.6.1-26.6.3		X	REINFORCING TO COMPLY WITH ALL CODE PROTECTION, SPACING AND TOLERANCE LIMITS.
INSPECT ANCHORS/BOLTS CAST IN CONCRETE	1705.3	ACI 318: 17.2.5 26.13		X	ALL CAST-IN-PLACE ANCHORS/BOLTS SHALL BE VISUALLY INSPECTED. REFERENCE STEEL INSPECTIONS FOR ADDITIONAL INSTALLATION, MATERIAL AND WELDING INSPECTIONS OF STEEL ITEMS EMBEDDED IN CONCRETE (HEADED STUDS, DBA'S, ETC.)
VERIFYING USE OF REQUIRED MIX DESIGN(S)	1904.1 1904.2 1908	ACI 318: CH. 19, 26.4.3, 26.4.4		X	
CONCRETE SPECIMENS FOR TESTING		ASTM C172 ASTM C31 ACI 318: 26.5, 26.12	X		PRIOR TO CONCRETE PLACEMENT, FABRICATE CONCRETE SPECIMENS FOR TESTING. SEE THE CONCRETE TESTING TABLE FOR ADDITIONAL INFORMATION.
VERIFICATION OF FORMWORK		ACI 318: 26.11.1.2(b), 26.13.3.3		X	SPECIAL INSPECTIONS APPLY TO SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED
EMBEDDED ITEMS IN CONCRETE				X	ALL NON-STRUCTURAL EMBEDDED ITEMS, SUCH AS CONDUITS, PIPES AND SLEEVES, SHALL BE REVIEWED FOR CONFORMANCE WITH STRUCTURAL DOCUMENTS FOR SIZE, SPACING, LOCATION, EDGE DISTANCE AND TRIM REINFORCING.
REINFORCING STEEL MECHANICAL COUPLERS, TERMINATORS AND FORMSAVERS		ICC EVALUATION REPORTS		X	

CONCRETE - TESTING				
SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	FREQUENCY (NOTE 6)	REMARKS
CONCRETE STRENGTH	1705.3 ASTM C172 ASTM C 31 ACI 318 26.12, ACI 318 26.5	ASTM C39	EACH 150 CY NOR LESS THAN EACH 5000 SF OF SLAB OR WALL PLACED EACH SHIFT	FABRICATE SPECIMENS AT TIME FRESH CONCRETE IS PLACED
CONCRETE SLUMP		ASTM C143		
CONCRETE AIR CONTENT		ASTM C231		
CONCRETE TEMPERATURE		ASTM C1064		

MASONRY MINIMUM VERIFICATION REQUIREMENTS					
MINIMUM VERIFICATION	REQUIRED FOR QUALITY ASSURANCE (a)			CODE REFERENCE	REMARKS
	QUALITY ASSURANCE LEVEL 1	QUALITY ASSURANCE LEVEL 2	QUALITY ASSURANCE LEVEL 3	TMS 602-16 TABLE 3 (S-25)	
PRIOR TO CONSTRUCTION, VERIFICATION OF COMPLIANCE OF SUBMITTALS	R	R	R	ART. 1.5	
PRIOR TO CONSTRUCTION, VERIFICATION OF 1m AND fACC, EXCEPT WHERE SPECIFICALLY EXEMPTED BY THE CODE	NR	R	R	ART. 1.4 B	
DURING CONSTRUCTION, VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) WHEN SELF-CONSOLIDATING GROUT IS DELIVERED TO THE PROJECT SITE	NR	R	R	ART. 1.5 AND 1.6.3	
DURING CONSTRUCTION, VERIFICATION OF 1m AND fACC FOR EVERY 5,000 SQ. FT.	NR	NR	R	ART. 1.4 B	
DURING CONSTRUCTION, VERIFICATION OF PROPORTIONS OF MATERIALS AS DELIVERED TO THE PROJECT SITE FOR PREMIXED OR PREBLENDED MORTAR, PRESTRESSING GROUT, AND GROUT OTHER THAN SELF-CONSOLIDATING GROUT	NR	NR	R	ART. 1.4 B	

NOTE: R=REQUIRED, NR=NOT REQUIRED (SEE NOTE 6)

PROJECT MASONRY INSPECTIONS TO BE PERFORMED FOR QUALITY ASSURANCE LEVEL 2

PIVOT ARCHITECTURE

REGISTERED PROFESSIONAL ENGINEER  
70889  
MATTHEW R. SMITH  
JULY 08, 2014

EXPIRES: 06-30-26

BID & PERMIT SET

KLAMATH COMMUNITY COLLEGE  
EARLY LEARNING CENTER

PROJECT # : K-6381-24  
KLAMATH COMMUNITY COLLEGE  
7390 S. 6TH AVE.  
KLAMATH FALLS, OR 97603

SHEET TITLE:

SPECIAL INSPECTIONS AND TESTING

REVISIONS:

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S003



MASONRY MINIMUM SPECIAL INSPECTION REQUIREMENTS						
INSPECTION TASK	FREQUENCY			CODE REFERENCE		REMARKS
	QUALITY ASSURANCE LEVEL 1	QUALITY ASSURANCE LEVEL 2	QUALITY ASSURANCE LEVEL 3	TMS 402-16	TMS 602-16 TABLE 4 (S-26)	
1. AS MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:						
A. PROPORTIONS OF SITE-PREPARED MORTAR.	NR	P	P		ART. 2.1, 2.6 A & 2.6 C	
C. GRADE, TYPE, AND SIZE OF REINFORCEMENT AND ANCHOR BOLTS, AND PRESTRESSING TENDONS AND ANCHORAGES	NR	P	P		ART. 3.4 & 3.6 A	
2. PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:						
A. GROUT SPACE.	NR	P	C		ART. 3.2 D & 3.2 F	
C. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND ANCHOR BOLTS.	NR	P	C	SEC. 6.1, 6.3.1, 6.3.6, & 6.3.7	ART. 3.2 E & 3.4	REFERENCE THE GENERAL TABLE FOR SPECIAL INSPECTION REQUIRED FOR POST INSTALLED ANCHORS INTO COMPLETED MASONRY
D. PROPORTIONS OF SITE-PREREPARED GROUT	NR	P	P		ART. 2.6 B & 2.4 G.1.b	
3. VERIFY COMPLIANCE OF THE FOLLOWING DURING CONSTRUCTION.						
A. MATERIALS AND PROCEDURES WITH THE APPROVED SUBMITTALS	NR	P	P		ART. 1.5	
B. PLACEMENT OF MASONRY UNITS AND MORTAR JOINT CONSTRUCTION	NR	P	P		ART. 3.3 B	
C. SIZE AND LOCATION OF STRUCTURAL MEMBERS	NR	P	P		ART. 3.3 F	
D. TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION	NR	P	C	SEC. 1.2.1 (e), 6.2.1, & 6.3.1		
E. WELDING OF REINFORCEMENT	NR	C	C	SEC. 6.1.6.1.2		
F. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F) OR HOT WEATHER (TEMPERATURE ABOVE 90°F)	NR	P	P		ART. 1.8 C & 1.8 D	
H. PLACEMENT OF GROUT	NR	C	C		ART 3.5 & 3.6 C	
4. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS	NR	P	C		ART. 1.4 B.2.a.3, 1.4 B.2.b.3, 1.4 B.2.c.3, 1.4 B.3, & 1.4 B.4	
NOTE: R=REQUIRED, NR=NOT REQUIRED, P=PERIODIC, C=CONTINUOUS (NOTE 6)						
(a) FREQUENCY REFERS TO THE FREQUENCY OF INSPECTION, WHICH MAY BE CONTINUOUS DURING THE LISTED TASK OR PERIODICALLY DURING THE LISTED TASK, AS DEFINED IN THE TABLE.						
(b) REQUIRED FOR THE FIRST 5,000 SQUARE FEET (465 SQUARE METERS) OF AAC MASONRY.						
(c) REQUIRED AFTER THE FIRST 5,000 SQUARE FEET (465 SQUARE METERS) OF AAC MASONRY.						
PROJECT MASONRY INSPECTIONS TO BE PERFORMED FOR QUALITY ASSURANCE LEVEL 2						

MASONRY - REQUIRED TESTING		
SYSTEM OR MATERIAL	IBC CODE REFERENCE OR REFERENCED STANDARD	REMARKS
UNIT STRENGTH METHOD - MASONRY UNIT	IBC 1705.4 , TMS 602 1.4.B, TMS 602.2.3, TMS 602 2.3 E, ASTM REFERENCES PER REMARKS	CONCRETE MASONRY (ASTM C90) CONCRETE MASONRY UNIT MATERIALS TO CONFORM TO ASTM C55, C73, C90, C129, C744, OR C1634 AS SPECIFIED. CLAY MASONRY (ASTM C62, ASTM C216, OR ASTM C652), CLAY OR SHALE MASONRY UNITS TO CONFORM TO ASTM C34, C56, C62, C126, C212, C216, C652, C1088, OR C1405 OR TO ANSI A 137.1 AS SPECIFIED. AAC MASONRY UNITS TO CONFORM TO ASTM C1691 AND ASTM C1693.
UNIT STRENGTH METHOD-COMPRESSIVE STRENGTH OF GROUT	IBC 1705.4, TMS 602 2.2, ASTM C476	GROUT SHALL COMPLY WITH ASTM C476. WHEN f'm EXCEEDS 2,000 PSI PROVIDE GROUT COMPRESSIVE STRENGTH THAT EQUALS OR EXCEEDS f'm. DETERMINE COMPRESSIVE STRENGTH OF GROUT IN ACCORDANCE WITH ASTM C1019. DO NOT USE ADMIXTURES UNLESS ACCEPTABLE. FIELD ADDITION OF ADMIXTURES IS NOT PERMITTED IN SELF-CONSOLIDATING GROUT.
PRISM TEST METHOD	IBC 1705.4, TMS 602 1.4 B.2.3, 1.4 B.4, ASTM C1314	FOR BOTH CLAY AND CONCRETE MASONRY, TEST IN ACCORDANCE WITH ASTM C1314. DETERMINE LENGTH, WIDTH AND HEIGHT DIMENSIONS OF THE PRISM AND TEST PRISMS WHEN AT LEAST 28 DAYS OLD IN ACCORDANCE WITH ASTM C1314.
GENERAL NOTES: 1.4 B.1 OR 1.4 B.2 EITHER THE UNIT STRENGTH METHOD OR THE PRISM TEST METHOD MAY BE CHOSEN TO SATISFY SPECIAL INSPECTIONS COMPRESSIVE STRENGTH FOR EACH WYTHE.		

STEEL - SPECIAL INSPECTIONS					
SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	INSPECTION (NOTES 5 AND 6)		REMARKS
			CONTINUOUS/ PERFORM	PERIODIC/ OBSERVE	
CONTRACTOR QUALITY CONTROL REQUIREMENTS		AISC 360 CHAPTER N	X	X	CONTRACTOR TO PROVIDE QUALITY CONTROL FOR ALL ITEMS INDICATED TO BE OBSERVED AND/OR PERFORMED IN TABLE BELOW
STEEL FABRICATION					
FABRICATION OF STRUCTURAL ELEMENTS	1704.2.5	AISC 360		X	REFER TO INSPECTION OF FABRICATOR REQUIREMENTS
MATERIAL VERIFICATION OF STRUCTURAL STEEL COMPONENTS	1705.2.1 TABLE 1705.2-3	ASTM A6 ASTM STANDARDS SPECIFIED IN CONSTRUCTION DOCUMENTS AISC 360 A3.1 AISC 360 N2.1		X	CERTIFIED MILL TEST REPORTS
MATERIAL VERIFICATION OF ANCHOR BOLTS AND THREADED RODS		AISC 360 A3.4 AISC 360 N3.2 ASTM STANDARDS SPECIFIED IN CONSTRUCTION DOCUMENTS		X	MANUFACTURER'S CERTIFIED TEST REPORTS
MATERIAL VERIFICATION OF WELD FILLER METALS	1705.2.1.1 TABLE 1705.2-5	AISC 360 A3.5 AISC 360 N3.2 APPLICABLE AWS A5 DOCUMENTS		X	MANUFACTURER'S CERTIFIED TEST REPORTS
STRUCTURAL STEEL WELDING					
VERIFYING USE OF PROPER WPS'S	1705.2.1 AWS D1.1	AISC 360 N3.2			RETAIN A RECORD OF WELDING PROCEDURE SPECIFICATIONS
VERIFYING WELDER QUALIFICATIONS	1705.2	AWS D1.1		X	RETAIN A RECORD OF QUALIFICATION CARDS
SINGLE PASS FILLET WELDS LESS THAN OR EQUAL TO 5/16"	TABLE 1705.2-6a	AWS D1.1 CLAUSE 6		X	

WOOD - SPECIAL INSPECTIONS					
SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	FREQUENCY (NOTE 6)		REMARKS
			CONTINUOUS	PERIODIC	
WOOD - REQUIRED STRUCTURAL SPECIAL INSPECTIONS					
FABRICATION OF PREFABRICATED STRUCTURAL ELEMENTS	1705.5			X	REFER TO INSPECTION OF FABRICATOR REQUIREMENTS
PREFABRICATED WOOD SHEAR PANELS	1705.5 1704.2.5	ICC EVALUATION REPORT		X	SPECIAL INSPECTIONS APPLY TO HOLDOWN ANCHOR SIZE AND PLACEMENT, INCLUDING EMBEDMENT LENGTH, SPACING, AND EDGE DISTANCE
WOOD - REQUIRED SEISMIC RESISTANCE INSPECTIONS					
CONNECTIONS FOR DIAPHRAGM CHORDS, COLLECTORS, BRACING, AND SHEAR WALL ANCHORAGE AND HOLDOWNS	1705.13.2.2			X	ALL FASTENERS/CONNECTIONS VISUALLY INSPECTED
FASTENING OF DIAPHRAGM AND SHEAR WALL SHEATHING WITH EDGE NAILING ≤ 4"	1705.13.2			X	FOR WOOD SHEAR WALLS, SHEAR PANELS, AND DIAPHRAGMS. THIS INCLUDES NAILING, BOLTING, ANCHORING AND OTHER FASTENING TO OTHER COMPONENTS IN THE SEISMIC FORCE RESISTING SYSTEM
WOOD - REQUIRED WIND RESISTANCE INSPECTIONS					
NAILING, BOLTING, ANCHORING, AND OTHER FASTENING OF COMPONENTS WITHIN THE MAIN WIND-FORCE RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, WOOD DIAPHRAGMS, COLLECTORS, BRACES, AND HOLDOWNS	1705.12.1			X	SPECIAL INSPECTIONS ARE NOT REQUIRED FOR WOOD SHEAR WALLS, SHEAR PANELS AND DIAPHRAGMS WHERE THE LATERAL RESISTANCE IS PROVIDED BY STRUCTURAL SHEATHING AND FASTENER SPACING IS MORE THAN 4 INCHES ON CENTER. REFER TO TABLE 2 FOR MATERIAL SPECIFIC REQUIREMENTS

REQUIRED SPECIAL INSPECTIONS for SEISMIC RESISTANCE					
SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	FREQUENCY (NOTE 6)		REMARKS
			CONTINUOUS	PERIODIC	
ARCHITECTURAL COMPONENTS					
ERECTION AND FASTENING OF EXTERIOR CLADDING, INTERIOR AND EXTERIOR NONBEARING WALLS AND INTERIOR AND EXTERIOR VENEER 30 FT AND GREATER IN HEIGHT ABOVE GRADE OR WALKING SURFACE	1705.13.5			X	
ERECTION AND FASTENING OF EXTERIOR CLADDING AND INTERIOR AND EXTERIOR VENEER WEIGHING MORE THAN 5 PSF.	1705.13.5			X	
ANCHORAGE OF SUSPENDED CEILING SYSTEMS		ASCE 7-16 SECTION 13.5.6		X	REQUIRED PER STATE OF OREGON BUILDING CODES DIVISION INTERPRETATIONS
INSTALLATION OF OTHER SEISMIC SUPPORTS FOR ARCHITECTURAL SYSTEMS AND THEIR COMPONENTS	1705.13.5			X	
PLUMBING MECHANICAL COMPONENTS					
INSTALLATION OF PIPING SYSTEM MEANT TO CARRY HAZARDOUS MATERIALS AND THEIR ASSOCIATED MECHANICAL UNITS	1705.13.6			X	
INSTALLATION OF OTHER SEISMIC SUPPORTS FOR MECHANICAL SYSTEMS AND THEIR COMPONENTS				X	
INSTALLATION OF MECHANICAL EQUIPMENT, INCLUDING DUCT WORK, PIPING AND THEIR STRUCTURAL SUPPORTS WHERE AUTOMATIC FIRE SPRINKLER SYSTEMS ARE INSTALLED				X	OSSC 1705.13.6 CHECKING FOR MINIMUM CLEARANCES. FLEXIBLE SPRINKLER HOSE FITTINGS ARE EXEMPT FROM THIS REQUIREMENT

STATEMENT OF SPECIAL INSPECTION NOTES:

1.

INSPECTIONS SHALL CONFORM TO SECTION 1705 OF THE 2022 OSSC, CONTRACT DOCUMENTS AND APPROVED SUBMITTALS. REFER TO SPECIAL INSPECTION AND TESTING TABLES FOR PROJECT REQUIREMENTS.
2.

SPECIAL INSPECTIONS AND ASSOCIATED TESTING SHALL BE PERFORMED BY AN APPROVED ACCREDITED INDEPENDENT AGENCY MEETING THE REQUIREMENTS OF ASTM E329 (MATERIALS). THE INSPECTION AND TESTING AGENCY SHALL FURNISH TO THE STRUCTURAL ENGINEER ARCHITECT A COPY OF THEIR SCOPE OF ACCREDITATION. SPECIAL INSPECTORS SHALL BE APPROVED BY THE BUILDING OFFICIAL. WELDING INSPECTORS SHALL BE QUALIFIED PER SECTION 6.1.4.1(1) OF AWS D1.1.
3.

THE SPECIAL INSPECTOR SHALL OBSERVE THE INDICATED WORK FOR COMPLIANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS. ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION AND NOTED IN THE INSPECTION REPORTS.
4.

THE SPECIAL INSPECTOR AND GEOTECHNICAL ENGINEER SHALL FURNISH INSPECTION REPORTS FOR EACH INSPECTION TO THE BUILDING OFFICIAL, STRUCTURAL ENGINEER, ARCHITECT, CONTRACTOR, AND OWNER. THE SPECIAL INSPECTION AGENCY SHALL SUBMIT A FINAL REPORT STATING THAT THE WORK REQUIRING SPECIAL INSPECTION WAS INSPECTED AND IS IN CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS AND THAT ALL DISCREPANCIES NOTED IN THE INSPECTION REPORTS HAVE BEEN CORRECTED.
5.

QUALITY ASSURANCE (QA) IS REQUIRED FOR STRUCTURAL STEEL ITEMS PER AISC 360 AND 341 UNLESS SPECIFICALLY NOTED OTHERWISE. QUALITY CONTROL (QC) TO BE PROVIDED BY THE FABRICATOR, ERECTOR OR OTHER RESPONSIBLE CONTRACTOR AS APPLICABLE. CONTRACTOR AND SPECIAL INSPECTOR TO DOCUMENT QUALITY CONTROL AS REQUIRED IN AISC 360 SECTION N3 AND AISC 341 SECTION J2.
6.

INSPECTION TYPES:

•

CONTINUOUS : THE FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED.

•

PERIODIC : THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK.

•

OBSERVE : OBSERVE THESE FUNCTIONS ON A RANDOM, DAILY BASIS. OPERATIONS NEED NOT BE DELAYED PENDING OBSERVATIONS.

•

PERFORM : INSPECTIONS SHALL BE PERFORMED PRIOR TO THE FINAL ACCEPTANCE OF THE ITEM.
7.

PERFORM INSPECTION PRIOR TO FINAL ACCEPTANCE OF THE ITEM FOR TEN WELDS TO BE MADE BY A GIVEN WELDER, WITH THE WELDER DEMONSTRATING UNDERSTANDING OF REQUIREMENTS AND POSSESSION OF SKILLS AND TOOLS TO VERIFY THESE ITEMS, THE PERFORM DESIGNATION OF THIS TASK SHALL BE REDUCED TO OBSERVE, AND THE WELDER SHALL PERFORM THIS TASK SHOULD THE INSPECTOR DETERMINE THAT THE WELDER HAS DISCONTINUED PERFORMANCE OF THIS TASK. THE TASK SHALL BE RETURNED TO PERFORM UNTIL SUCH TIME AS THE INSPECTOR HAS RE-ESTABLISHED ADEQUATE ASSURANCE THAT THE WELDER WILL PERFORM THE INSPECTION TASKS LISTED.
8.

SPECIAL INSPECTION OF MECHANICAL POST INSTALLED ANCHORS SHALL BE IN STRICT CONFORMANCE WITH THE ICC REPORT AND MANUFACTURER'S INSTALLATION REQUIREMENTS. ANCHOR INSTALLERS SHALL BE QUALIFIED AS REQUIRED BY JURISDICTION REQUIREMENTS.

•

INSPECTION REPORTS SHALL IDENTIFY NAMES OF INSTALLERS.

•

SPECIAL INSPECTOR SHALL PROVIDE DOCUMENTATION AT THE END OF ANCHOR INSTALLATIONS STATING THAT THE ANCHORS WERE INSPECTED PER APPROVED ANCHOR EVALUATION REPORT.

9.

TESTING ABBREVIATIONS:

A.

NDT - NON-DESTRUCTIVE TESTING

B.

C.J.P. - COMPLETE JOINT PENETRATION

C.

MT- MAGNETIC PARTICLE TESTING

D.

RBS - REDUCED BEAM SECTION

10.

DOCUMENT (D): INDICATES CONTRACTOR AND SPECIAL INSPECTOR TO PROVIDE DOCUMENTATION IN ACCORDANCE WITH AISC 341.

CONTRACTOR RESPONSIBILITY:

EACH CONTRACTOR RESPONSIBLE FOR THE CONSTRUCTION OF THE MAIN WIND-OR SEISMIC-FORCE-RESISTING SYSTEM, DESIGNATED SEISMIC SYSTEM OR A WIND-OR SEISMIC-RESISTING COMPONENT LISTED THE TABLES SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND THE OWNER PRIOR TO THE COMMENCEMENT OF WORK ON THE SYSTEM OR COMPONENT. THE CONTRACTOR'S STATEMENT OF RESPONSIBILITY SHALL CONTAIN THE FOLLOWING:

1.

ACKNOWLEDGEMENT THAT CONTROL WILL BE EXERCISED TO OBTAIN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS APPROVED BY THE BUILDING OFFICIAL.

2.

PROCEDURES FOR EXERCISING CONTROL WITHIN THE CONTRACTOR'S ORGANIZATION , THE METHOD AND FREQUENCY OF REPORTING AND DISTRIBUTION OF THE REPORTS.

3.

IDENTIFICATION AND QUALIFICATIONS OF THE PERSON(S) EXERCISING SUCH CONTROL AND THEIR POSITION(S) IN THE ORGANIZATION.

PIVOT  
ARCHITECTURE

BID & PERMIT SET  
KLAMATH COMMUNITY COLLEGE  
EARLY LEARNING CENTER

PROJECT #: K-6381-24  
KLAMATH COMMUNITY COLLEGE  
7380 S. 6TH AVE.  
KLAMATH FALLS, OR 97603

SHEET TITLE:

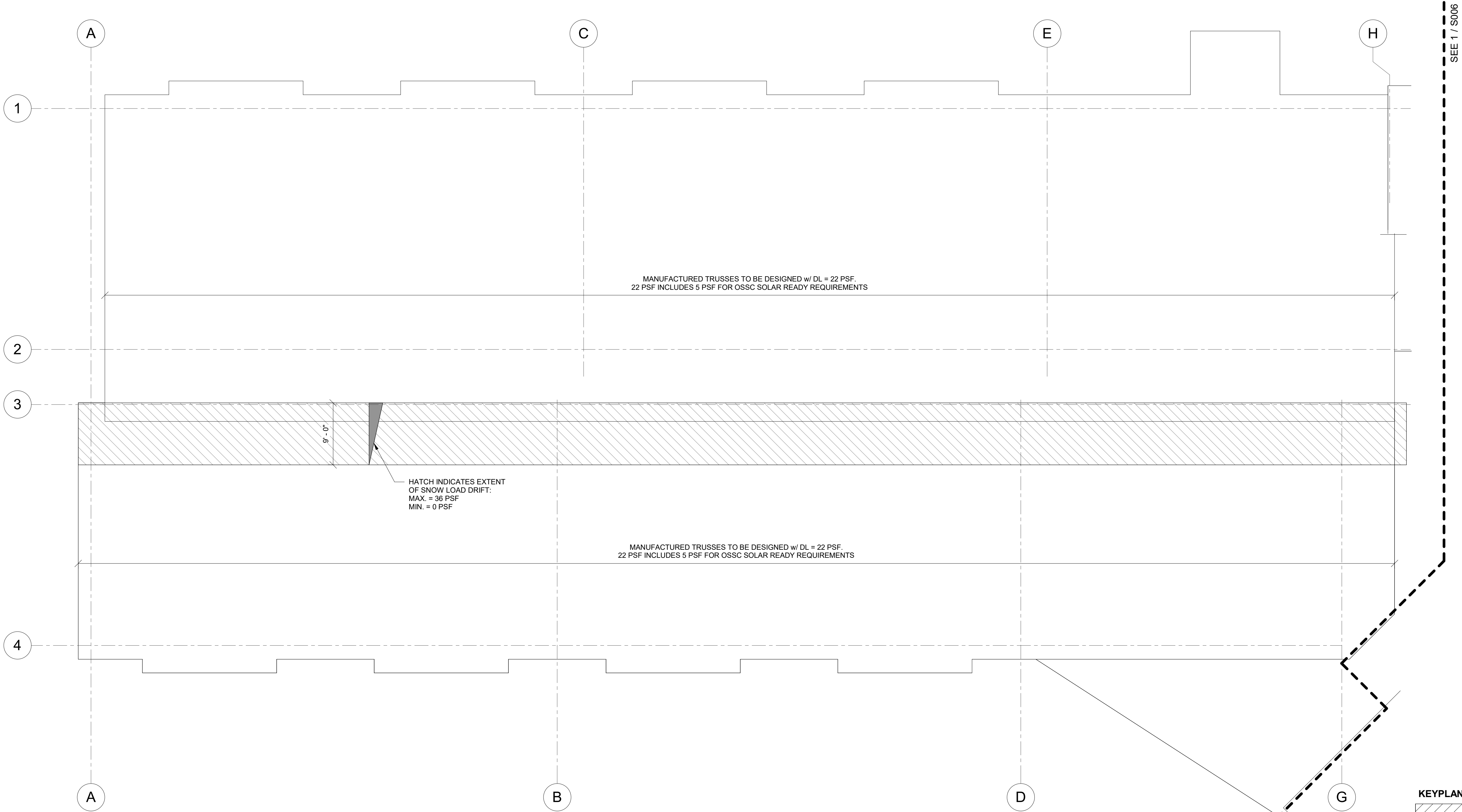
SPECIAL  
INSPECTIONS  
AND TESTING

REVISIONS:

#      DESCRP.      DATE

ISSUE DATE: 08/01/2025

S004



1  
S005 ROOF LOADING PLAN - SNOW LOADING SECTOR A  
1/8" = 1'-0"

ROOF LOADING PLAN NOTES

- A. ALL ROOF AREAS SHALL BE DESIGNED FOR A MINIMUM UNIFORM SNOW LOAD OF  $P_u = 27.0$  PSF PER STRUCTURAL GENERAL NOTES.
- B. DRIFT SNOW LOADS AND SLIDING SNOW LOADS INDICATED ON THIS PLAN ARE IN ADDITION TO UNIFORM SNOW LOAD OF  $PF = 14.6$  PSF PER ASCE 7-16 SECTION 7.4. THIS LOADING SHALL BE ANALYZED AS A SEPARATE LOAD CASE FROM THE MINIMUM BALANCED SNOW LOAD INDICATED IN NOTE A.
- C. AREAS INDICATED ON PLAN ARE DESIGNED w/ ADDITIONAL COLLATERAL  $DL = 5$  PSF FOR SOLAR READY ZONE AS REQUIRED PER SECTION 3111.4 OF OSSC. OVERHANGS ARE EXCLUDED FROM SOLAR READY ZONE.



BID & PERMIT SET  
KLAMATH COMMUNITY COLLEGE  
EARLY LEARNING CENTER

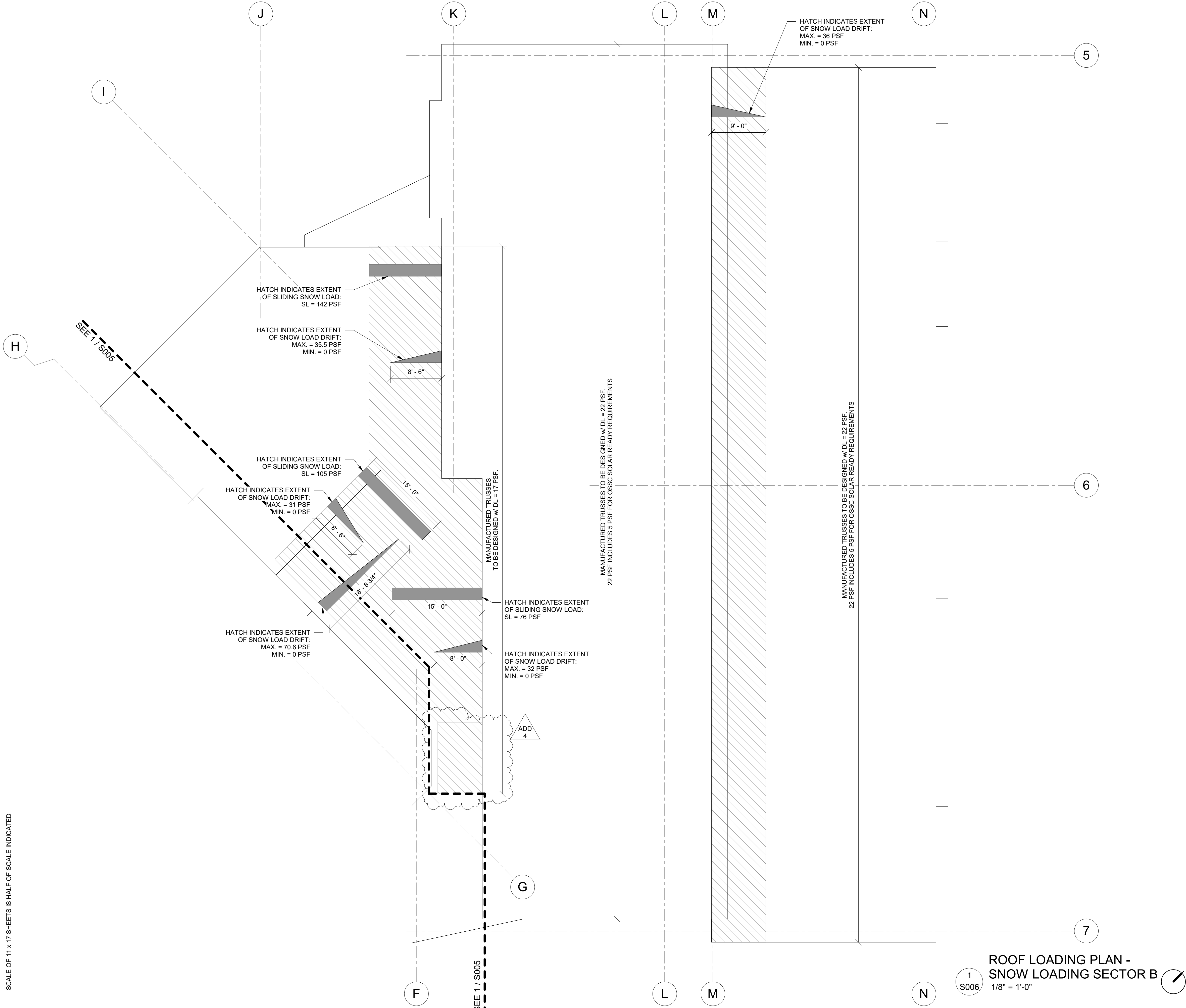
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KLAMATH COMMUNITY COLLEGE  
7390 S. 6TH AVE.  
KLAMATH FALLS, OR 97603

SHEET TITLE:  
ROOF LOADING PLAN - SNOW LOADING SECTOR A

REVISIONS:		
#	DESCRP.	DATE

ISSUE DATE: 08/01/2025

S005



- ROOF LOADING PLAN NOTES
- A.

ALL ROOF AREAS SHALL BE DESIGNED FOR A MINIMUM UNIFORM SNOW LOAD OF  $P_u = 27.0$  PSF PER STRUCTURAL GENERAL NOTES.
- B.

DRIFT SNOW LOADS AND SLIDING SNOW LOADS INDICATED ON THIS PLAN ARE IN ADDITION TO UNIFORM SNOW LOAD OF  $P_u = 14.6$  PSF PER ASCE 7-16 SECTION 7.4. THIS LOADING SHALL BE ANALYZED AS A SEPARATE LOAD CASE FROM THE MINIMUM BALANCED SNOW LOAD INDICATED IN NOTE A.
- C.

AREAS INDICATED ON PLAN ARE DESIGNED w/ ADDITIONAL COLLATERAL  $DL = 5$  PSF FOR SOLAR READY ZONE AS REQUIRED PER SECTION 3111.4 OF OSSC. OVERHANGS ARE EXCLUDED FROM SOLAR READY ZONE.

PIVOT

ARCHITECTURE

REGISTERED PROFESSIONAL ENGINEER

70989

MATTHEW R. SMITH

EXPIRES: 06-30-26

ZCS

BID & PERMIT SET

KLAMATH COMMUNITY COLLEGE  
EARLY LEARNING CENTER

PROJECT # : K-6381-24  
KLAMATH COMMUNITY COLLEGE  
7390 S. 6TH AVE.  
KLAMATH FALLS, OR 97603

SHEET TITLE:

ROOF LOADING PLAN - SNOW LOADING SECTOR B

REVISIONS:

#	DESCRP.	DATE
1	ADD 4	08.29.2025

FOUNDATION SCHEDULE					
MARK	WIDTH	LENGTH	DEPTH	REINFORCEMENT	
F-1	1' - 6"	CONT.	4' - 0"	(2) #5 CONT. LONG.	
F-2	4' - 0"	CONT.	2' - 0"	(7) #5 TOP & BOTTOM & #5 TRANS. @ 12" o.c. TOP & BOTTOM	
F-3	3' - 0"	3' - 0"	4' - 0"	(3) #5 E-W	
F-4	4' - 0"	CONT.	4' - 0"	(6) #5 CONT. LONG.	
F-5	4' - 0"	4' - 0"	1' - 6"	(4) #5 E-W.	
F-6	4' - 6"	CONT.	1' - 6"	(5) #5 CONT. LONG.	
F-7	1' - 6"	CONT.	1' - 6"	(2) #5 CONT. LONG.	
F-8	1' - 0"	CONT.	2' - 0"	(2) #5 CONT. LONG.	

1. UNLESS NOTED OTHERWISE, REINFORCEMENT TO BE SPACED EVENLY IN FOOTING WITH CORRECT SIDE AND BOTTOM CLEARANCES.
2. ALL FOOTINGS TO BE PLACED ON 18" COMPACTED STRUCTURAL FILL. STRUCTURAL FILL TO CONSIST OF 3/4" MINUS TO 4" MINUS CRUSHED ROCK. THE UPPER 6" OF STRUCTURAL FILL TO CONSIST OF 3/4" MINUS.
3. FILL TO EXTEND 12" BEYOND EDGES OF FOOTINGS.
4. ALL INTERIOR SLAB ON GRADE TO BE PLACED ON 12" OF 3/4" MINUS COMPACED STRUCTURAL FILL.
5. COMPACT STRUCTURAL FILL TO 98% OF THE MAXIMUM DRY DENSITY (ASTM D-698).
6. SEE GEOTECH REPORT FOR ADDITIONAL INFORMATION.

HOLDOWN SCHEDULE						
#	SIMPSON MODEL	ALLOWABLE LOAD (lb)	POST THICKNESS	FASTENER	ANCHOR	REMARKS
1	HDU2-SDS2.5	3,075	3"	(6) 1/4 x 2 1/2" SDS	5/8"	PL 1/4x3x0'-3" w/ DBL NUT AT BOT OF THREADED ROD
2	HDU5-SDS2.5	5,645	3 1/2"	(14) 1/4 x 2 1/2" SDS	5/8"	PL 1/4x3x0'-3" w/ DBL NUT AT BOT OF THREADED ROD
3	HDU19	19,070	PER PLAN	(5) 1" DIA. STUD BOLTS	1 1/4"	PL 1/4x4x0'-4" w/ DBL NUT AT BOT OF THREADED ROD

1. NAILS ARE TO BE COMMON WIRE NAILS, U.N.O.
2. HARDWARE IS TO BE SIMPSON, U.N.O.
3. HOLDOWN HARDWARE CAN BE EXTENDED WITH A307 THREADED ROD AND COUPLER.
4. ALIGN ALL HOLDOWNS FOR THE FULL HEIGHT OF STRUCTURE.
5. ALL HARDWARE TO BE INSTALLED PER MANUFACTURE'S SPECIFICATIONS.
6. HOLDOWN ANCHOR BOLTS ARE IN ADDITION TO TYPICAL SILL PLATE ANCHOR BOLTS.
7. EXTEND THREADED ROD TO WITHIN 3" CLEAR OF BOTTOM OF FOOTING.

SIMPSON CATALOG C-2025  
UPDATED 05/30/2025

SHEAR WALL SCHEDULE									
X	SHEATHING	NAIL SIZE (LENGTH, SHANK DIA., HEAD DIA.)	EDGE NAILING (o.c.)	FIELD NAILING (o.c.)	PANEL EDGE STUDS	TOP PLATE A35 (o.c.)	BOTTOM PLATE DIMENSION	SILL PLATE ANCHORAGE (o.c.)	REMARKS
A	1/2" APA RATED, ONE SIDE	3"x0.148"x0.312"	6"	12"	2x	2'-0"	2x	5/8" DIA. @ 4'-0"	--
B	1/2" APA RATED, ONE SIDE	3"x0.148"x0.312"	4"	12"	2x	1'-6"	2x	5/8" DIA. @ 3'-6"	--
C	1/2" APA RATED, ONE SIDE	3"x0.148"x0.312"	3"	12"	(2) 2x	1'-0"	(2) 2x	5/8" DIA. @ 3'-0"	STAGGER NAILING
D	1/2" APA RATED, ONE SIDE	3"x0.148"x0.312"	2"	12"	(2) 2x	1'-0"	(2) 2x	5/8" DIA. @ 2'-6"	STAGGER NAILING
E	1/2" APA RATED, BOTH SIDES	3"x0.148"x0.312"	3"	12"	(2) 2x	N/A	(2) 2x	5/8" DIA. @ 1'-0"	STAGGER NAILING




1. ALL PLYWOOD TO BE APA RATED STRUCTURAL 1 EXTERIOR SHEATHING
2. ALL NAILS TO BE COMMON OR GALVANIZED BOX TYPE.
3. ATTACH RIM JOIST AND / OR BLOCKING TO SHEAR WALL AS INDICATED IN TABLE ABOVE.
4. ALL WALL SHEATHING TO EXTEND FULL HEIGHT OF WALL, TOP PLATE TO BOTTOM PLATE.
5. ALL SHEAR WALLS AND HOLDOWNS MUST HAVE CONTINUOUS LOAD PATH TO FOUNDATION.
6. USE WASHER PL 1/4x3x0'-3" TYPICAL AT ALL ANCHOR BOLTS.
7. WHERE TOP PLATE FASTENING IS LESS THAN 12" o.c., USE MINIMUM BLOCKING OF 2 1/2" MANUFACTURED LUMBER (MICROLLAM LVL, OR PARALLAM PSL).
8. ALL SHEAR WALLS TO BE FULLY BLOCKED U.N.O. BLOCKING TO MATCH REQUIREMENTS FOR PANEL EDGE STUDS.
9. FOR SHEAR WALLS w/ (2) ROWS OF BOTTOM PLATE NAILING, USE MIN. 1 3/4" RIM BOARD, SPACE ROWS A MIN. OF 1/2" APART, AND STAGGER NAILS.
10. FOR SHEAR WALLS w/ STUDS SPACED @ 24" o.c. MAX. INSTALL SHEATHING WITH LONG DIMENSION ACROSS STUDS.
11. ALL SHEAR WALLS TO HAVE (2) 2x AT END OF WALL, U.N.O.

HEADER SCHEDULE				
HDR-X	BEAM SIZE	POST SIZE	INTERIOR WALL KING STUD	EXTERIOR WALLS KING STUD
HDR-1	6x8 DF-L #2	(1) 2x6	(1) 2x6	(2) 2x6
HDR-2	5 1/2x7 1/2 GLB 24F-V4	(2) 2x6	(1) 2x6	(3) 2x6
HDR-3	5 1/2x9 1/2 GLB 24F-V4	(2) 2x6	(2) 2x6	(3) 2x6
HDR-4	5 1/2x16 GLB 24F-V4	PER PLAN	N/A	N/A
HDR-5	(2) 2x8 DF-L #2	(1) LVL STUDS PER PLAN	N/A	(2) LVL STUDS PER PLAN

PROVIDE POST AND KING STUDS FOR EACH HEADER U.N.O. PER PLAN

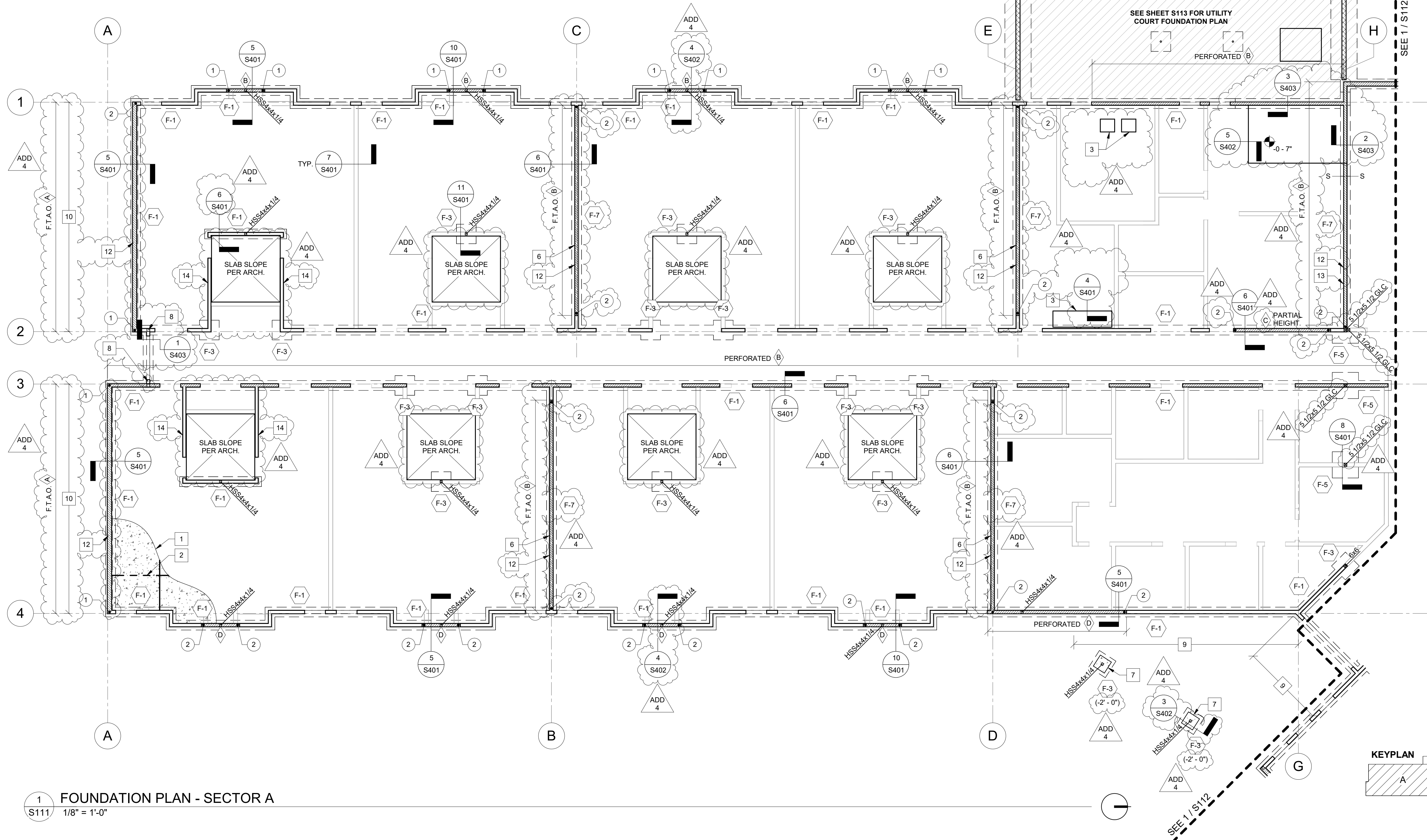


## FOUNDATION PLAN NOTES

- |    |  |    |   |    |   |  |
|----|--|----|---|----|---|--|
| A. | DIMENSIONS SHOWN ARE FOR REFERENCE ONLY. CONFIRM w/ ARCHITECTURAL PLAN & DETAILS.  | E. | ALL FOOTINGS TO BEAR OVER GRADE OVER FIRM, UNDISTURBED, NON-ORGANIC, NON-EXPANSIVE NATIVE MATERIAL, OR STRUCTURAL FILL AS REQUIRED PER GEOTECHNICAL REPORT.                           | J. |  | INDICATES 2x6 @ 16" o.c. BEARING STUD WALL ABOVE FOUNDATION w/ DOUBLE TOP PLATE AND P.T. BOTTOM PLATE, U.N.O.  |
| B. | BOTTOM OF FOOTINGS TO BE PLACED BELOW FROST DEPTH OR AS NOTED IN THE GEOTECHNICAL REPORT, WHICHEVER IS GREATER.  | F. | SEE SHEET S001 AND S002 FOR ALL NOTES.  | K. |   | ALL SHEAR WALLS INDICATED AS "PERFORATED" THE CONTRACTOR SHALL PROVIDE NAILING PATTERN AROUND ALL WALL PENETRATIONS AS CALLED OUT ON FRAMING PLANS IN CORRESPONDENCE WITH THE SHEAR WALL SCHEDULE. |
| C. | COORDINATE PENETRATIONS OF SITE UTILITIES, MECHANICAL DUCTS, PIPING, AND ELECTRICAL CONDUIT/PANELS TO MINIMIZE IMPACT TO STRUCTURAL FRAMING. PLUMBING FIXTURES SHOWN ON FLOOR FOR REFERENCE AND POSSIBLE FRAMING CONFLICTS ONLY. | G. | SEE SHEET S007 FOR ALL SCHEDULES.   | L. | F.T.A.O.  | INDICATES FORCE TRANSFER AROUND OPENING IN WALL. PROVIDE PANEL EDGE NAILING AROUND ALL WALL PENETRATIONS PER SHEAR WALL SCHEDULE. SEE ASSOCIATED ELEVATION FOR ADDITIONAL INFORMATION.             |
| D. | ALL FOOTINGS ARE TO BE CENTERED UNDER COLUMNS U.N.O.   | H. | S — S<br>INDICATES STEP IN FOOTING. SEE DETAIL 2/S402 FOR ADDITIONAL INFORMATION.   |    |   |  |
|    |  | I. |  INDICATES SHEAR WALL TYPE. SEE SHEAR WALL SCHEDULE FOR ADDITIONAL INFORMATION.                      |    |   |  |
|    |  | J. |  INDICATES SHEAR WALL LOCATION ABOVE FOUNDATION. SEE SHEAR WALL SCHEDULE FOR ADDITIONAL INFORMATION. |    |   |  |

## FOUNDATION PLAN KEYNOTES

1. 4" CONCRETE SLAB ON GRADE w/ #4 @ 18" o.c. E.W. TYP. THROUGHOUT. SEE DETAILS 1, 2, AND 3 ON S401 FOR ADDITIONAL INFORMATION.
2. INDICATES CONTROL JOINTS SPACED @ 8'-0" o.c. MAX. SEE DETAILS 1, 2, AND 3 ON S401 FOR ADDITIONAL INFORMATION.
3. 4" THICK CONCRETE HOUSEKEEPING PAD w/ #4 @ 18" o.c. E.W. VERIFY LOCATION AND SIZE w/ ARCH. AND MEP PLANS.
4. 1 1/2x7 1/4 VERSA-LAM LVL 1.8E 2400 STUDS @ 16" o.c. w/ SINGLE P.T. BOTTOM PLATE AND DOUBLE TOP PLATES
5. KING STUD EACH SIDE OF POST.
6. FULL HEIGHT 2x6 @ 16" o.c. RAKE STUD WALL w/ DOUBLE TOP PLATE AND SINGLE P.T. BOTTOM PLATE. USE 1 1/2x5 1/2 VERSA-STUD 1.7 2400 FOR STUD HEIGHTS GREATER THAN 16'-0".
7. 24"x24" CONCRETE PEDESTAL w/ (4) #5 VERT. AT EACH FACE w/ #3 HOOPS @ 10" o.c.
8. HSS5x5x1/4.
9. INDICATES APPROXIMATE EXTENT OF BRICK VENEER. STUDS TO BE @ 12" o.c.
10. FULL HEIGHT RAKE WALL w/ 1 1/2x7 1/4 VERSA-LAM LVL 1.8 2400 STUDS @ 16" o.c. w/ SINGLE P.T. BOTTOM PLATE AND DOUBLE 2x8 TOP PLATE, U.N.O. PROVIDE DOUBLE KING STUD LVL EACH SIDE OF WALL PENETRATIONS, U.N.O.
11. (3) 1 1/2x7 1/4 VERSA-LAM LVL 1.8 2400 KING STUD EACH SIDE OF WALL PENETRATION.
12. STRAP AND BLOCK ABOVE AND BELOW WALL DUCT PENETRATIONS OF F.T.A.O. SHEAR WALL. SEE DETAIL 6/S503 FOR ADDITIONAL INFORMATION.
13. FULL HEIGHT RAKE WALL w/ 1 1/2x5 1/2 VERSA-STUD 1.7 2400 STUDS @ 16" o.c.
14. BALLOON FRAME STUD WALL w/ 1 1/2x5 1/2 VERSA-STUD 1.7 2400 STUDS @ 16" o.c. w/ DOUBLE TOP PLATE AND SINGLE P.T. BOTTOM PLATE.



**BID & PERMIT SET**

**KLAMATH COMMUNITY COLLEGE**

**EARLY LEARNING CENTER**

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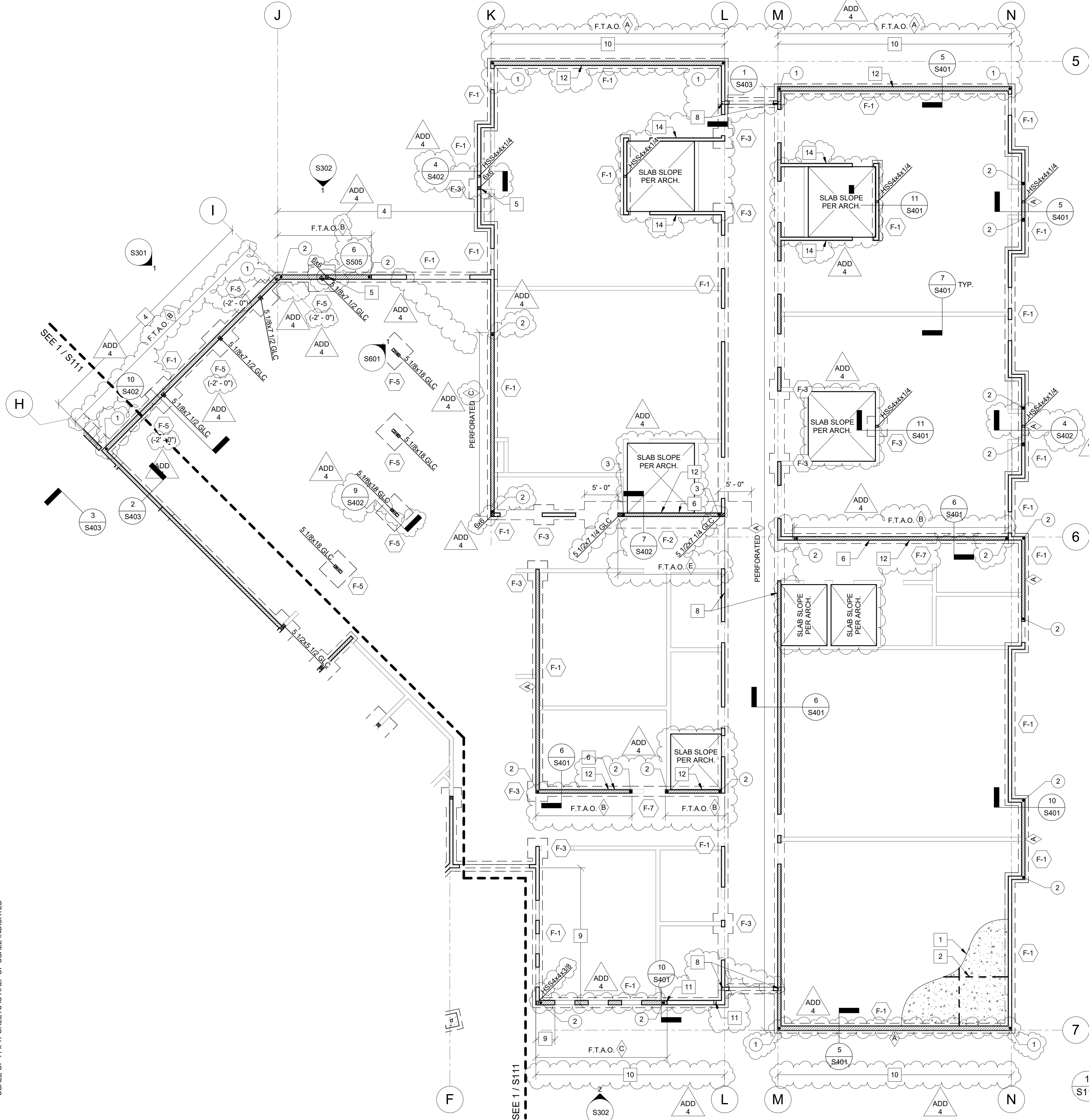
**FOUNDATION  
PLAN - SECTOR A**

REVISIONS:		
#	DESCRP.	DATE
1	ADD 4	08.29.2025

ISSUE DATE: 08/01/2025

S111





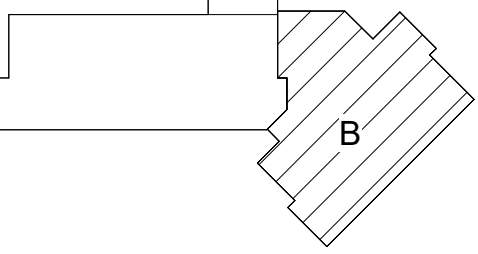
FOUNDATION PLAN NOTES

- A. DIMENSIONS SHOWN ARE FOR REFERENCE ONLY, CONFIRM W/ ARCHITECTURAL PLAN & DETAILS.
- B. BOTTOM OF FOOTINGS TO BE PLACED BELOW FROST DEPTH OR AS NOTED IN THE GEOTECHNICAL REPORT, WHICHEVER IS GREATER.
- C. COORDINATE PENETRATIONS OF SITE UTILITIES, MECHANICAL DUCTS, PIPING, AND ELECTRICAL CONDUIT/PANELS TO MINIMIZE IMPACT TO STRUCTURAL FRAMING. PLUMBING FIXTURES SHOWN ON FLOOR FOR REFERENCE AND POSSIBLE FRAMING CONFLICTS ONLY.
- D. ALL FOOTINGS ARE TO BE CENTERED UNDER COLUMNS U.N.O.
- E. ALL FOOTINGS TO BEAR OVER GRADE OVER FIRM, UNDISTURBED, NON-ORGANIC, NON-EXPANSIVE NATIVE MATERIAL, OR STRUCTURAL FILL AS REQUIRED PER GEOTECHNICAL REPORT.
- F. SEE SHEET S001 AND S002 FOR ALL NOTES.
- G. SEE SHEET S007 FOR ALL SCHEDULES.
- H. S — S INDICATES STEP IN FOOTING. SEE DETAIL 2/S402 FOR ADDITIONAL INFORMATION.
- I. INDICATES SHEAR WALL TYPE. SEE SHEAR WALL SCHEDULE FOR ADDITIONAL INFORMATION.
- J. INDICATES SHEAR WALL LOCATION ABOVE FOUNDATION. SEE SHEAR WALL SCHEDULE ON S007 FOR ADDITIONAL INFORMATION.
- K. INDICATES 2x6 @ 16" o.c. BEARING STUD WALL ABOVE FOUNDATION w/ DOUBLE TOP PLATE AND P.T. BOTTOM PLATE, U.N.O.
- L. ALL SHEAR WALLS INDICATED AS "PERFORATED" THE CONTRACTOR SHALL PROVIDE NAILING PATTERN AROUND ALL WALL PENETRATIONS AS CALLED OUT ON FRAMING PLANS IN CORRESPONDENCE WITH THE SHEAR WALL SCHEDULE.
- M. F.T.A.O. INDICATES FORCE TRANSFER AROUND OPENING IN WALL. PROVIDE PANEL EDGE NAILING AROUND ALL WALL PENETRATIONS PER SHEAR WALL SCHEDULE. SEE ASSOCIATED ELEVATION FOR ADDITIONAL INFORMATION.
- N. INDICATES HOLDOWN TYPE AND LOCATION. SEE HOLDOWN SCHEDULE FOR ADDITIONAL INFORMATION.
- O. INDICATES TOP OF SLAB ELEVATION. COORDINATE WITH ARCHITECT.
- P. INDICATES FOOTING TYPE PER FOUNDATION SCHEDULE AND TOP OF FOOTING ELEVATION.
- Q. INDICATES EXTERIOR 8" NOMINAL FULLY GROUTED CMU WALL. REINFORCEMENT PER PLAN.

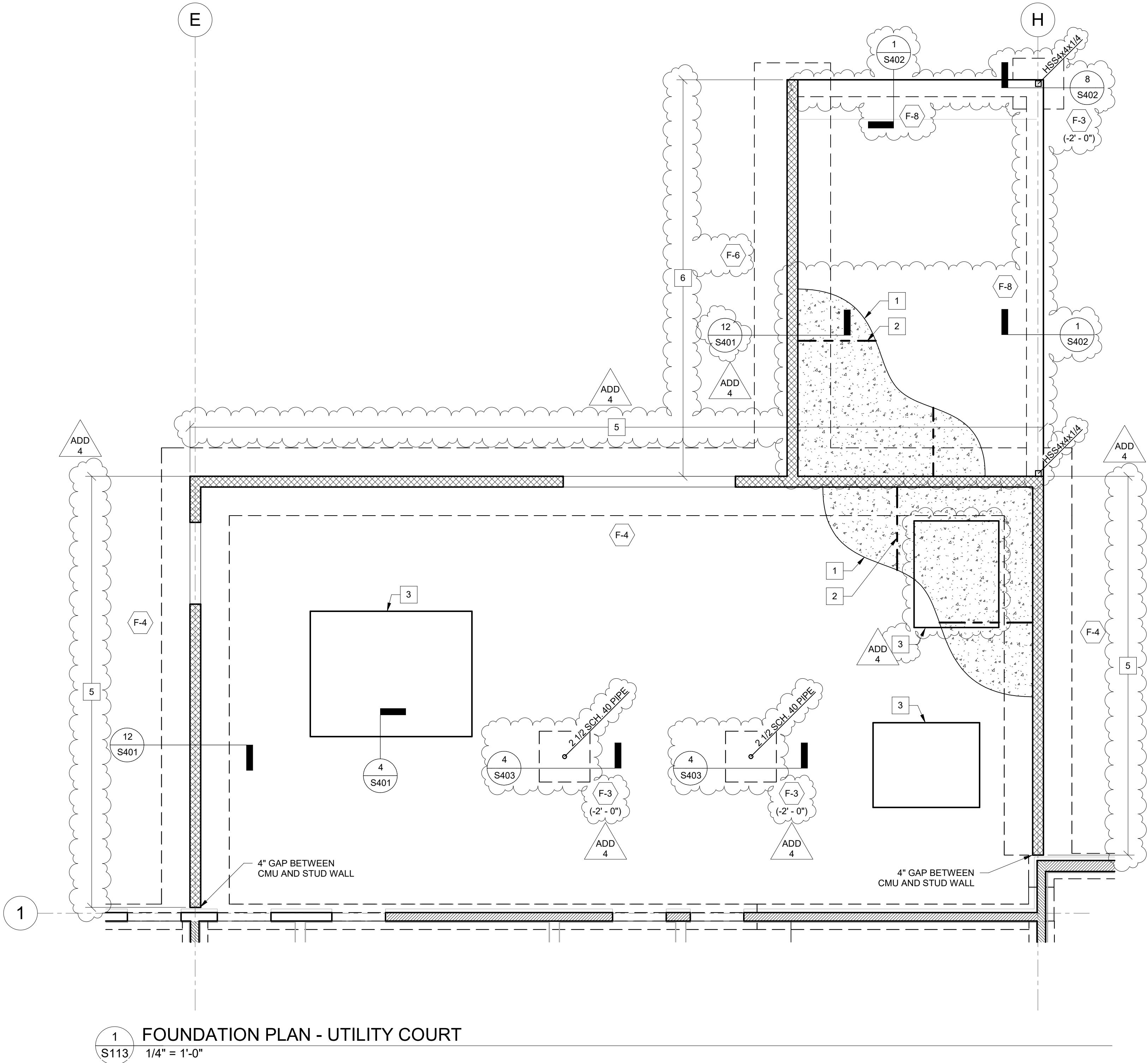
FOUNDATION PLAN KEYNOTES

1. 4" CONCRETE SLAB ON GRADE w/ #4 @ 18" o.c. E.W. TYP. THROUGHOUT. SEE DETAILS 1, 2, AND 3/S401 FOR ADDITIONAL INFORMATION.
2. INDICATES CONTROL JOINTS SPACED @ 8'-0" o.c. MAX. SEE DETAILS 1, 2, AND 3/S401 FOR ADDITIONAL INFORMATION.
3. 4" THICK CONCRETE HOUSEKEEPING PAD w/ #4 @ 18" o.c. E.W. VERIFY LOCATION AND SIZE w/ ARCH. AND MEP PLANS.
4. 1 1/2x7 1/4 VERSA-LAM LVL 1.8E 2400 STUDS @ 16" o.c. w/ SINGLE P.T. BOTTOM PLATE AND DOUBLE TOP PLATES, U.N.O.
5. KING STUD EACH SIDE OF POST.
6. FULL HEIGHT 2x6 @ 16" o.c. RAKE STUD WALL w/ DOUBLE TOP PLATE AND SINGLE P.T. BOTTOM PLATE. USE 1 1/2x5 1/2 VERSA-STUD 1.7 2400 FOR STUD HEIGHTS GREATER THAN 16'-0".
7. 24"x24" CONCRETE PEDESTAL w/ (4) #5 VERT. AT EACH FACE w/ #3 HOOPS @ 10" o.c.
8. HSS5x5x1/4.
9. INDICATES APPROXIMATE EXTENT OF BRICK VENEER. STUDS TO BE @ 12" o.c.
10. FULL HEIGHT RAKE WALL w/ 1 1/2x7 1/4 VERSA-LAM LVL 1.8 2400 STUDS @ 16" o.c. w/ SINGLE P.T. BOTTOM PLATE AND DOUBLE 2x8 TOP PLATE, U.N.O. PROVIDE DOUBLE KING STUD LVL EACH SIDE OF WALL PENETRATIONS, U.N.O.
11. (3) 1 1/2x7 1/4 VERSA-LAM LVL 1.8 2400 KING STUD EACH SIDE OF WALL PENETRATION.
12. STRAP AND BLOCK ABOVE AND BELOW WALL DUCT PENETRATIONS OF F.T.A.O. SHEAR WALL. SEE DETAIL 6/S503 FOR ADDITIONAL INFORMATION.
13. FULL HEIGHT RAKE WALL w/ 1 1/2x5 1/2 VERSA-STUD 1.7 2400 STUDS @ 16" o.c.
14. BALLOON FRAME STUD WALL w/ 1 1/2x5 1/2 VERSA-STUD 1.7 2400 STUDS @ 16" o.c. w/ DOUBLE TOP PLATE AND SINGLE P.T. BOTTOM PLATE.

KEYPLAN

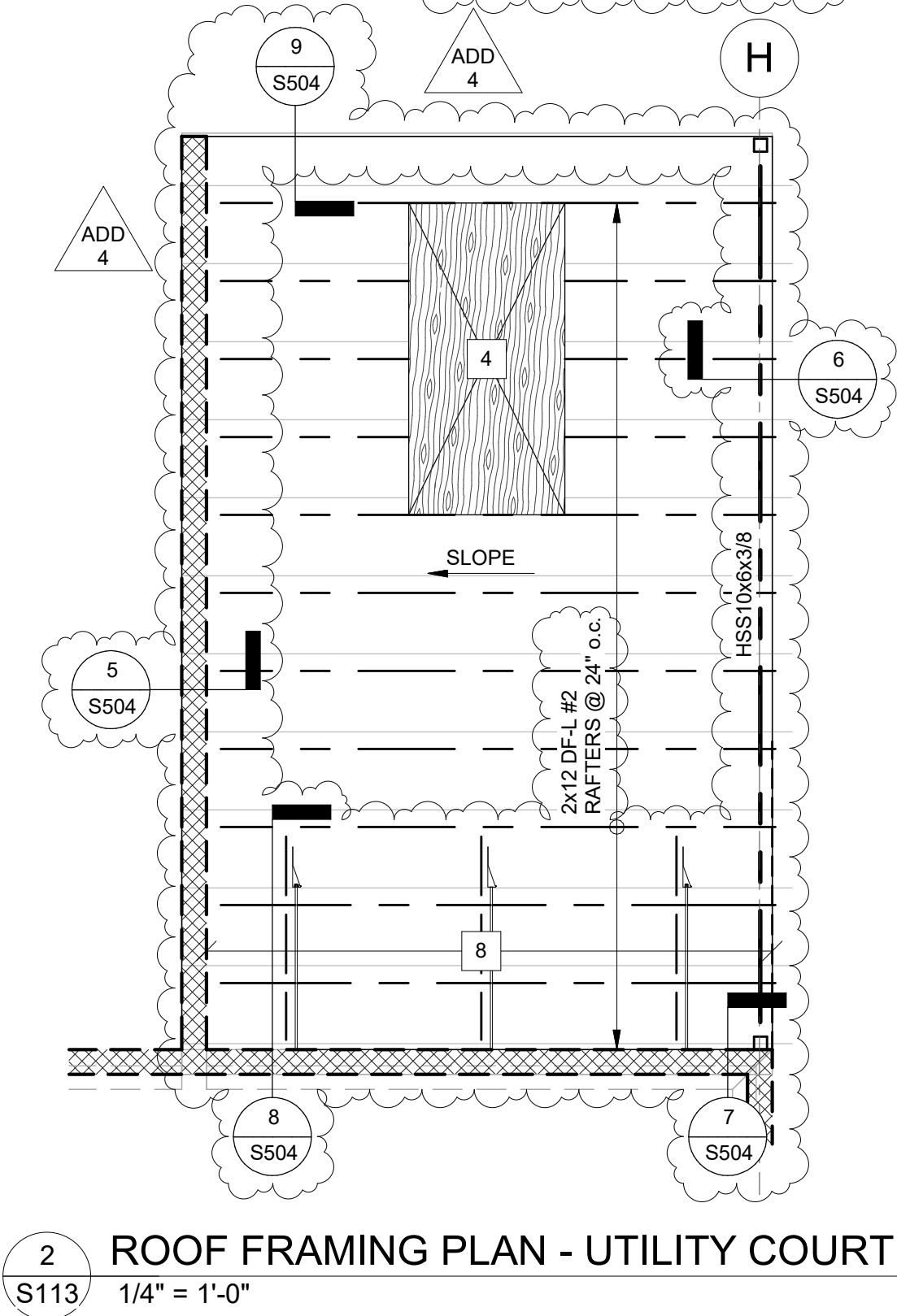


FOUNDATION PLAN - SECTOR B  
1/8" = 1'-0"



FOUNDATION PLAN NOTES

- A. DIMENSIONS SHOWN ARE FOR REFERENCE ONLY. CONFIRM w/ ARCHITECTURAL PLAN & DETAILS.
- B. BOTTOM OF FOOTINGS TO BE PLACED BELOW FROST DEPTH OR AS NOTED IN THE GEOTECHNICAL REPORT, WHICHEVER IS GREATER.
- C. COORDINATE PENETRATIONS OF SITE UTILITIES, MECHANICAL DUCTS, PIPING, AND ELECTRICAL CONDUIT/PANELS TO MINIMIZE IMPACT TO STRUCTURAL FRAMING. PLUMBING FIXTURES SHOWN ON FLOOR FOR REFERENCE AND POSSIBLE FRAMING CONFLICTS ONLY.
- D. ALL FOOTINGS ARE TO BE CENTERED UNDER COLUMNS U.N.O.
- E. ALL FOOTINGS TO BEAR OVER GRADE OVER FIRM, UNDISTURBED, NON-ORGANIC, NON-EXPANSIVE, NATIVE MATERIAL, OR STRUCTURAL FILL AS REQUIRED PER GEOTECHNICAL REPORT.
- F. SEE SHEET S001 AND S002 FOR ALL NOTES.
- G. SEE SHEET S007 FOR ALL SCHEDULES.
- H. S — S INDICATES STEP IN FOOTING. SEE DETAIL 2/S402 FOR ADDITIONAL INFORMATION.
- I. INDICATES SHEAR WALL TYPE. SEE SHEAR WALL SCHEDULE FOR ADDITIONAL INFORMATION.
- J. INDICATES SHEAR WALL LOCATION ABOVE FOUNDATION. SEE SHEAR WALL SCHEDULE ON S007 FOR ADDITIONAL INFORMATION.
- K. INDICATES 2x6 @ 16" o.c. BEARING STUD WALL ABOVE FOUNDATION w/ DOUBLE TOP PLATE AND P.T. BOTTOM PLATE, U.N.O.
- L. ALL SHEAR WALLS INDICATED AS "PERFORATED" THE CONTRACTOR SHALL PROVIDE NAILING PATTERN AROUND ALL WALL PENETRATIONS AS CALLED OUT ON FRAMING PLANS IN CORRESPONDENCE WITH THE SHEAR WALL SCHEDULE.
- M. F.T.A.O. INDICATES FORCE TRANSFER AROUND OPENING IN WALL. PROVIDE PANEL EDGE NAILING AROUND ALL WALL PENETRATIONS PER SHEAR WALL SCHEDULE. SEE ASSOCIATED ELEVATION FOR ADDITIONAL INFORMATION.
- N. INDICATES HOLDOWN TYPE AND LOCATION. SEE HOLDOWN SCHEDULE FOR ADDITIONAL INFORMATION.
- O. X'-X' INDICATES TOP OF SLAB ELEVATION. COORDINATE WITH ARCHITECT.
- P. (F-XX) (XXX'-X') INDICATES FOOTING TYPE PER FOUNDATION SCHEDULE AND TOP OF FOOTING ELEVATION.
- Q. INDICATES EXTERIOR 8" NOMINAL FULLY GROUTED CMU WALL. REINFORCEMENT PER PLAN.



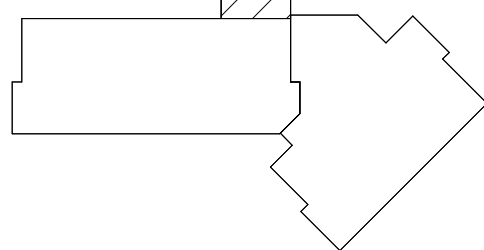
ROOF AND FLOOR FRAMING PLAN NOTES

- A. COORDINATE ALL DIMENSIONS & FEATURES NOT SHOWN WITH ARCHITECT.
- B. SEE SHEET S001 AND S002 FOR ALL NOTES.
- C. SEE SHEET S007 FOR ALL SCHEDULES.
- D. ALL KEYNOTES INDICATE NEW ITEMS TYPICALLY UNLESS NOTED OTHERWISE.
- E. TRUSS LAYOUT IS FOR REFERENCE ONLY. CONFIRM LAYOUT & BEARING CONDITIONS WITH TRUSS SUPPLIER'S LAYOUT PLAN.
- F. INDICATES ROOF STEP TYP. SEE ARCHITECTURAL PLANS FOR ADDITIONAL INFORMATION.
- G. BEAMS ARE EQUALLY SPACED IN BAYS, U.N.O.
- H. BEAMS ARE CENTERED ON COLUMNS, WALLS, AND/OR GRID LINES, U.N.O.
- I. INDICATES SHEAR WALL TYPE. SEE SHEAR WALL SCHEDULE FOR ADDITIONAL INFORMATION.
- J. INDICATES SHEAR WALL LOCATION BELOW FRAMING. SEE SHEAR WALL SCHEDULE ON SHEET S007 FOR ADDITIONAL INFORMATION.
- K. INDICATES 2x6 @ 16" o.c. BEARING STUD WALL BELOW FRAMING w/ DOUBLE TOP PLATE AND SINGLE P.T. BOTTOM PLATE, U.N.O.
- L. ALL SHEAR WALLS INDICATED AS "PERFORATED" THE CONTRACTOR SHALL PROVIDE NAILING PATTERN AROUND ALL WALL PENETRATIONS AS CALLED OUT ON FRAMING PLANS IN CORRESPONDENCE WITH THE SHEAR WALL SCHEDULE.
- M. F.T.A.O. INDICATES FORCE TRANSFER AROUND OPENING IN WALL. PROVIDE PANEL EDGE NAILING AROUND ALL WALL PENETRATIONS PER SHEAR WALL SCHEDULE. SEE ASSOCIATED ELEVATION FOR ADDITIONAL INFORMATION.
- N. INDICATES HOLDOWN TYPE AND LOCATION. SEE HOLDOWN SCHEDULE FOR ADDITIONAL INFORMATION.
- O. HDR-X INDICATES HEADER SIZE. SEE HEADER SCHEDULE ON SHEET S007 FOR ADDITIONAL INFORMATION. TYPICAL INTERIOR NON-BEARING WALL HEADER TO BE (2) 2x6 DF #2 w/ SINGLE TRIMMER AND KING STUD, U.N.O.

UTILITY COURT  
STRUCTURAL PLAN  
KEYNOTES

1. 4" CONCRETE SLAB ON GRADE w/ #4 @ 18" o.c. E.W. TYP. THROUGHOUT. SEE DETAILS 1, 2, AND 3/S401 FOR ADDITIONAL INFORMATION.
2. INDICATES CONTROL JOINTS SPACED @ 8'-0" o.c. MAX. SEE DETAILS 1, 2, AND 3/S401 FOR ADDITIONAL INFORMATION.
3. 4" THICK CONCRETE HOUSEKEEPING PAD w/ #4 @ 18" o.c. E.W.
4. 5/8" PLYWOOD ROOF SHEATHING w/ 3"x0.148"Ø NAILS @ 6" o.c. PANEL EDGE AND 12" o.c. FIELD NAILING. BLOCK ALL PANEL EDGES w/ 2x4 FLAT BLOCKING.
5. 8" NOMINAL CMU WALL w/ #7 VERT. @ 32" o.c. AND #5 HORIZ. @ 24" o.c.
6. 8" NOMINAL CMU WALL w/ #7 VERT. @ 16" o.c. AND #5 HORIZ. @ 24" o.c.
7. DOUBLE TWO END RAFTERS. SEE DETAIL 9/S504 FOR ADDITIONAL INFORMATION.
8. SIMPSON HTT4 OUT-OF-PLANE TIE @ 48" o.c. w/ (3) FULL BAYS OF FULL DEPTH 2x BLOCKING.

KEYPLAN





# ROOF AND FLOOR FRAMING PLAN NOTES

- A.

COORDINATE ALL DIMENSIONS & FEATURES NOT SHOWN WITH ARCHITECT.
- B.

SEE SHEET S001 AND S002 FOR ALL NOTES.
- C.

SEE SHEET S007 FOR ALL SCHEDULES.
- D.

ALL KEYNOTES INDICATE NEW ITEMS TYPICALLY UNLESS NOTED OTHERWISE.
- E.

TRUSS LAYOUT IS FOR REFERENCE ONLY. CONFIRM LAYOUT & BEARING CONDITIONS WITH TRUSS SUPPLIER'S LAYOUT PLAN.
- F.

INDICATES ROOF STEP, TYP. SEE ARCHITECTURAL PLANS FOR ADDITIONAL INFORMATION.
- G.

BEAMS ARE EQUALLY SPACED IN BAYS, U.N.O.
- H.

BEAMS ARE CENTERED ON COLUMNS, WALLS, AND/OR GRID LINES, U.N.O.
- I.

X

INDICATES SHEAR WALL TYPE. SEE SHEAR WALL SCHEDULE ON SHEET S007 FOR ADDITIONAL INFORMATION.
- J.

|||||

INDICATES SHEAR WALL LOCATION BELOW FRAMING. SEE SHEAR WALL SCHEDULE ON SHEET S007 FOR ADDITIONAL INFORMATION.
- K.

==

INDICATES 2x6 @ 16" o.c. BEARING STUDWALL BELOW FRAMING w/ DOUBLE TOP PLATE AND SINGLE P.T. BOTTOM PLATE, U.N.O.
- L.

ALL SHEAR WALLS INDICATED AS "PERFORATED" THE CONTRACTOR SHALL PROVIDE NAILING PATTERN AROUND ALL WALL PENETRATIONS AS CALLED OUT ON FRAMING PLANS IN CORRESPONDENCE WITH THE SHEAR WALL SCHEDULE.
- M.

F.T.A.O.

INDICATES FORCE TRANSFER AROUND OPENING IN WALL. PROVIDE PANEL EDGE NAILING AROUND ALL WALL PENETRATIONS PER SHEAR WALL SCHEDULE. SEE ASSOCIATED ELEVATION FOR ADDITIONAL INFORMATION.
- N.

#

INDICATES HOLDOWN TYPE AND LOCATION. SEE HOLDOWN SCHEDULE FOR ADDITIONAL INFORMATION.
- O.

HDR-X

INDICATES HEADER SIZE. SEE HEADER SCHEDULE ON SHEET S007 FOR ADDITIONAL INFORMATION. TYPICAL INTERIOR NON-BEARING WALL HEADER TO BE (2) 2x6 DF #2 w/ SINGLE TRIMMER AND KING STUD, U.N.O.

## ROOF FRAMING KEYNOTES

- #

1.

W16x26 BEAM FOR FOLDING PARTITION WALL PER ARCH. PLAN. FOLDING PARTITION WALL MAX. HEIGHT OF 14'-0".
2.

5/8" PLYWOOD SHEATHING OVER 2x T&G DECKING w/ 16 GA. x 2" STAPLES @ 6" o.c. BOUNDARY EDGES, 6" o.c. PANEL EDGES, AND 12" o.c. FIELD FASTENING.
3.

5/8" PLYWOOD SHEATHING w/ 3"x0.148" NAILS @ 6" o.c. BOUNDARY AND PANEL EDGE, AND 12" o.c. FIELD NAILING.
4.

3 1/8x6 GLB @ 8'-0" o.c.
5.

A.

NOTE USED

B.

16" RED-I65 ROOF JOISTS @ 24" o.c. w/ SIMPSON U3516/20 FACE MOUNT SKEWED HANGER w/ WEB STIFFENER AT HANGER USING (6) 1 1/2"x0.148" NAILS INTO JOIST. MAX. 16'-0" SPAN w/ 36" CANTILEVER.

C.

16" RED-I90HS ROOF JOISTS @ 24" o.c. w/ SIMPSON U414 FACE MOUNT SKEWED HANGER w/ WEB STIFFENER AT HANGER USING (6) 1 1/2"x0.148" NAILS INTO JOIST.
6.

4x FULL DEPTH DRAG BLOCKING IN WALL CAVITY FOR FULL LENGTH OF SHEAR WALL w/ SIMPSON CMST12 COIL STRAP w/ 3"x0.148" NAILS.
7.

NOT USED.
8.

4x4 BLOCKING w/ CONTINUOUS SIMPSON CMST14 COIL STRAP USING 2 1/2"x0.148" NAILS.
9.

SIMPSON CS14 STRAP w/ (26) 3"x0.148" NAILS AND 16" END LENGTH.
10.

5/8" PLYWOOD SHEATHING w/ 3"x0.148" NAILS @ 6" o.c. BOUNDARY AND PANEL EDGE AND 12" o.c. FIELD NAILING. ALL PANEL EDGES TO BE BLOCKED w/ FLAT 2x4.
11.

CONT. FLAT 2x6 BLOCKING AND SIMPSON CS14 COIL STRAP.
12.

6x8 DRAG BLOCKING IN WALL CAVITY w/ (2) FULL LENGTH OF SHEAR WALL SIMPSON CMST14 COIL STRAPS.
13.

(2) SIMPSON CMST14 STRAPS w/ (66) 2 1/2"x0.148" NAILS AND 30" END LENGTH.
14.

5/8" PLYWOOD SHEATHING OVER 3x T&G DECKING w/ 16 GA. x 2" STAPLES @ 6" o.c. BOUNDARY EDGE, 6" o.c. PANEL EDGE, AND 12" o.c. FIELD FASTENING.
15.

SIMPSON CMST12 COIL STRAP w/ (86) 3"x0.148" NAILS AND 39" END LENGTH.
16.

STRAP OVER BREAKS IN DOUBLE TOP PLATE w/ SIMPSON CMST14 COIL STRAP w/ (66) 3"x0.148" NAILS AND 30" END LENGTH.
17.

SIMPSON HGU5.50/10 FACE MOUNT HANGER w/ (46) 3"x0.162" NAILS INTO HEADER AND (16) 3 1/2"x0.162" NAILS INTO BEAM.
18.

HSS5x4x1/4 SPANDREL AT MID HEIGHT AND TOP OF STOREFRONT.
19.

HSS5x4x1/4 SPANDREL AT THIRD POINTS AND TOP OF STOREFRONT.
20.

ALIGN TRUSS w/ GLB OR SHEAR WALL. TRUSS MANUFACTURER TO DESIGN TRUSS TOP CHORD FOR AXIAL LOADS LISTED ON PLANS.
21.

(2) 2x4 @ 12" o.c. w/ SIMPSON LUS24-2 HANGER.
22.

(2) 2x4 w/ SIMPSON LUS24-2 HANGER. ALIGN w. STUD WALL.
23.

(3) 2x4 OUTLOOKER w/ LUS24-2 HANGER.
24.

CUSTOM HANGER. SEE DETAIL 1/S505 FOR ADDITIONAL INFORMATION.
25.

CUSTOM STEEL HANGER. SEE DETAIL 3/S504 FOR ADDITIONAL INFORMATION.
26.

3 1/2x16 VERSA-LAM LVL 1.8E 2650 LEDGER w/ 6'-0" BACKSPAN. ATTACHED TO EACH STUD w/ (3) SIMPSON SDWS22600DB SCREWS.
27.

3 1/2x16 VERSA-LAM LVL 1.8E 2650 w/ 5'-0" BACKSPAN AND SIMPSON HU412 HANGER.
28.

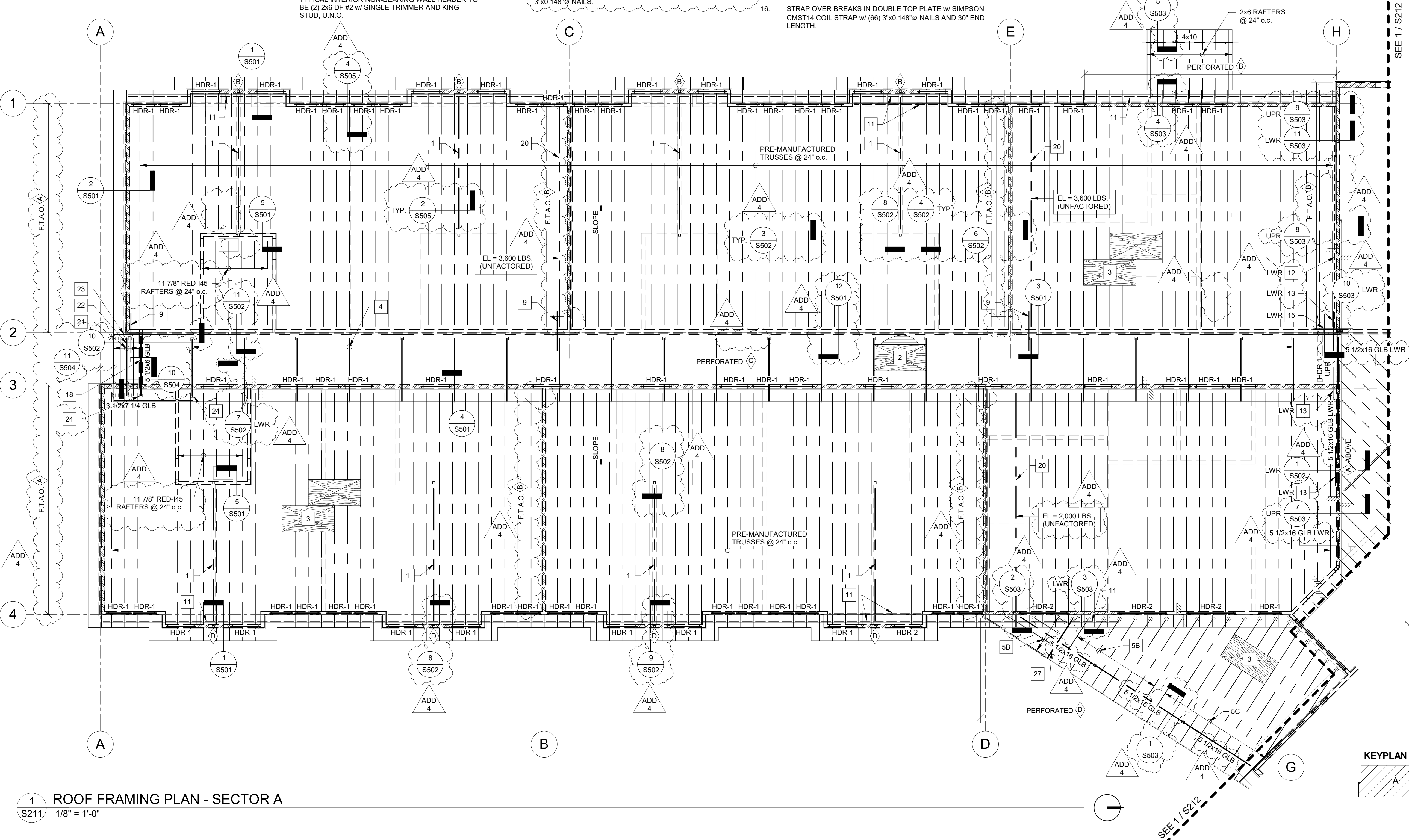
GIRDER TRUSS BY OTHERS. TO BE DESIGNED FOR 1,500 LB (UNFACTORED) DL + SL AT HEADER.
29.

CUSTOM STEEL BRACKET TO ATTACH COLUMN TO BLOCKING. SEE DETAIL 4/S504 FOR ADDITIONAL INFORMATION.
30.

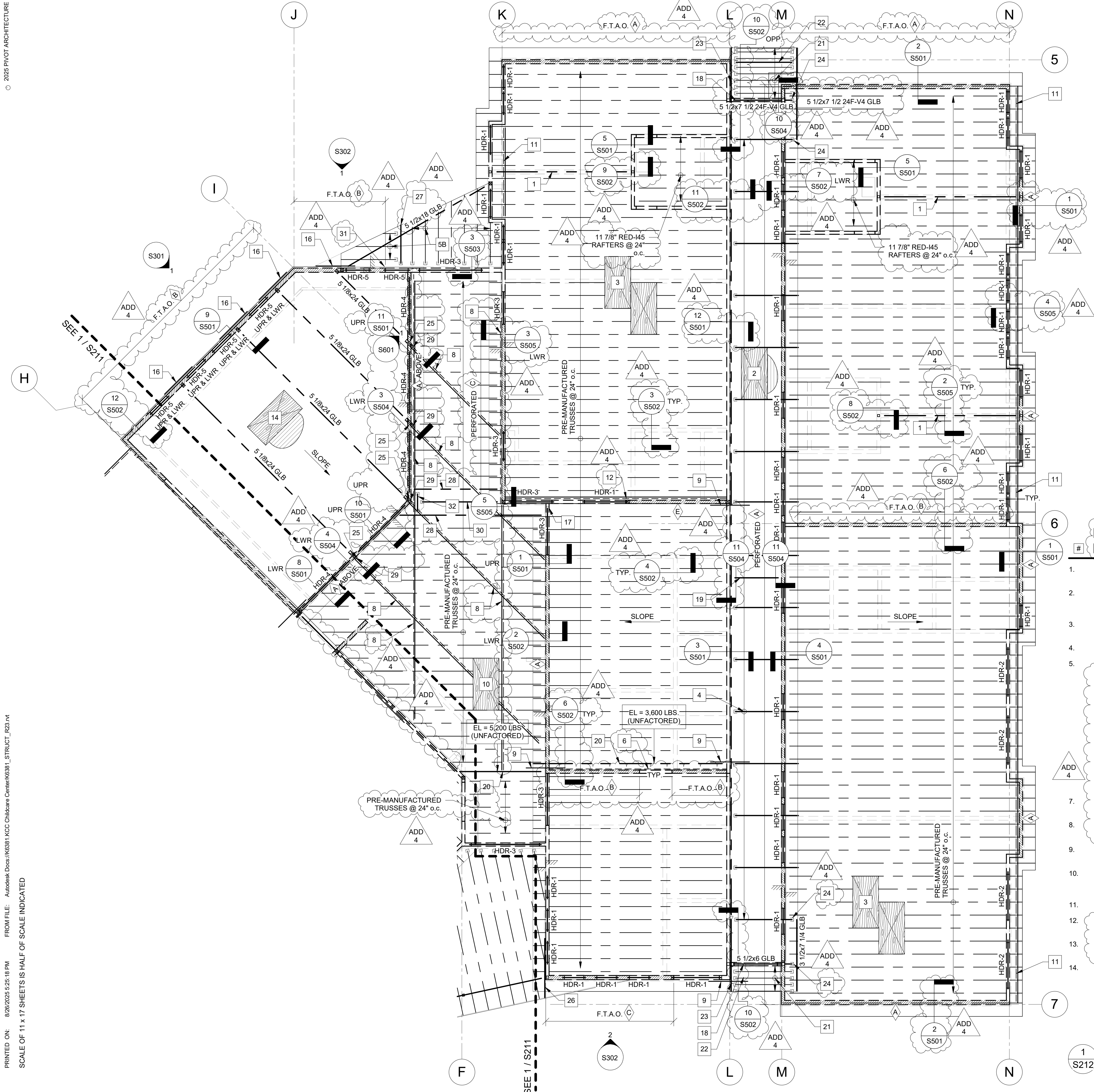
5 1/2x11 1/4 GLB w/ SIMPSON HU612TF HANGER.
31.

STRAP OVER BREAKS IN LEDGER w/ SIMPSON CS14 STRAP w/ (26) 3"x0.148" NAILS AND 16" END LENGTH.
32.

5 1/2 x 11 1/4 GLB HEADER w/ SIMPSON HU612TF HANGERS



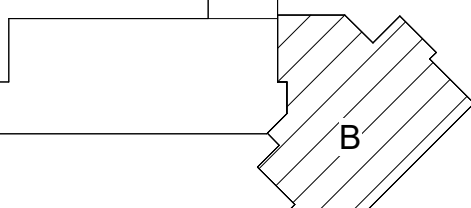




### ROOF FRAMING KEYNOTES

- |   |   |
|---|---|
| 1. W16x26 BEAM FOR FOLDING PARTITION WALL PER ARCH. PLAN. FOLDING PARTITION WALL MAX. HEIGHT OF 14'-0".   | 15. SIMPSON CMST12 COIL STRAP w/ (86) 3"x0.148"Ø NAILS AND 39" END LENGTH.  |
| 2. 5/8" PLYWOOD SHEATHING OVER 2x T&G DECKING w/ 16 GA. x 2" STAPLES @ 6" o.c. BOUNDARY EDGES, 6" o.c. PANEL EDGES, AND 12" o.c. FIELD FASTENING.   | 16. STRAP OVER BREAKS IN DOUBLE TOP PLATE w/ SIMPSON CMST14 COIL STRAP w/ (66) 3"x0.148"Ø NAILS AND 30" END LENGTH.     |
| 3. 5/8" PLYWOOD SHEATHING w/ 3"x0.148" NAILS @ 6" o.c. BOUNDARY AND PANEL EDGE, AND 12" o.c. FIELD NAILING.   | 17. SIMPSON HGU5.50/10 FACE MOUNT HANGER w/ (46) 3"x0.162"Ø NAILS INTO HEADER AND (16) 3 1/2"x0.162"Ø NAILS INTO BEAM.  |
| 4. 3 1/8x6 GLB @ 8'-0" o.c.   | 18. HSS5x4x1/4 SPANDREL AT MID HEIGHT AND TOP OF STOREFRONT.  |
| 5. A. NOT USED.<br>B. 16" RED-I65 ROOF JOISTS @ 24" o.c. w/ SIMPSON U3516/20 FACE MOUNT SKEWED HANGER w/ WEB STIFFENER AT HANGER USING (6) 1 1/2"x0.148"Ø NAILS INTO JOIST. MAX. 16'-0" SPAN w/ 36" CANTILEVER.<br>C. 16" RED-I90HS ROOF JOISTS @ 24" o.c. w/ SIMPSON U414 FACE MOUNT SKEWED HANGER w/ WEB STIFFENER AT HANGER USING (6) 1 1/2"x0.148"Ø NAILS INTO JOIST. | 19. HSS5x4x1/4 SPANDREL AT THIRD POINTS AND TOP OF STOREFRONT.  |
| 6. FULL DEPTH 4x DRAG BLOCKING IN WALL CAVITY FOR FULL LENGTH OF SHEAR WALL w/ SIMPSON CMST12 COIL STRAP w/ 3"x0.148"Ø NAILS.   | 20. ALIGN TRUSS w/ GLB OR SHEAR WALL. TRUSS MANUFACTURER TO DESIGN TRUSS TOP CHORD FOR AXIAL LOADS LISTED ON PLANS.     |
| 7. NOT USED.  | 21. (2) 2x4 @ 12" o.c. w/ SIMPSON LUS24-2 HANGER.   |
| 8. 4x4 BLOCKING w/ CONTINUOUS SIMPSON CMST14 COIL STRAP USING 2 1/2"x0.148" NAILS.  | 22. (2) 2x4 w/ SIMPSON LUS24-2 HANGER. ALIGN w. STUD WALL.  |
| 9. SIMPSON CS14 STRAP w/ (26) 3"x0.148"Ø NAILS AND 16" END LENGTH.  | 23. (3) 2x4 OUTLOOKER w/ LUS24-2 HANGER.  |
| 10. 5/8" PLYWOOD SHEATHING w/ 3"x0.148" NAILS @ 6" o.c. BOUNDARY AND PANEL EDGE AND 12" o.c. FIELD NAILING. ALL PANEL EDGES TO BE BLOCKED w/ FLAT 2x4.  | 24. CUSTOM HANGER. SEE DETAIL 1/S505 FOR ADDITIONAL INFORMATION.  |
| 11. CONT. FLAT 2x6 BLOCKING AND SIMPSON CS14 COIL STRAP.  | 25. CUSTOM STEEL HANGER. SEE DETAIL 3/S504 FOR ADDITIONAL INFORMATION.  |
| 12. 6x8 DRAG BLOCKING IN WALL CAVITY w/ (2) FULL LENGTH OF SHEAR WALL SIMPSON CMST14 COIL STRAPS.   | 26. 3 1/2x16 VERSA-LAM LVL 1.8E 2650 LEDGER w/ 6'-0" BACKSPAN. ATTACHED TO EACH STUD w/ (3) SIMPSON SDWS22600DB SCREWS. |
| 13. (2) SIMPSON CMST14 STRAPS w/ (66) 2 1/2"x0.148" NAILS AND 30" END LENGTH.   | 27. 3 1/2x16 VERSA-LAM LVL 1.8E 2650 w/ 5'-0" BACKSPAN AND SIMPSON HU412 HANGER.  |
| 14. 5/8" PLYWOOD SHEATHING OVER 3x T&G DECKING w/ 16 GA. x 2" STAPLES @ 6" o.c. BOUNDARY EDGE, 6" o.c. PANEL EDGE, AND 12" o.c. FIELD FASTENING.  | 28. GIRDER TRUSS BY OTHERS. TO BE DESIGNED FOR 1,500 LB (UNFACTORED) DL + SL AT HEADER                                  |
|   | 29. CUSTOM STEEL BRACKET TO ATTACH COLUMN TO BLOCKING. SEE DETAIL 4/S504 FOR ADDITIONAL INFORMATION                     |
|   | 30. 5 1/2x11 1/4 GLB w/ SIMPSON HU612TF HANGER.   |
|   | 31. STRAP OVER BREAKS IN LEDGER w/ SIMPSON CS14 STRAP w/ (26) 3"x0.148"Ø NAILS AND 16" END LENGTH.                      |
|   | 32. 5 1/2 x 11 1/4 GLB HEADER w/ SIMPSON HU612TF HANGERS  |

### KEYPLAN



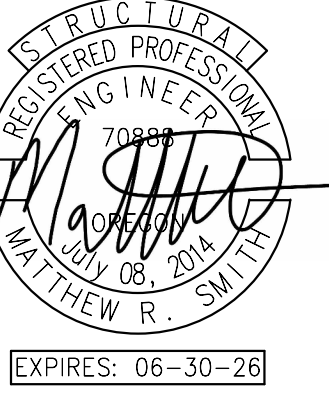
### ROOF FRAMING PLAN - SECTOR B

1/8" = 1'-0"

### ROOF AND FLOOR FRAMING PLAN NOTES

- A. COORDINATE ALL DIMENSIONS & FEATURES NOT SHOWN WITH ARCHITECT.
- B. SEE SHEET S001 AND S002 FOR ALL NOTES.
- C. SEE SHEET S007 FOR ALL SCHEDULES.
- D. ALL KEYNOTES INDICATE NEW ITEMS TYPICALLY UNLESS NOTED OTHERWISE.
- E. TRUSS LAYOUT IS FOR REFERENCE ONLY. CONFIRM LAYOUT & BEARING CONDITIONS WITH TRUSS SUPPLIER'S LAYOUT PLAN.
- F. INDICATES ROOF STEP TYP. SEE ARCHITECTURAL PLANS FOR ADDITIONAL INFORMATION.
- G. BEAMS ARE EQUALLY SPACED IN BAYS, U.N.O.
- H. BEAMS ARE CENTERED ON COLUMNS, WALLS, AND/OR GRID LINES, U.N.O.
- I. INDICATES SHEAR WALL TYPE. SEE SHEAR WALL SCHEDULE FOR ADDITIONAL INFORMATION.
- J. INDICATES SHEAR WALL LOCATION BELOW FRAMING. SEE SHEAR WALL SCHEDULE ON SHEET S007 FOR ADDITIONAL INFORMATION.
- K. INDICATES 2x6 @ 16" o.c. BEARING STUD WALL BELOW FRAMING w/ DOUBLE TOP PLATE AND SINGLE P.T. BOTTOM PLATE, U.N.O.
- L. ALL SHEAR WALLS INDICATED AS "PERFORATED" THE CONTRACTOR SHALL PROVIDE NAILING PATTERN AROUND ALL WALL PENETRATIONS AS CALLED OUT ON FRAMING PLANS IN CORRESPONDENCE WITH THE SHEAR WALL SCHEDULE.
- M. F.T.A.O. INDICATES FORCE TRANSFER AROUND OPENING IN WALL. PROVIDE PANEL EDGE NAILING AROUND ALL WALL PENETRATIONS PER SHEAR WALL SCHEDULE. SEE ASSOCIATED ELEVATION FOR ADDITIONAL INFORMATION.
- N. INDICATES HOLDOWN TYPE AND LOCATION. SEE HOLDOWN SCHEDULE FOR ADDITIONAL INFORMATION.
- O. HDR-X INDICATES HEADER SIZE. SEE HEADER SCHEDULE ON SHEET S007 FOR ADDITIONAL INFORMATION. TYPICAL INTERIOR NON-BEARING WALL HEADER TO BE (2) 2x6 DF #2 w/ SINGLE TRIMMER AND KING STUD, U.N.O.

PIVOT  
ARCHITECTURE



ZCS

BID & PERMIT SET  
KLAMATH COMMUNITY COLLEGE  
EARLY LEARNING CENTER

SHEET TITLE:  
ROOF FRAMING  
PLAN - SECTOR B

REVISIONS:		
#	DESCRP.	DATE
1	ADD 4	08.29.2025

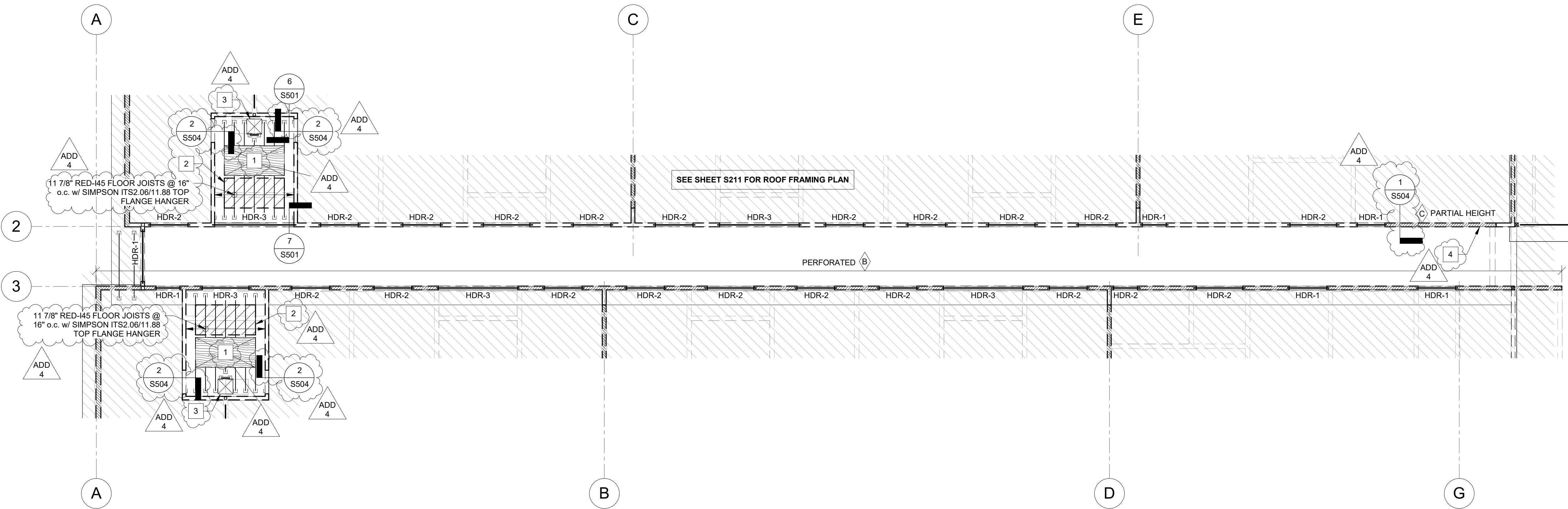
ISSUE DATE: 08/01/2025

S212

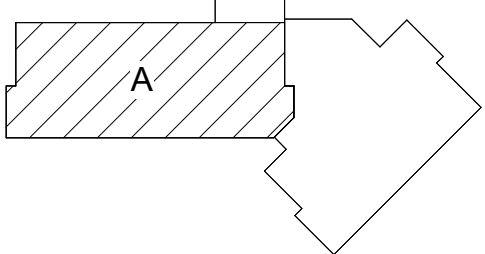
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S213

HALLWAY WALL FRAMING PLAN - SECTOR A

1/8" = 1'-0"



KEYPLAN



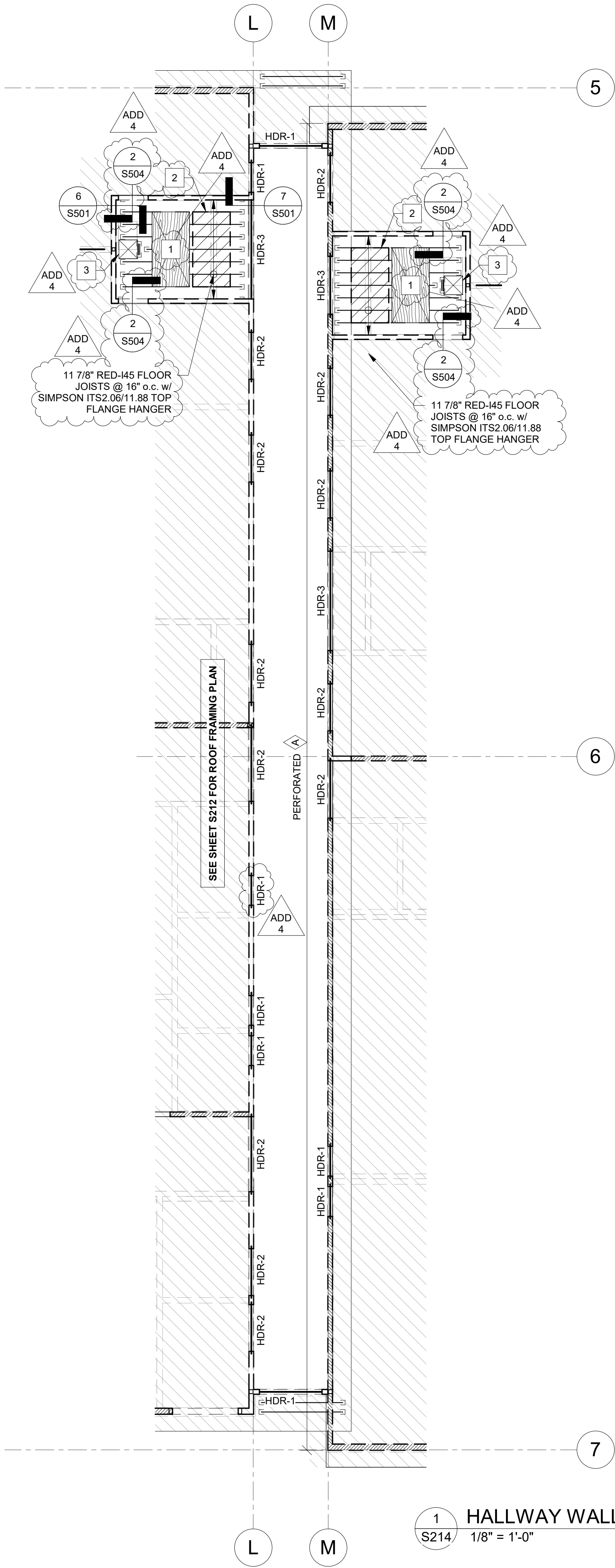
ROOF AND FLOOR FRAMING  
PLAN NOTES

- |    |  |    |          |  |
|----|--|----|----------|--|
| A. | COORDINATE ALL DIMENSIONS & FEATURES NOT SHOWN WITH ARCHITECT.   | J. |          | INDICATES SHEAR WALL LOCATION BELOW FRAMING. SEE SHEAR WALL SCHEDULE ON SHEET S007 FOR ADDITIONAL INFORMATION.   |
| B. | SEE SHEET S001 AND S002 FOR ALL NOTES.   | K. |          | INDICATES 2x6 @ 16" o.c. BEARING STUDWALL BELOW FRAMING w/ DOUBLE TOP PLATE AND SINGLE P.T. BOTTOM PLATE, U.N.O.   |
| C. | SEE SHEET S007 FOR ALL SCHEDULES.  | L. |          | ALL SHEAR WALLS INDICATED AS "PERFORATED" THE CONTRACTOR SHALL PROVIDE NAILING PATTERN AROUND ALL WALL PENETRATIONS AS CALLED OUT ON FRAMING PLANS IN CORRESPONDENCE WITH THE SHEAR WALL SCHEDULE. |
| D. | ALL KEYNOTES INDICATE NEW ITEMS TYPICALLY UNLESS NOTED OTHERWISE.  | M. | F.T.A.O. | INDICATES FORCE TRANSFER AROUND OPENING IN WALL. PROVIDE PANEL EDGE NAILING AROUND ALL WALL PENETRATIONS PER SHEAR WALL SCHEDULE. SEE ASSOCIATED ELEVATION FOR ADDITIONAL INFORMATION.             |
| E. | TRUSS LAYOUT IS FOR REFERENCE ONLY. CONFIRM LAYOUT & BEARING CONDITIONS WITH TRUSS SUPPLIER'S LAYOUT PLAN. | N. |          | INDICATES HOLDOWN TYPE AND LOCATION. SEE HOLDOWN SCHEDULE FOR ADDITIONAL INFORMATION.  |
| F. | INDICATES ROOF STEP, TYP. SEE ARCHITECTURAL PLANS FOR ADDITIONAL INFORMATION.                              | O. | HDR-X    | INDICATES HEADER SIZE. SEE HEADER SCHEDULE ON SHEET S007 FOR ADDITIONAL INFORMATION. TYPICAL INTERIOR NON-BEARING WALL HEADER TO BE (2) 2x6 DF #2 w/ SINGLE TRIMMER AND KING STUD, U.N.O.          |
| G. | BEAMS ARE EQUALLY SPACED IN BAYS, U.N.O.   |    |          |  |
| H. | BEAMS ARE CENTERED ON COLUMNS, WALLS, AND/OR GRID LINES, U.N.O.  |    |          |  |
| I. | INDICATES SHEAR WALL TYPE. SEE SHEAR WALL SCHEDULE ON SHEET S007 FOR ADDITIONAL INFORMATION.               |    |          |  |

MEZZANINE FLOOR  
FRAMING KEYNOTES

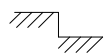


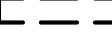
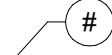
- |    |  |
|----|--|
| #  |  |
| 1. | 1 1/8" PLYWOOD FLOOR SHEATHING w/ 3"x0.148"Ø NAILS @ 6" o.c. BOUNDARY & PANEL EDGE AND 12" o.c. FIELD NAILING.   |
| 2. | MECHANICAL UNIT BY OTHERS. MAX. WEIGHT = 3,000 LBS.  |
| 3. | INDICATES APPROXIMATE LOCATION OF MEZZANINE ACCESS PANEL. VERIFY LOCATION PER ARCH. PLANS. PROVIDE DOUBLE TRIMMER JOIST EACH SIDE OF OPENING AND DOUBLE JOIST HEADER, BOTH w/ SIMPSON MIU4.12/11 FACE MOUNT HANGER. PROVIDE WEB STIFFENER FOR HEADER HANGER PER MANUFACTURER REQUIREMENTS. |
| 4. | 4x FULL DEPTH DRAG BLOCKING IN WALL CAVITY FOR FULL LENGTH OF SHEAR WALL w/ SIMPSON CMST12 COIL STRAP w/ 3"x0.148"Ø NAILS.   |

ADD  
4



1 HALLWAY WALL FRAMING PLAN - SECTOR B  
S214 1/8" = 1'-0"

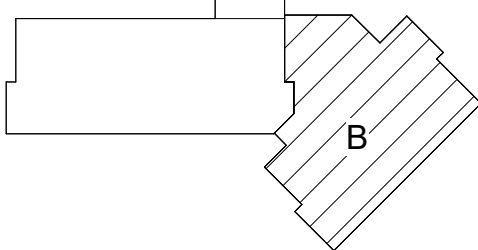
## ROOF AND FLOOR FRAMING PLAN NOTES

- A. COORDINATE ALL DIMENSIONS & FEATURES NOT SHOWN WITH ARCHITECT.
- B. SEE SHEET S001 AND S002 FOR ALL NOTES.
- C. SEE SHEET S007 FOR ALL SCHEDULES.
- D. ALL KEYNOTES INDICATE NEW ITEMS TYPICALLY UNLESS NOTED OTHERWISE.
- E. TRUSS LAYOUT IS FOR REFERENCE ONLY. CONFIRM LAYOUT & BEARING CONDITIONS WITH TRUSS SUPPLIER'S LAYOUT PLAN.
- F.  INDICATES ROOF STEP TYP. SEE ARCHITECTURAL PLANS FOR ADDITIONAL INFORMATION.
- G. BEAMS ARE EQUALLY SPACED IN BAYS, U.N.O.
- H. BEAMS ARE CENTERED ON COLUMNS, WALLS, AND/OR GRID LINES, U.N.O.
- I.  INDICATES SHEAR WALL TYPE. SEE SHEAR WALL SCHEDULE FOR ADDITIONAL INFORMATION.
- J.  INDICATES SHEAR WALL LOCATION BELOW FRAMING. SEE SHEAR WALL SCHEDULE ON SHEET S007 FOR ADDITIONAL INFORMATION.
- K.  INDICATES 2x6 @ 16" o.c. BEARING STUD WALL BELOW FRAMING w/ DOUBLE TOP PLATE AND SINGLE P.T. BOTTOM PLATE, U.N.O.
- L. ALL SHEAR WALLS INDICATED AS "PERFORATED" THE CONTRACTOR SHALL PROVIDE NAILING PATTERN AROUND ALL WALL PENETRATIONS AS CALLED OUT ON FRAMING PLANS IN CORRESPONDENCE WITH THE SHEAR WALL SCHEDULE.
- M. F.T.A.O. INDICATES FORCE TRANSFER AROUND OPENING IN WALL. PROVIDE PANEL EDGE NAILING AROUND ALL WALL PENETRATIONS PER SHEAR WALL SCHEDULE. SEE ASSOCIATED ELEVATION FOR ADDITIONAL INFORMATION.
- N.  INDICATES HOLDOWN TYPE AND LOCATION. SEE HOLDOWN SCHEDULE FOR ADDITIONAL INFORMATION.
- O. HDR-X INDICATES HEADER SIZE. SEE HEADER SCHEDULE ON SHEET S007 FOR ADDITIONAL INFORMATION. TYPICAL INTERIOR NON-BEARING WALL HEADER TO BE (2) 2x6 DF #2 w/ SINGLE TRIMMER AND KING STUD, U.N.O.

## MEZZANINE FLOOR FRAMING KEYNOTES

1. 1 1/8" PLYWOOD FLOOR SHEATHING w/ 3"x0.148"Ø NAILS @ 6" o.c. BOUNDARY & PANEL EDGE AND 12" o.c. FIELD NAILING.
2. MECHANICAL UNIT BY OTHERS. MAX. WEIGHT = 3,000 LBS.
3. INDICATES APPROXIMATE LOCATION OF MEZZANINE ACCESS PANEL. VERIFY LOCATION PER ARCH. PLANS. PROVIDE DOUBLE TRIMMER JOIST EACH SIDE OF OPENING AND DOUBLE JOIST HEADER, BOTH w/ SIMPSON MIU4.12/11 FACE MOUNT HANGER. PROVIDE WEB STIFFENER FOR HEADER HANGER PER MANUFACTURER REQUIREMENTS.
4. 4x FULL DEPTH DRAG BLOCKING IN WALL CAVITY FOR FULL LENGTH OF SHEAR WALL w/ SIMPSON CMST12 COIL STRAP w/ 3"x0.148"Ø NAILS.

### KEYPLAN



### BID & PERMIT SET

### KLAMATH COMMUNITY COLLEGE EARLY LEARNING CENTER

PROJECT #: K-6381-24  
KLAMATH COMMUNITY COLLEGE  
7390 S. 6TH AVE.  
KLAMATH FALLS, OR 97603

SHEET TITLE:

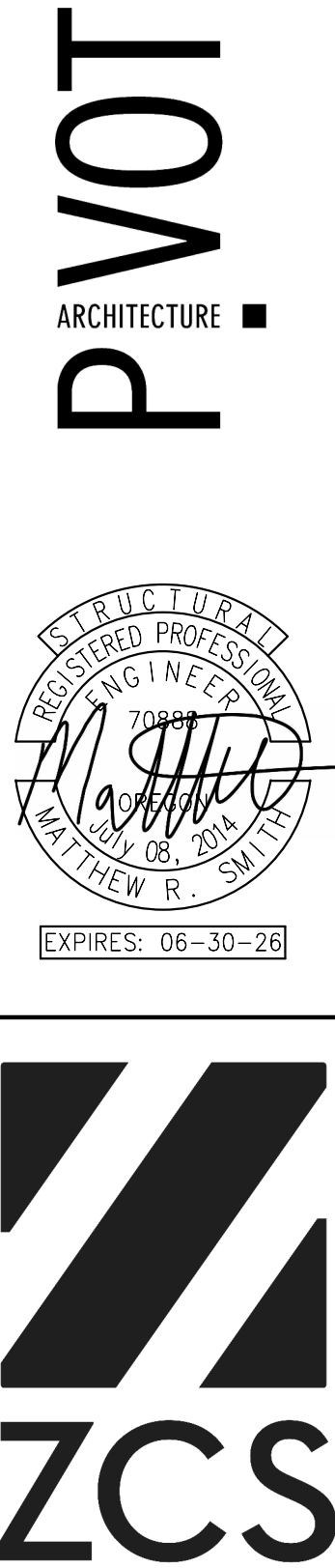
**HALLWAY WALL  
FRAMING PLAN -  
SECTOR B**

REVISIONS:

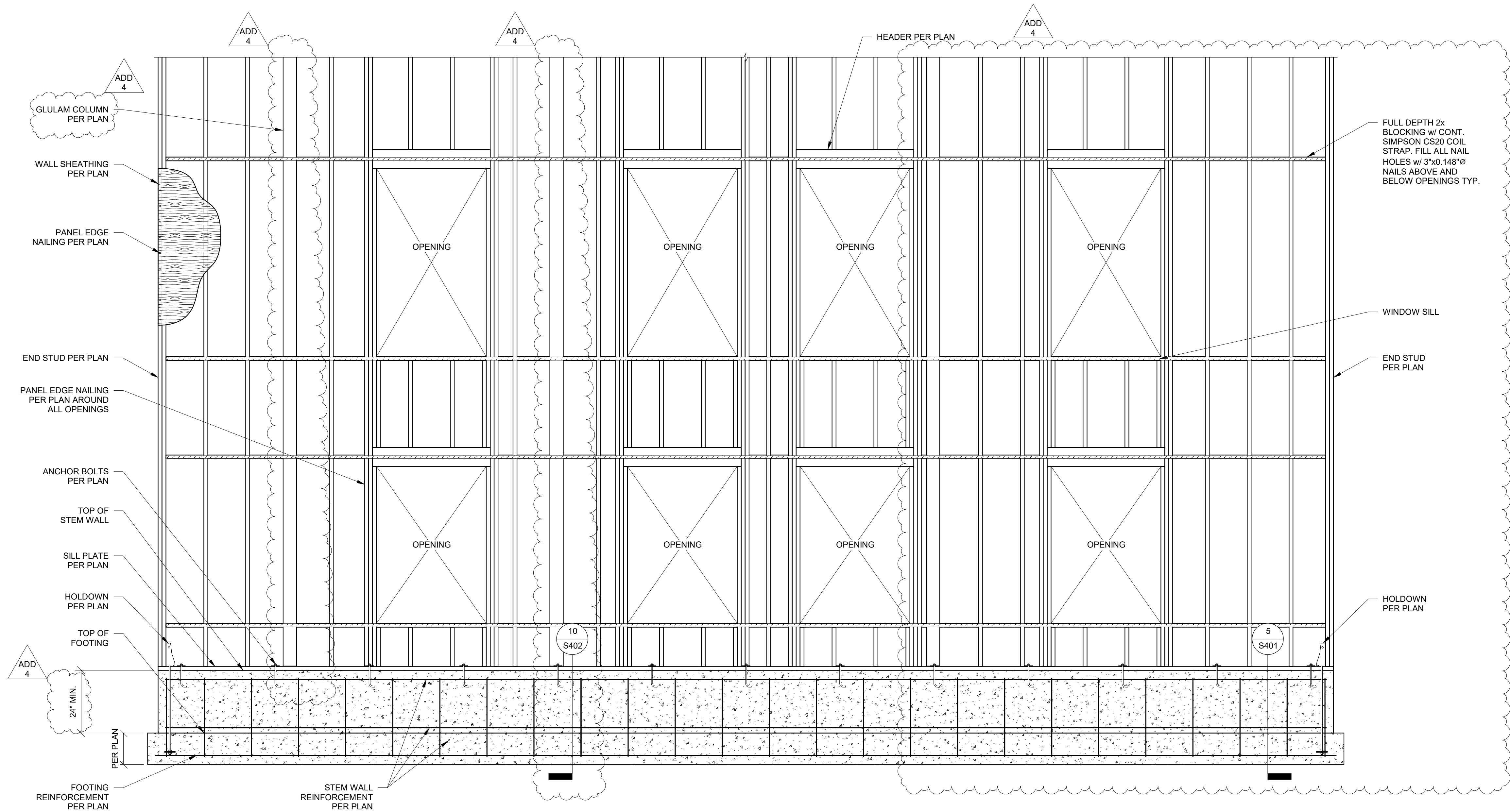
#	DESCRP.	DATE
1	ADD 4	08.29.2025

ISSUE DATE: 08/01/2025

S214

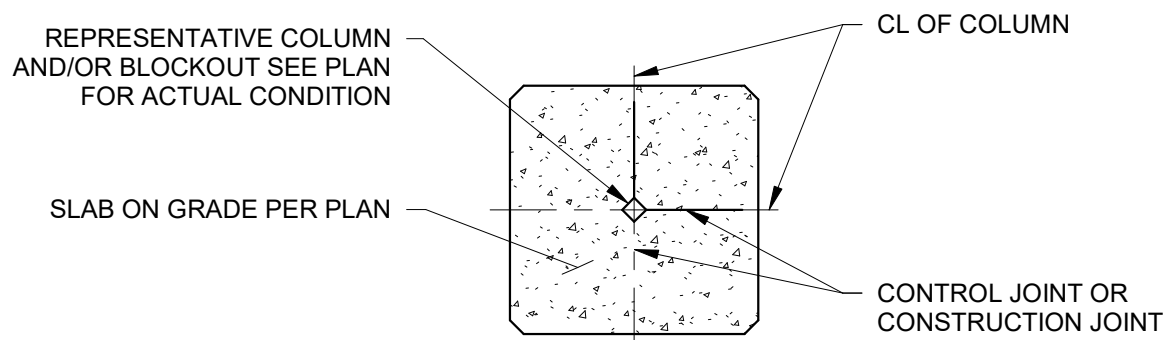




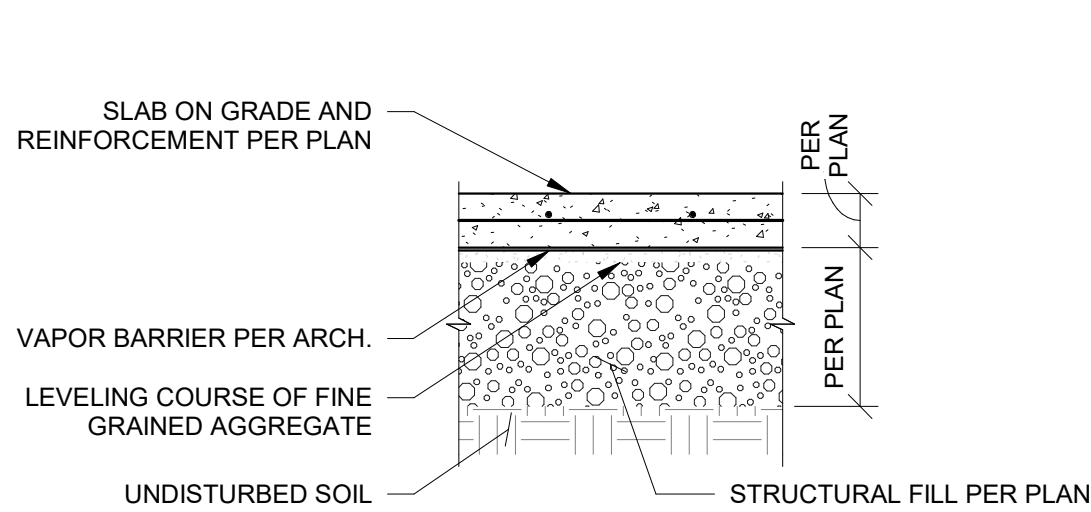


1 GL 0.9 F.T.A.O. SHEAR WALL ELEVATION  
S301 1/2" = 1'-0"

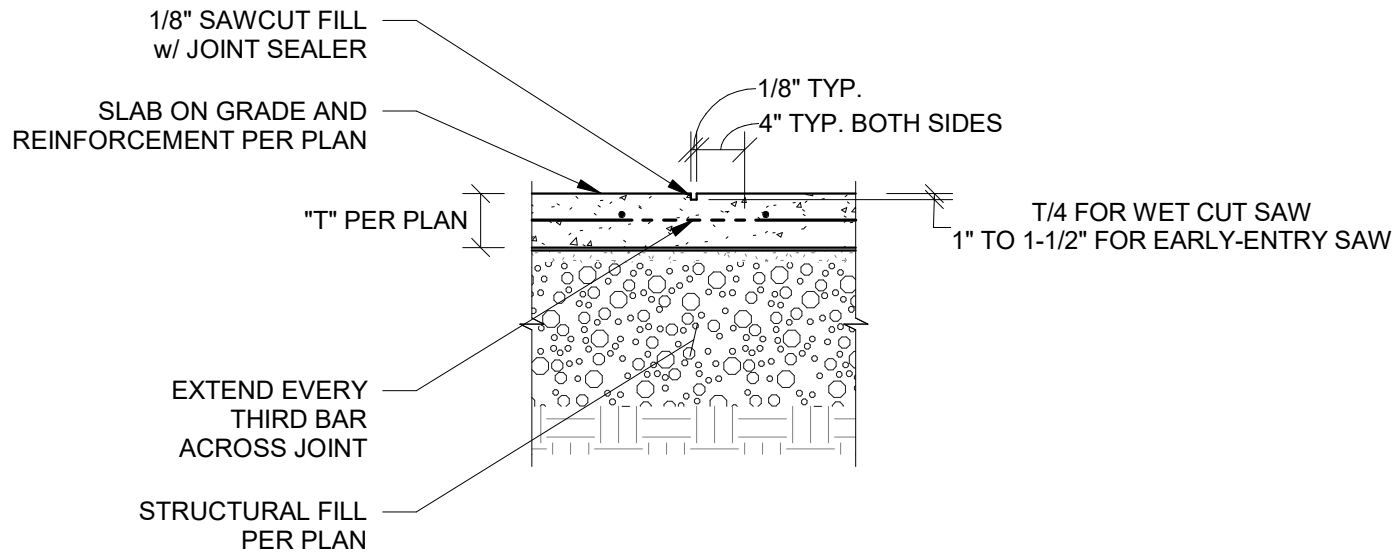




- NOTES:
- SEE PLAN FOR SLAB ON GRADE THICKNESS AND REINFORCEMENT.
  - CONTRACTOR SHALL SUBMIT SHOP DRAWINGS SHOWING JOINT LAYOUT FOR REVIEW AND APPROVAL BY ARCHITECT AND STRUCTURAL ENGINEER.
  - JOINT MAXIMUM SPACING PER PLAN. SEE ARCHITECTURAL PLANS FOR SPECIFIC JOINT LAYOUT.
  - SAWED JOINTS SHALL BE MADE AS SOON AS THE JOINT CAN BE CUT WITHOUT EDGES RAVELING AND WITHIN 12 HOURS OF SLAB PLACEMENT.

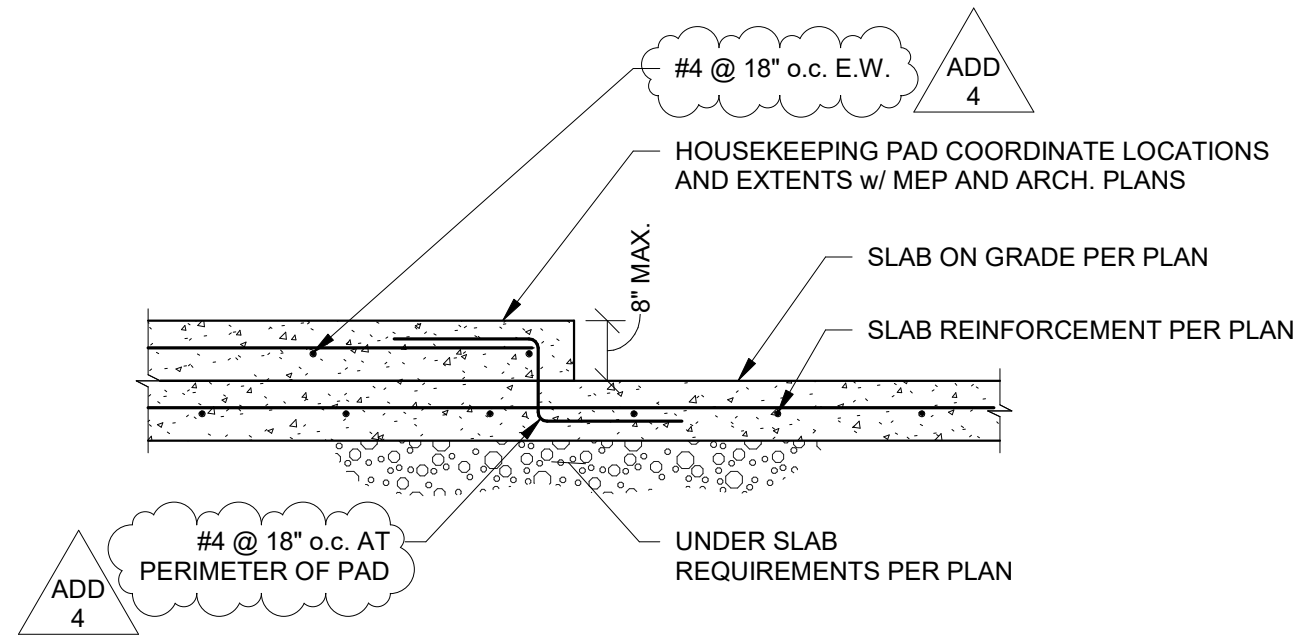


NOTE:  
SEE 1/S401 FOR ADDITIONAL INFORMATION.



**SAW CUT JOINT**

NOTE:  
SEE 1/S401 FOR ADDITIONAL INFORMATION.

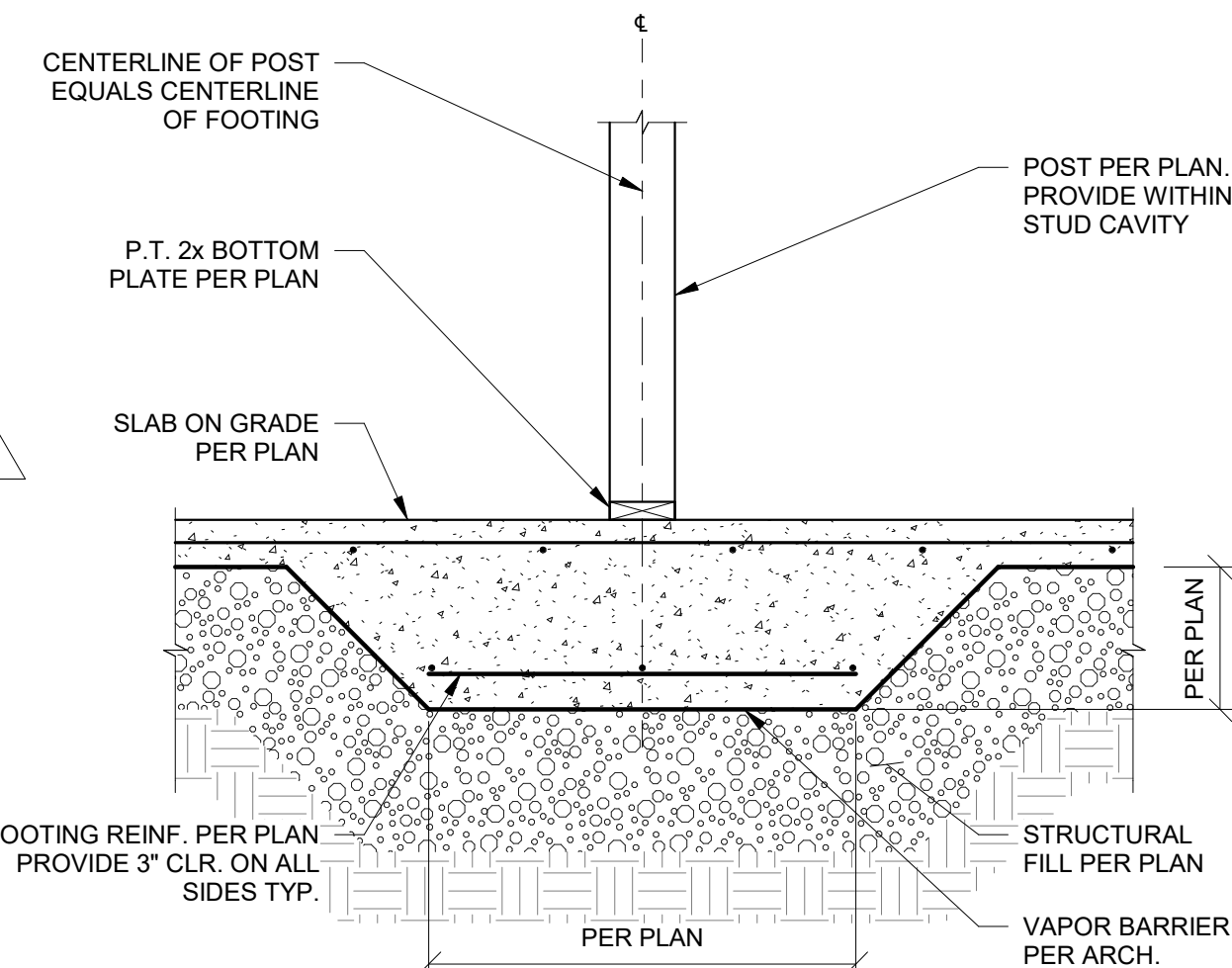
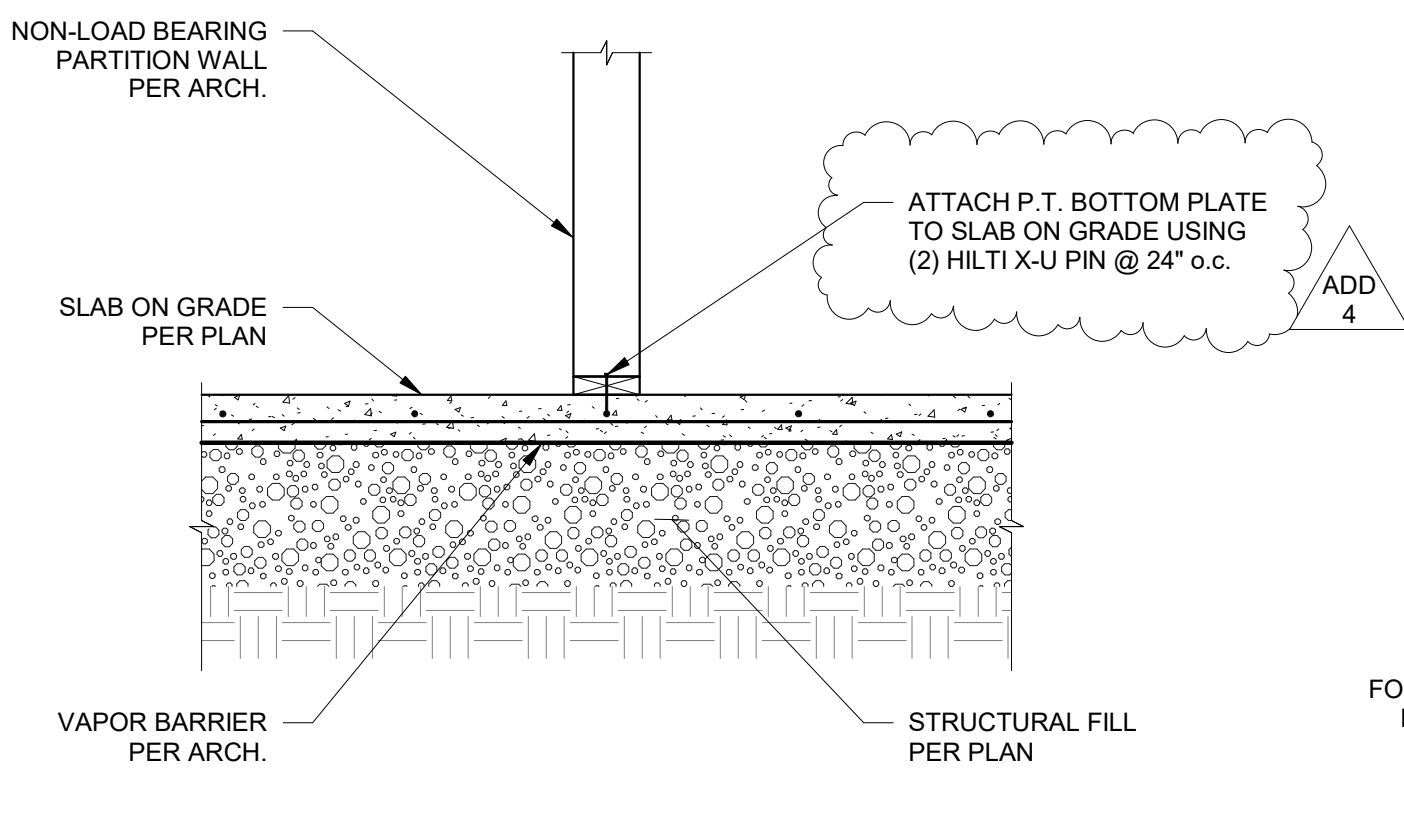
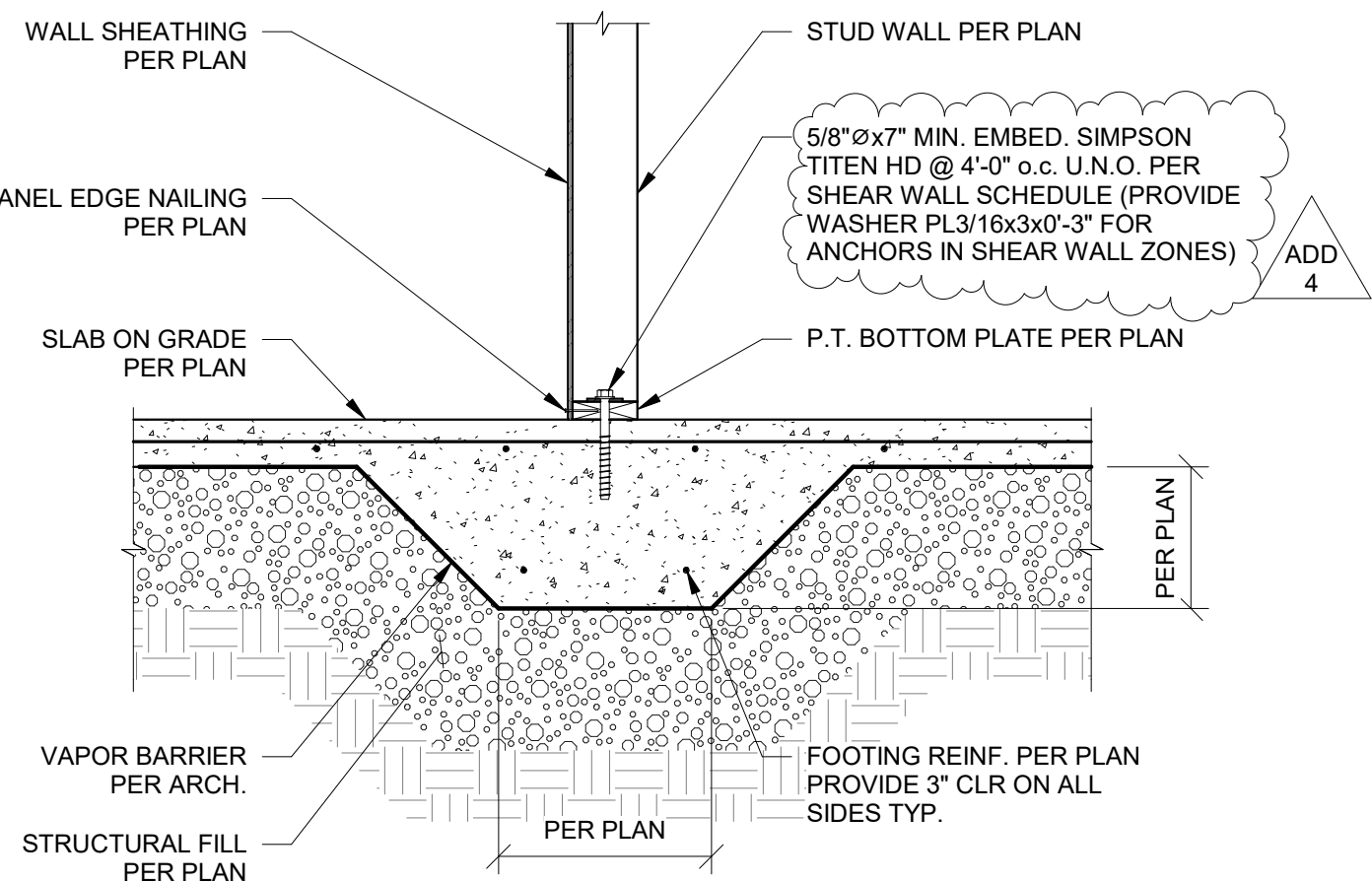
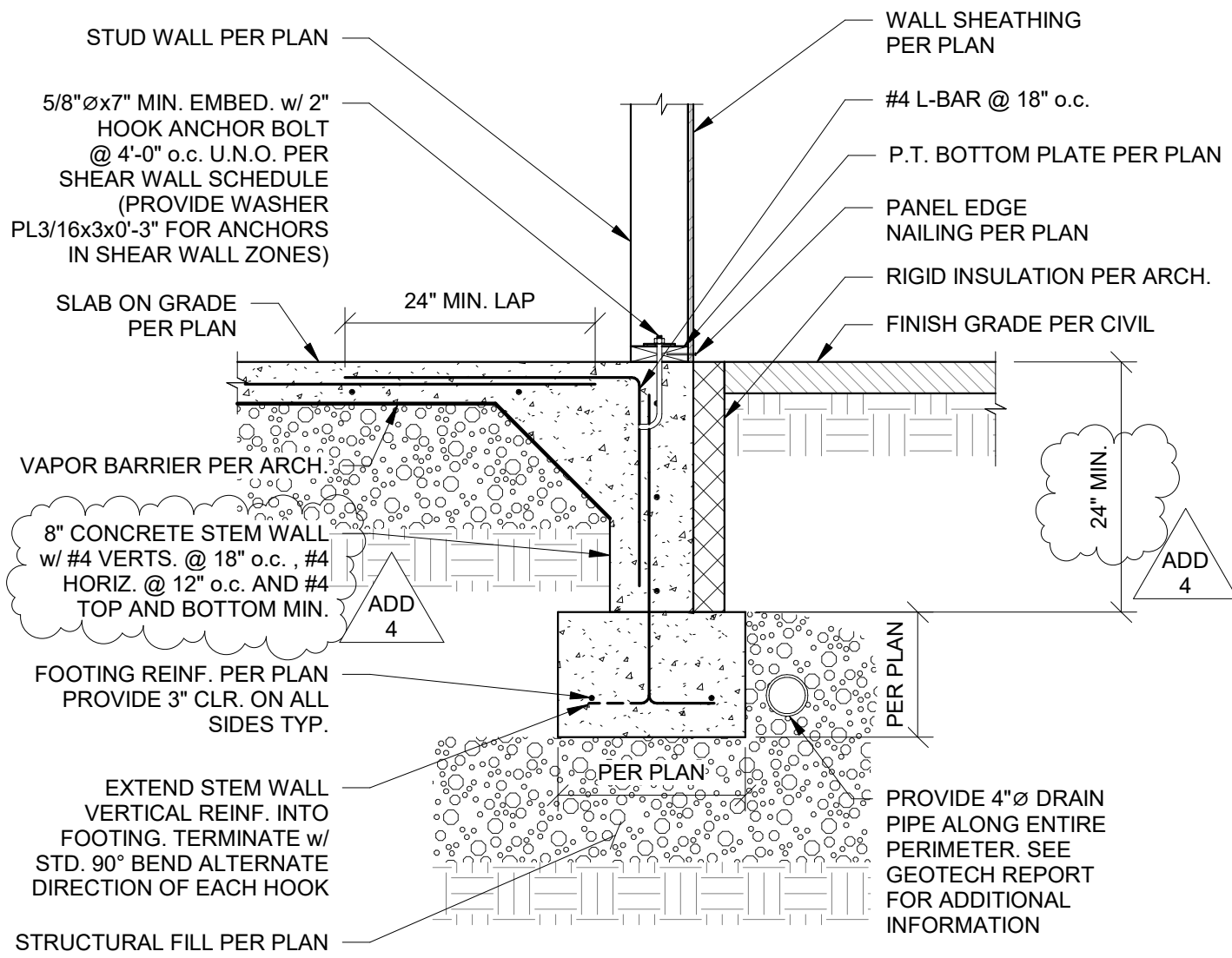


**1 JOINTS AT SLAB ON GRADE**  
S401 3/4" = 1'-0"

**2 UNDER SLAB PREPARATION AT SLAB ON GRADE**  
S401 3/4" = 1'-0"

**3 CONTROL JOINTS IN SLAB ON GRADE**  
S401 3/4" = 1'-0"

**4 HOUSEKEEPING PAD AT SLAB ON GRADE**  
S401 3/4" = 1'-0"

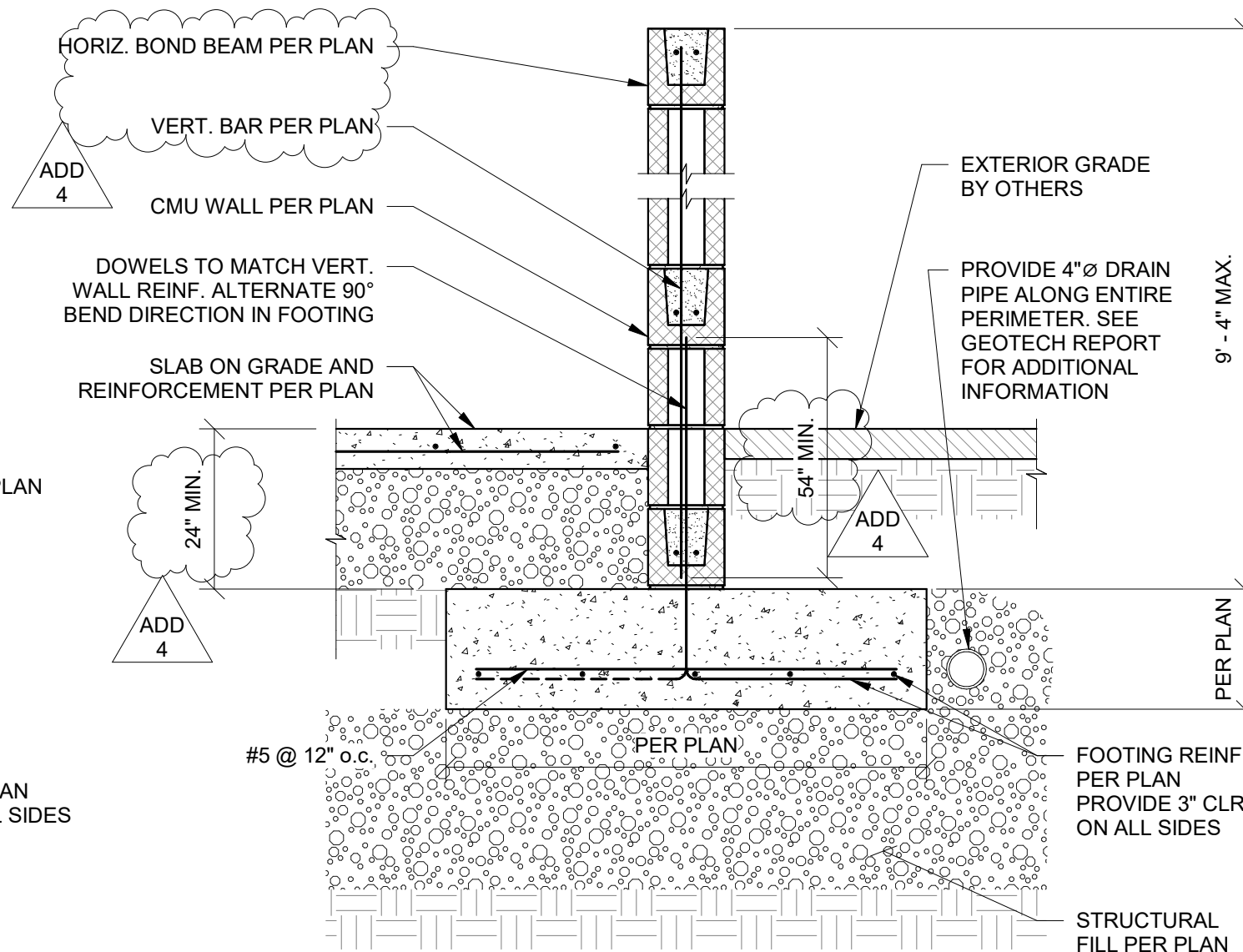
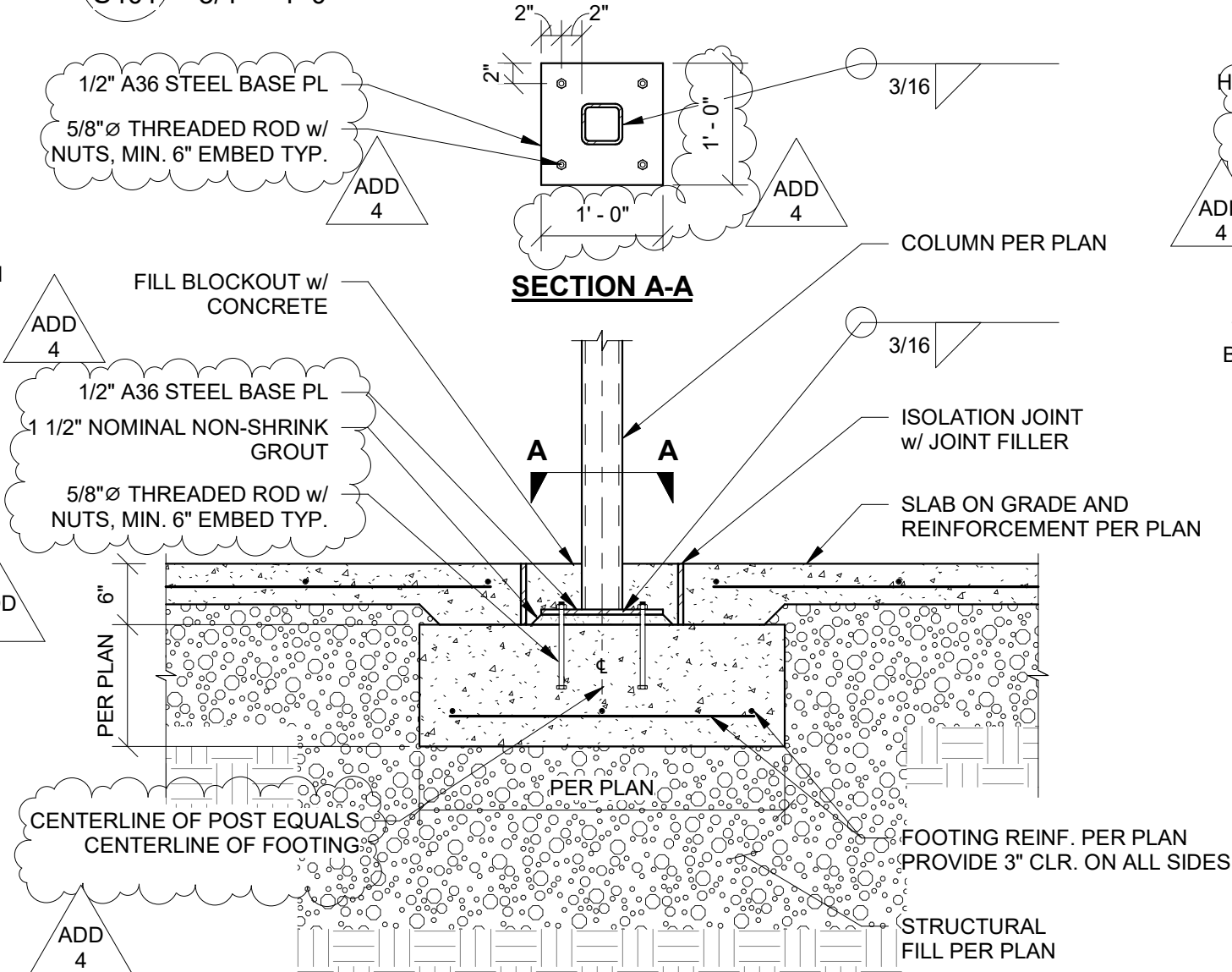
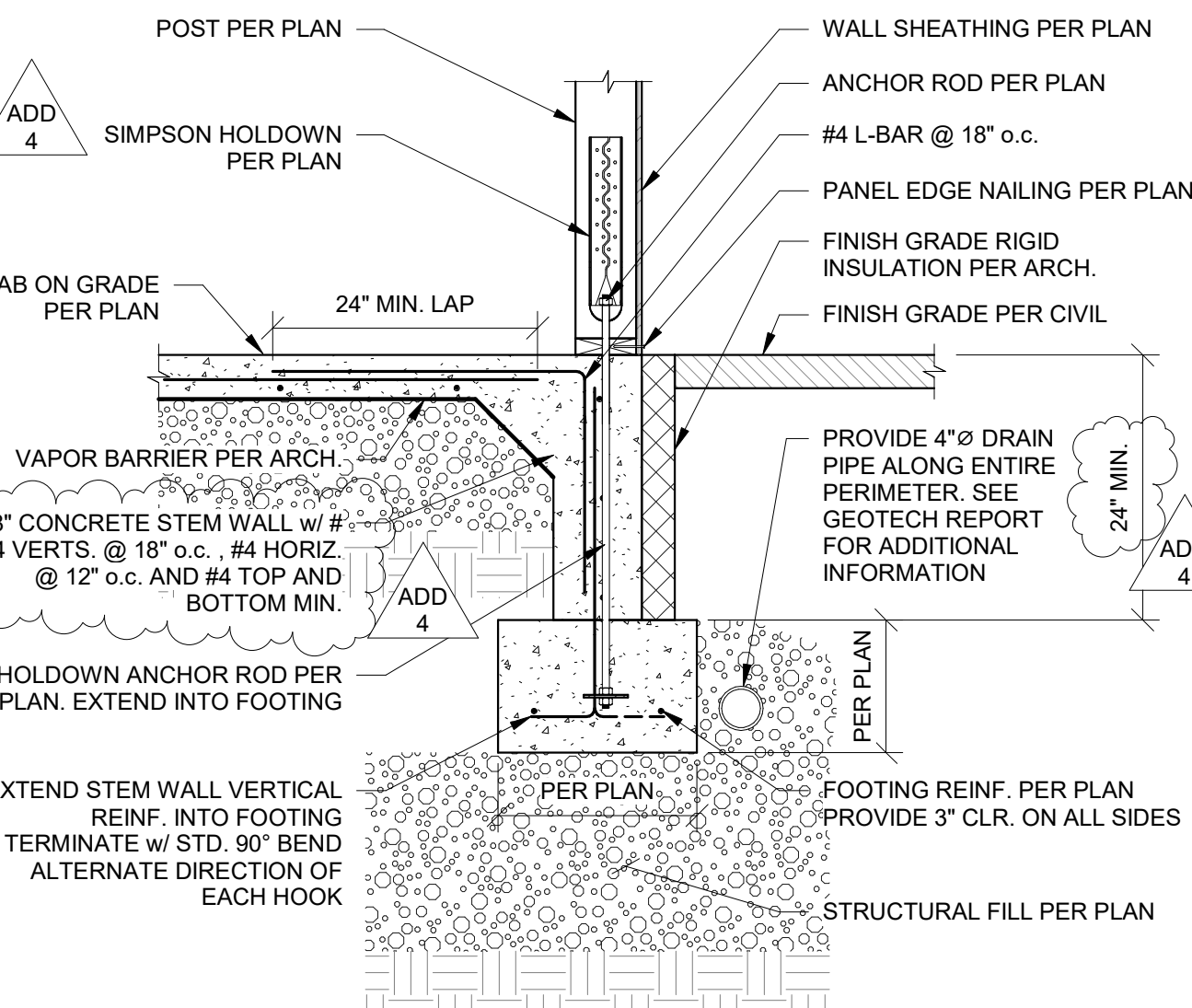
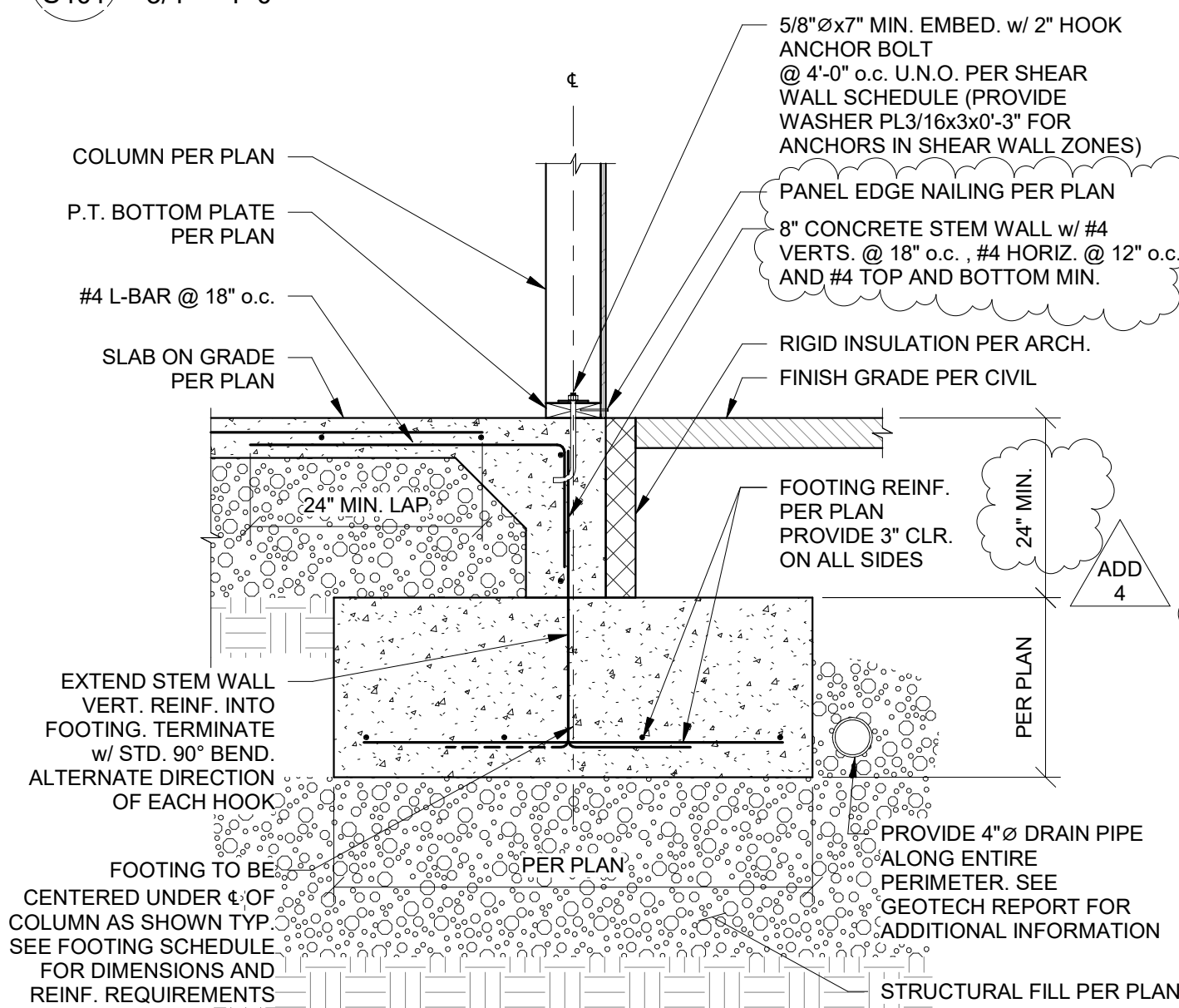


**5 TYP. PERIMETER FOOTING AND STEM WALL**  
S401 3/4" = 1'-0"

**6 TYP. THICKENED SLAB FOOTING AT SHEAR WALL**  
S401 3/4" = 1'-0"

**7 TYP. INT. NON-LOAD BEARING WALL ANCHORAGE**  
S401 3/4" = 1'-0"

**8 TYP. INTERIOR SPREAD FOOTING**  
S401 3/4" = 1'-0"



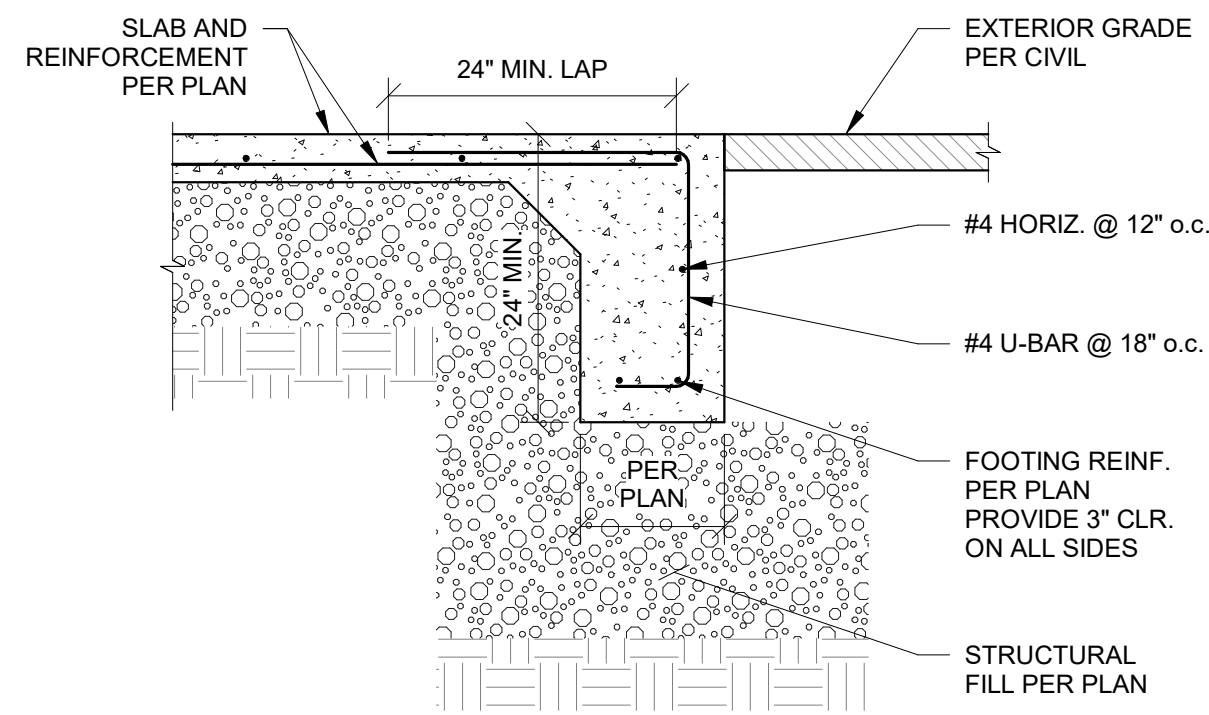
**9 TYP. EXTERIOR SPREAD FOOTING**  
S401 3/4" = 1'-0"

**10 SECTION AT END OF EXTERIOR SHEAR WALL**  
S401 3/4" = 1'-0"

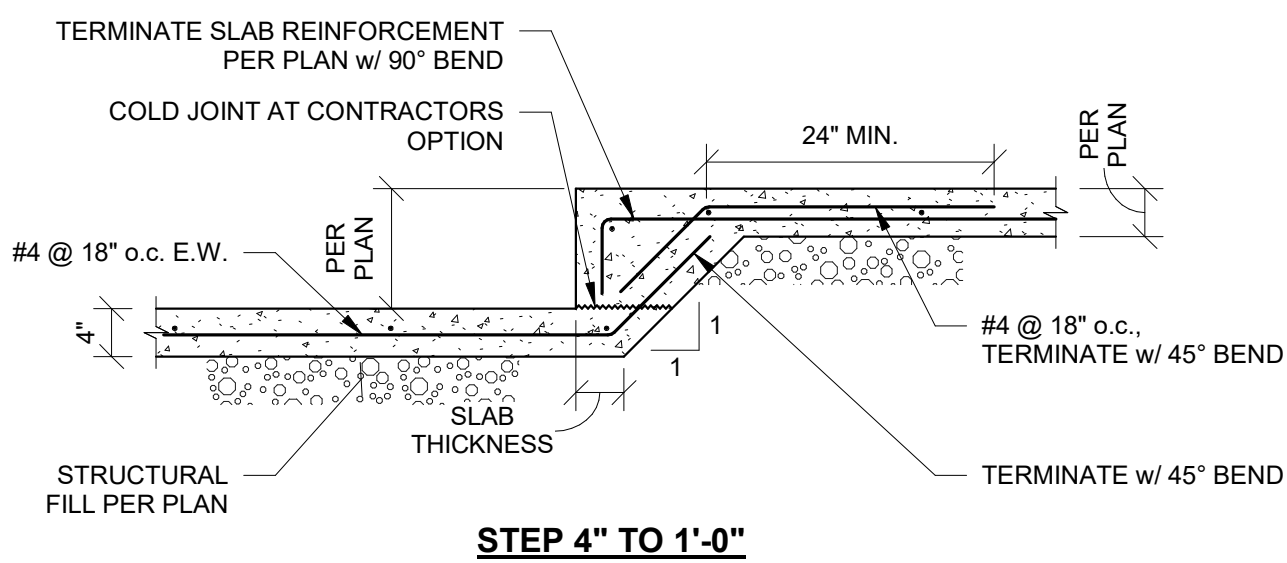
**11 INTERIOR FOOTING DETAIL AT STEEL COLUMN**  
S401 3/4" = 1'-0"

**12 UTILITY COURT CMU WALL**  
S401 3/4" = 1'-0"



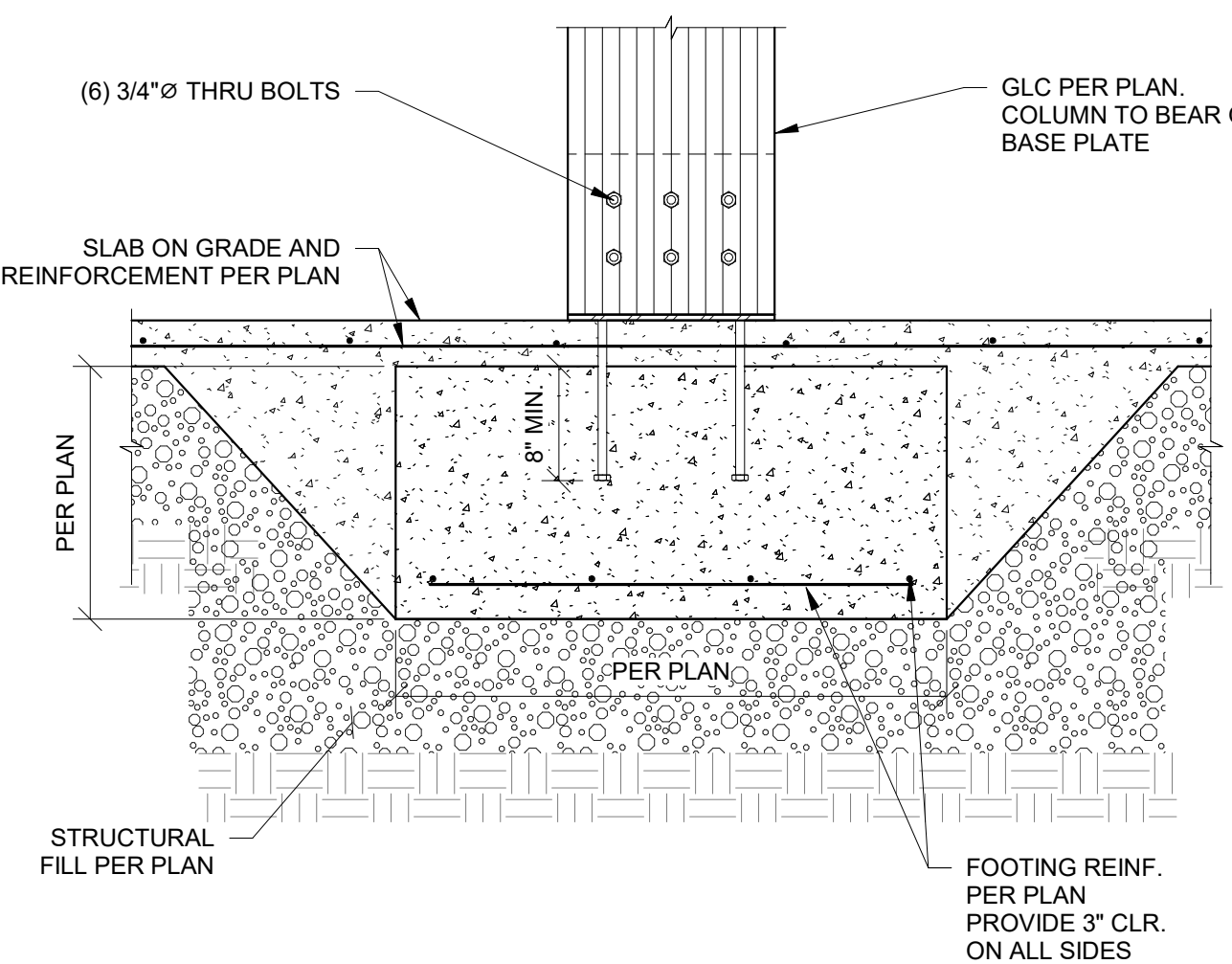


1 EXTERIOR TURNDOWN SLAB  
S402 3/4" = 1'-0"

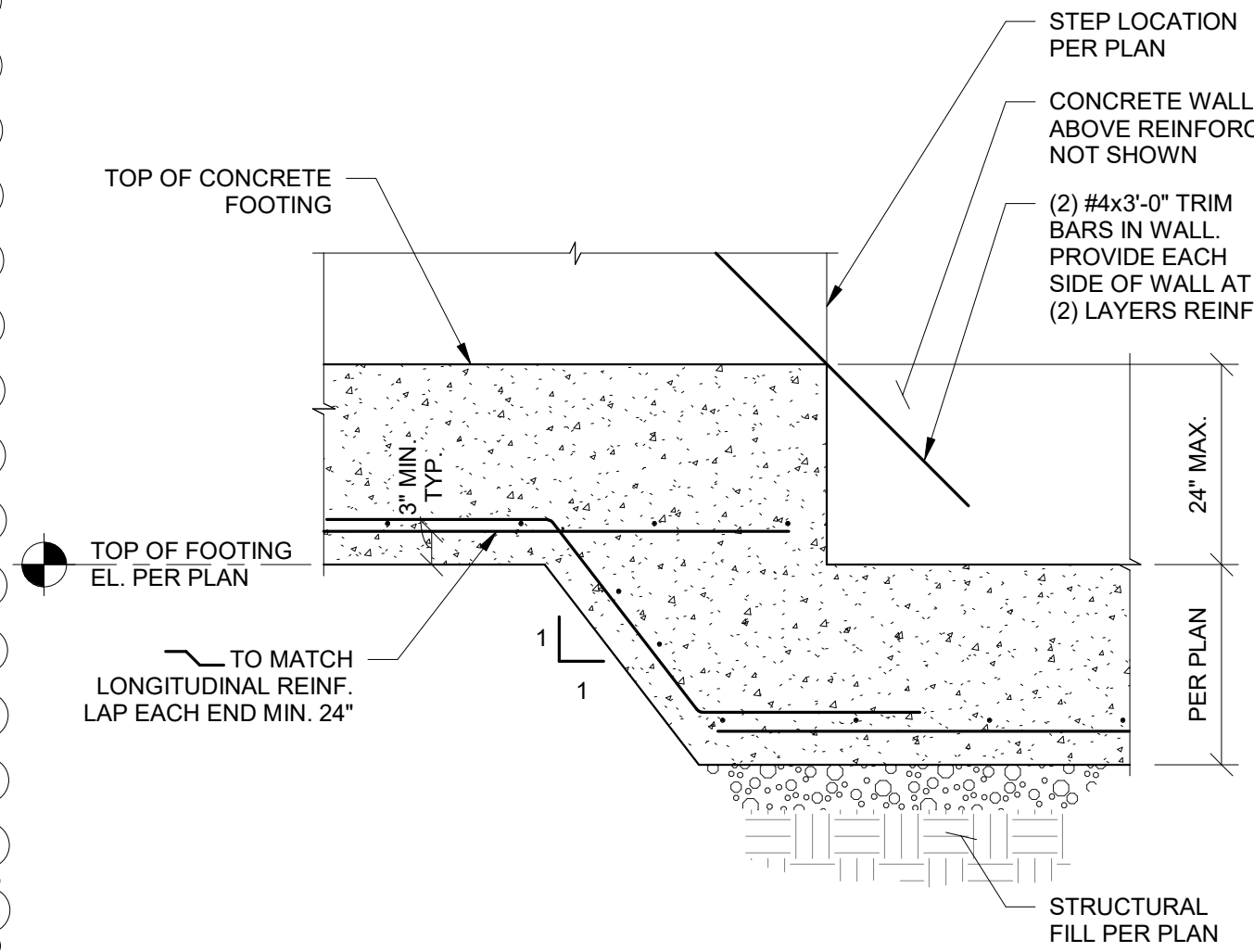


4 HSS COLUMN AT PERIMETER FOOTING  
S402 3/4" = 1'-0"

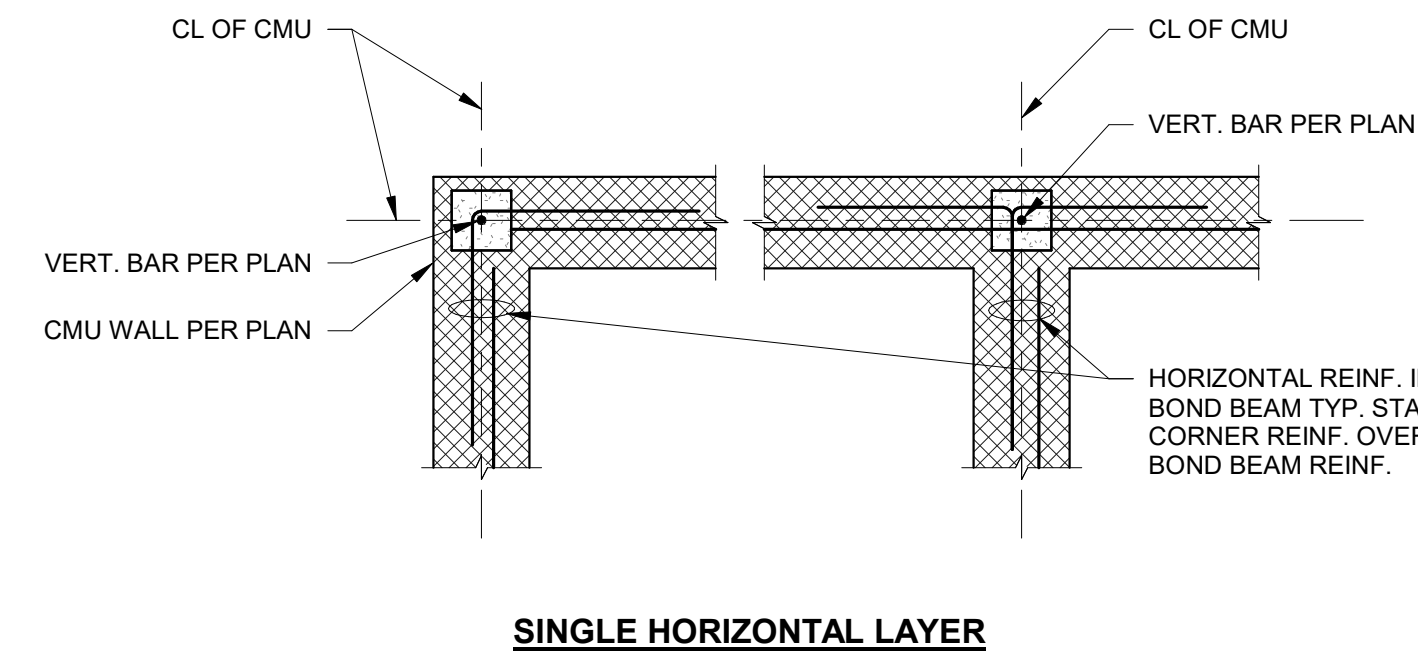
5 TYP. STEP IN SLAB ON GRADE DETAILS  
S402 3/4" = 1'-0"



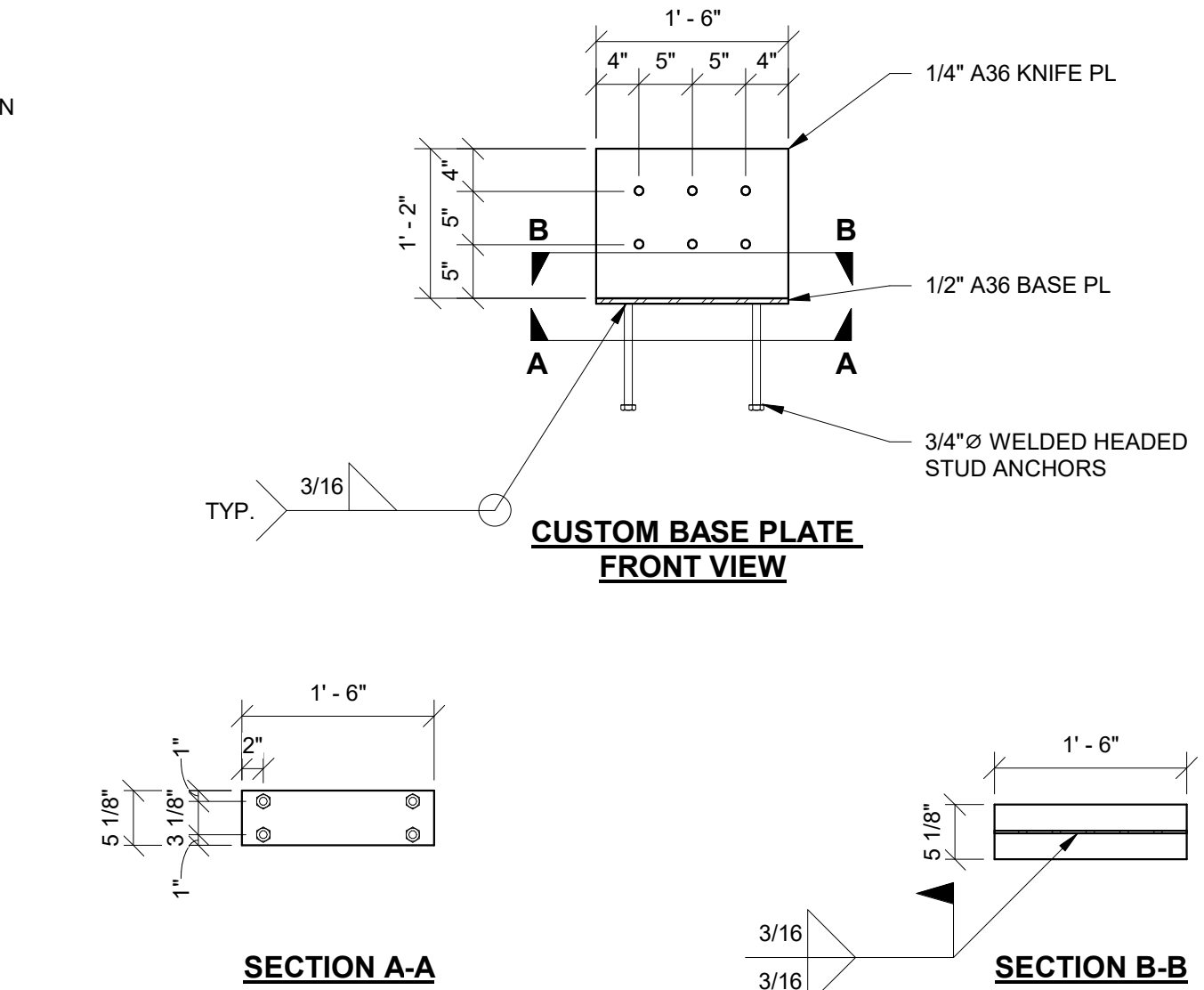
9 GLC BASE PLATE CONNECTION  
S402 3/4" = 1'-0"



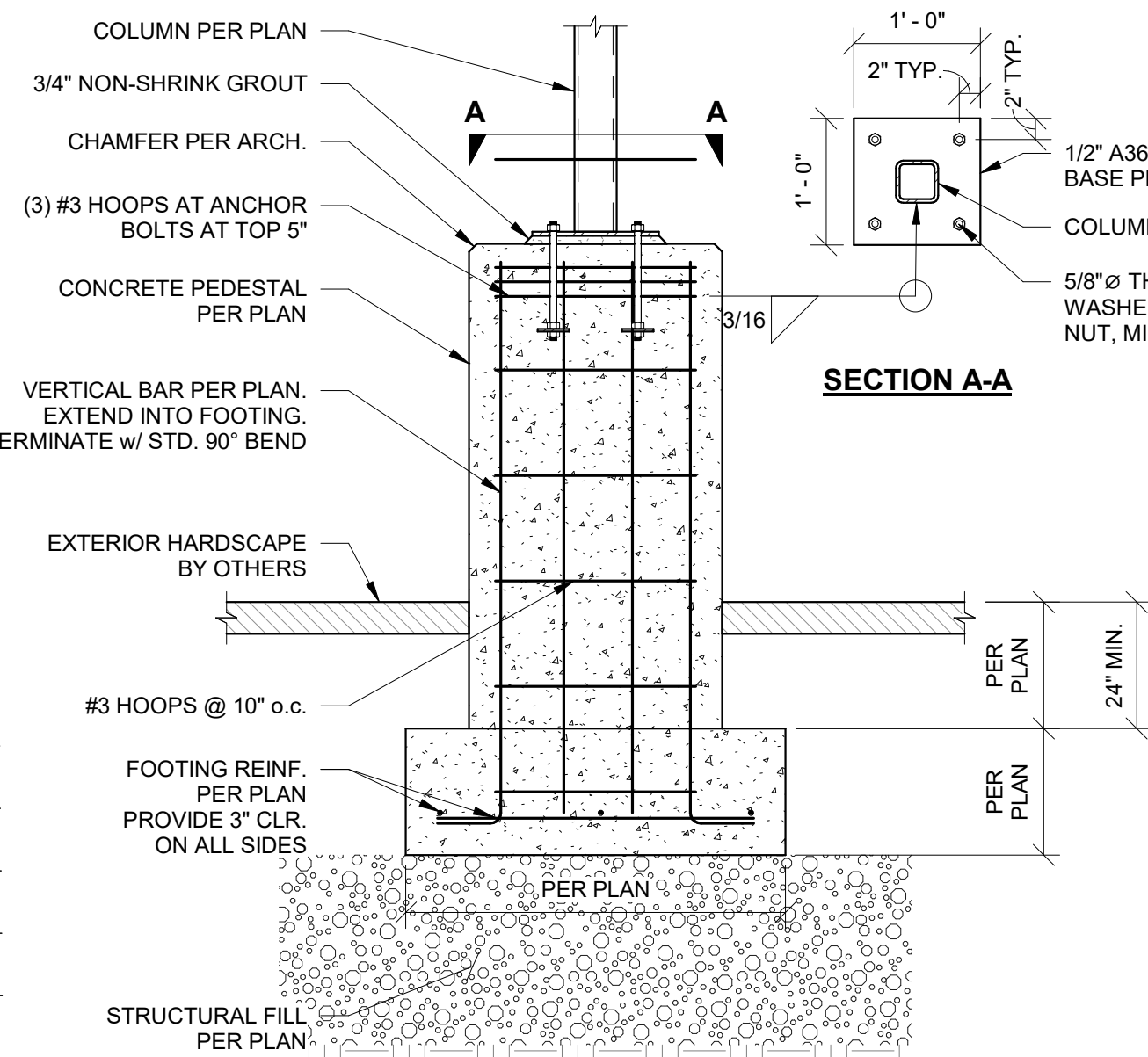
2 TYP. STEPPED WALL FOOTING DETAIL  
S402 3/4" = 1'-0"



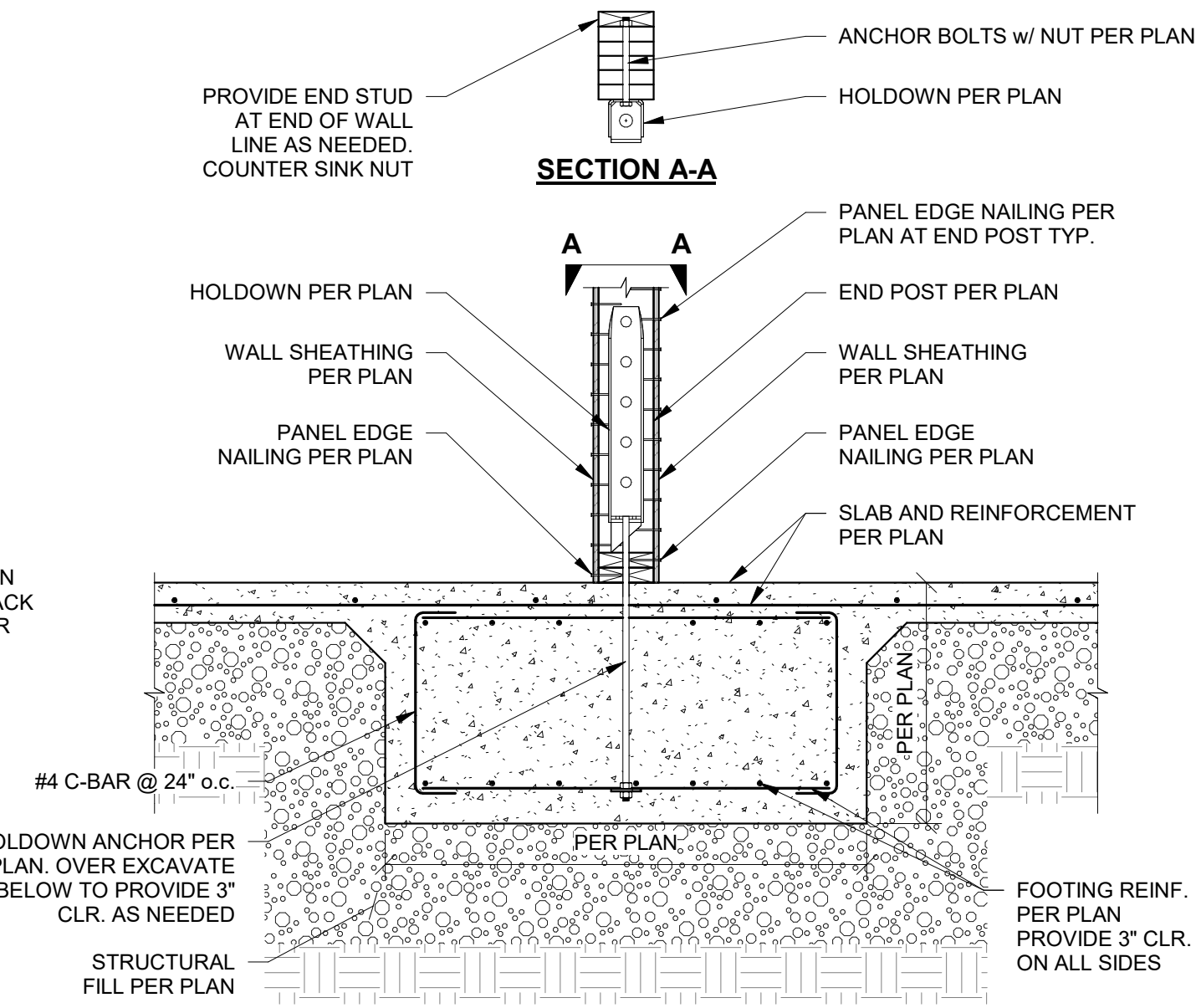
6 TYP. CMU WALL CORNER PLAN DETAILS  
S402 3/4" = 1'-0"



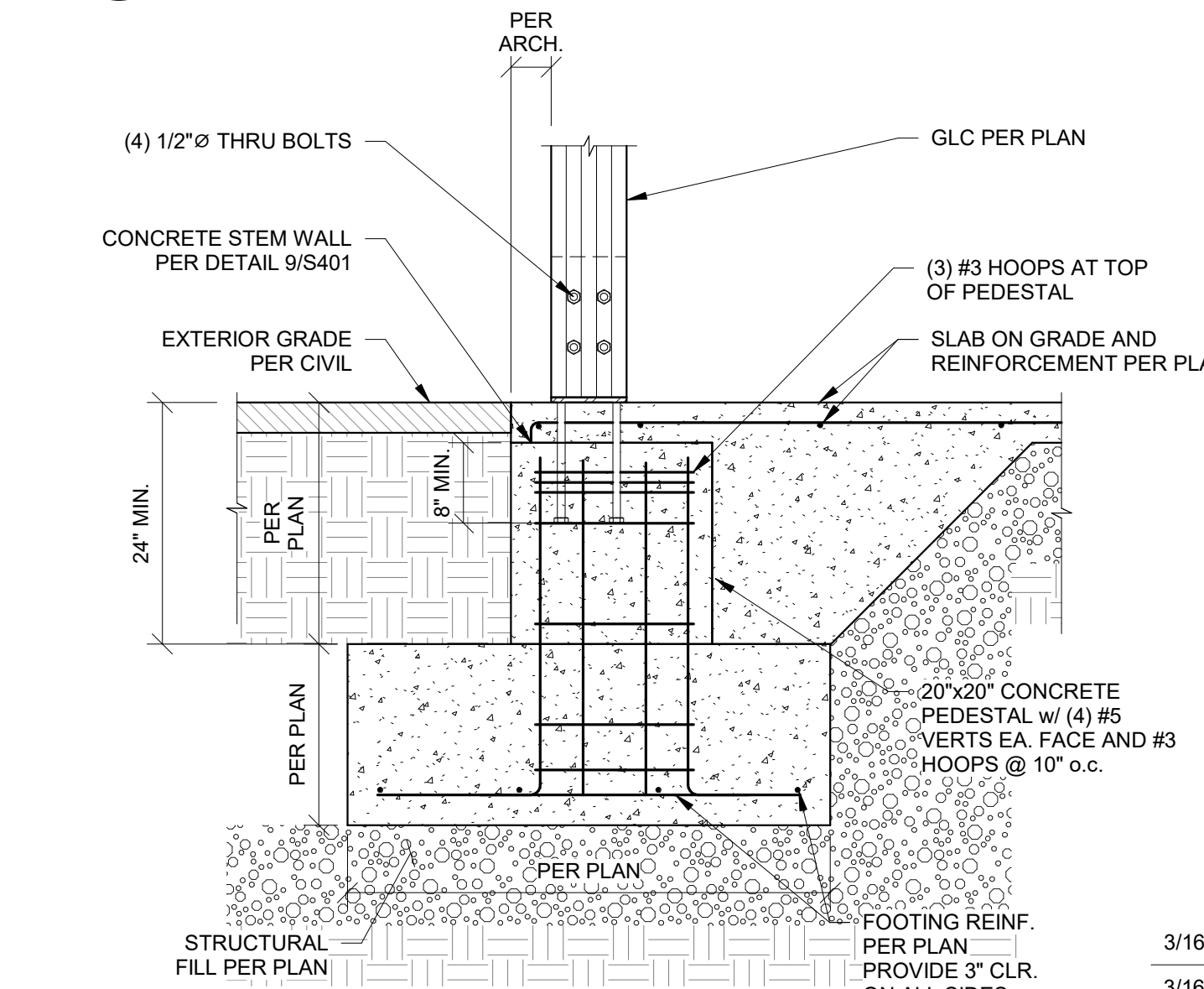
7 INTERIOR SHEAR WALL FOOTING  
S402 3/4" = 1'-0"



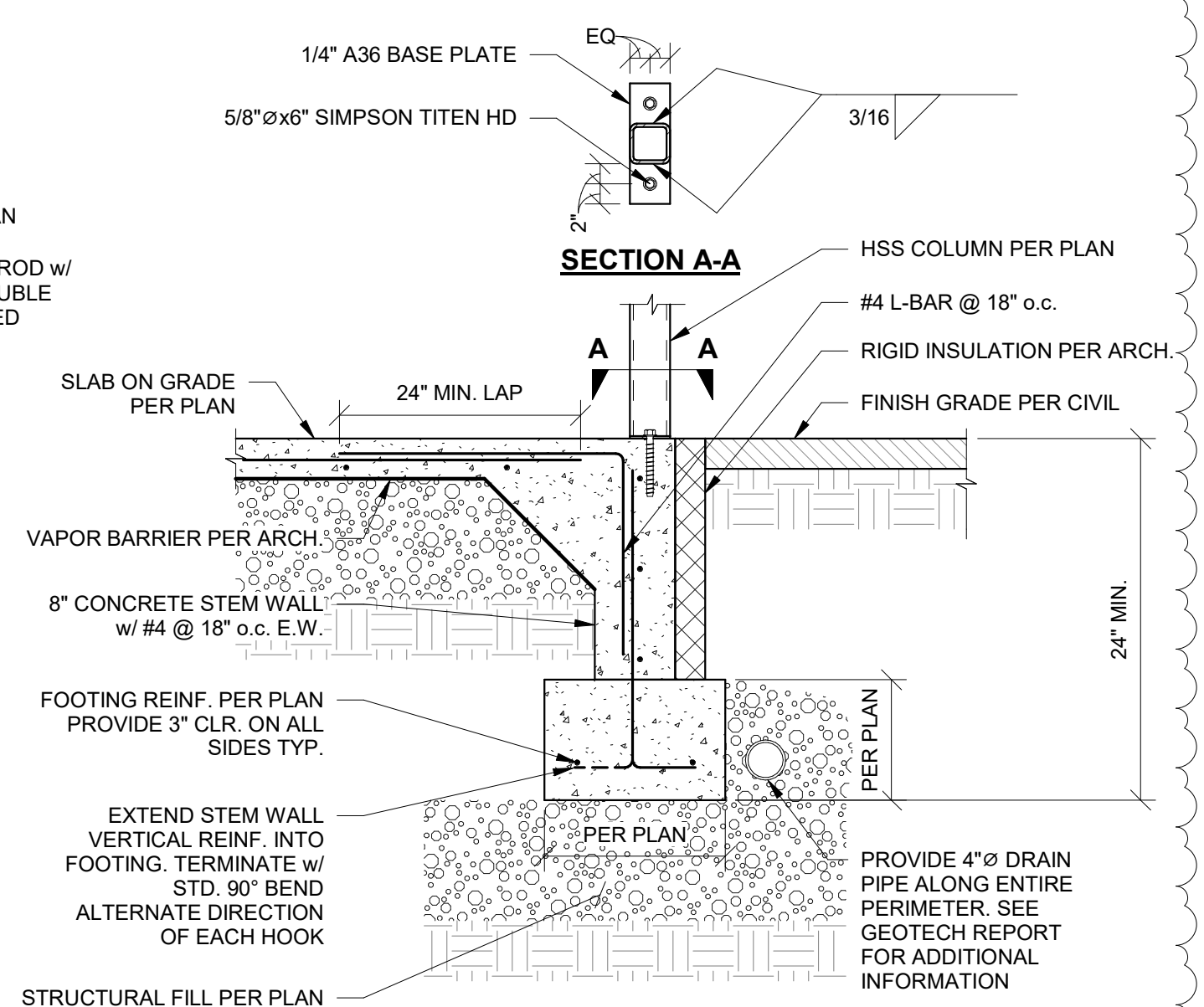
10 GLC BASE PLATE CONNECTION  
S402 3/4" = 1'-0"



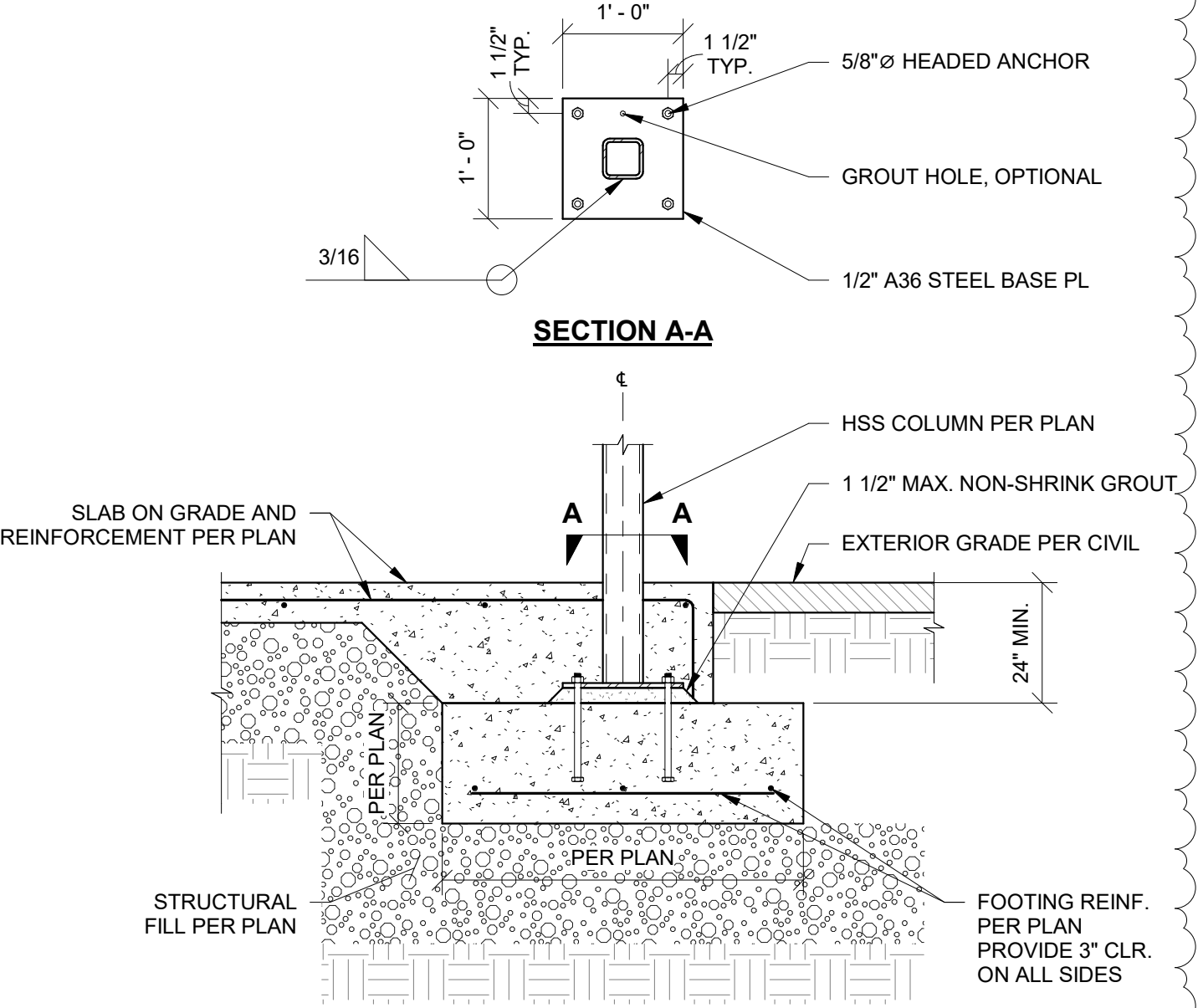
7 INTERIOR SHEAR WALL FOOTING  
S402 3/4" = 1'-0"



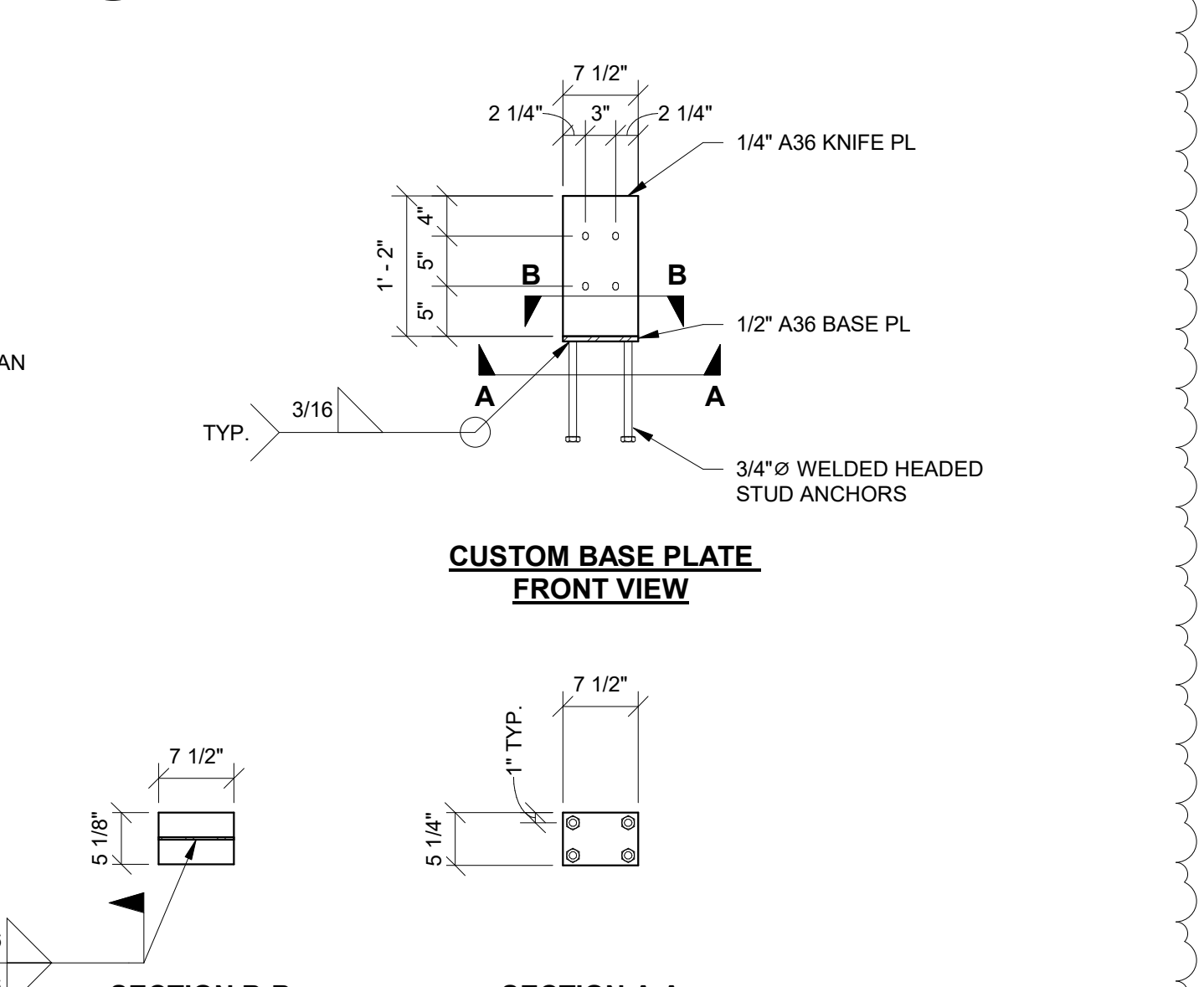
10 GLC BASE PLATE CONNECTION  
S402 3/4" = 1'-0"



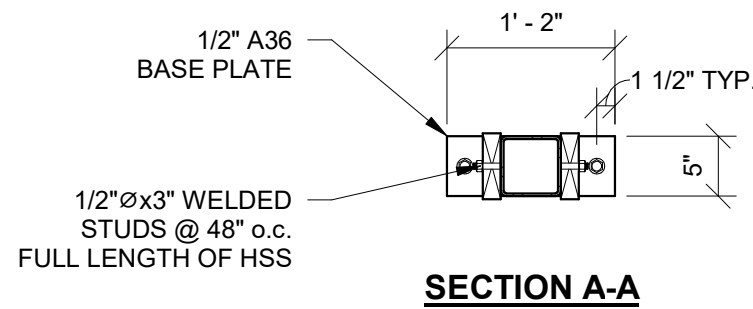
4 HSS COLUMN AT PERIMETER FOOTING  
S402 3/4" = 1'-0"



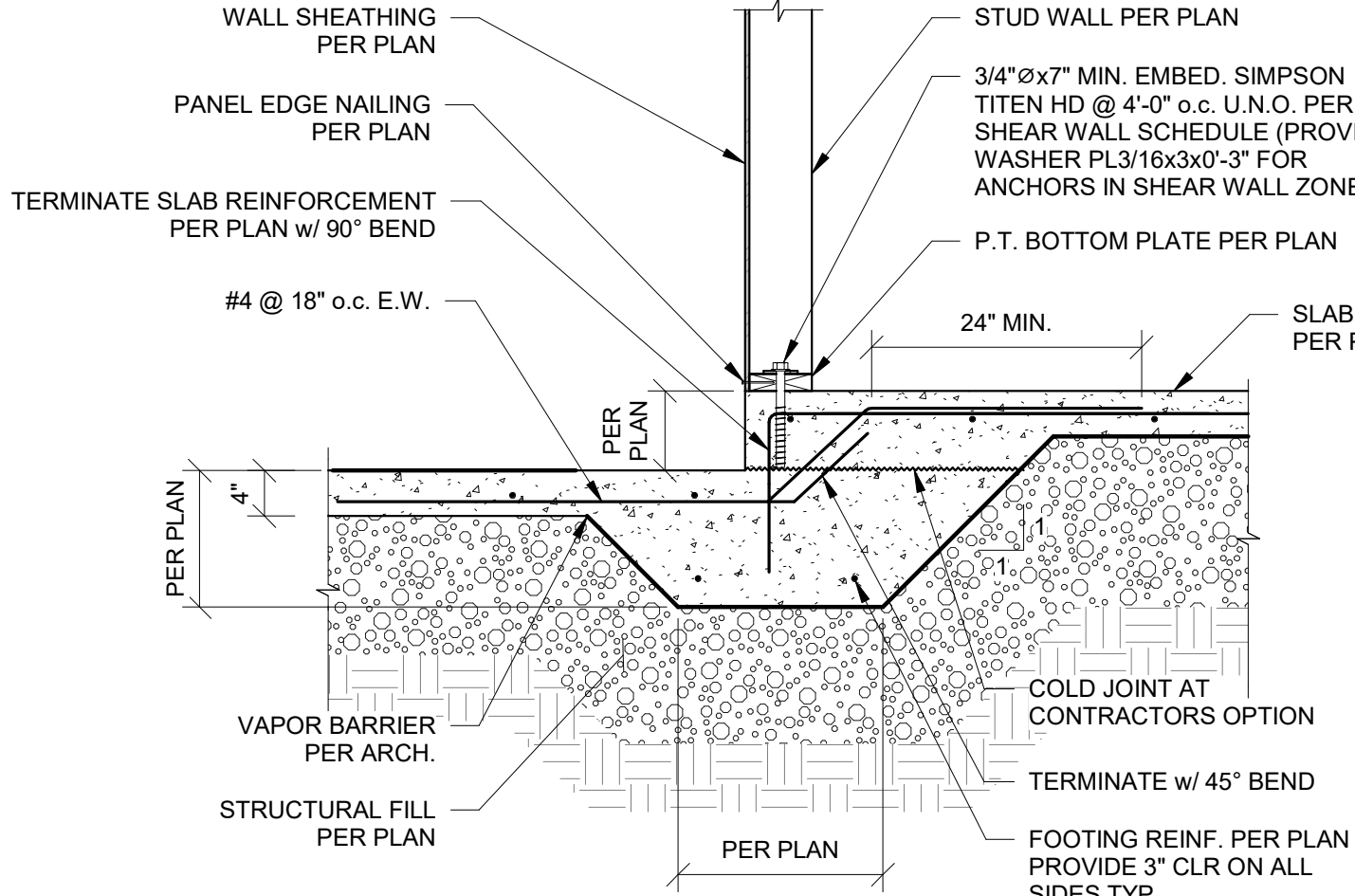
8 EXTERIOR HSS COLUMN BASE CONNECTION  
S402 3/4" = 1'-0"



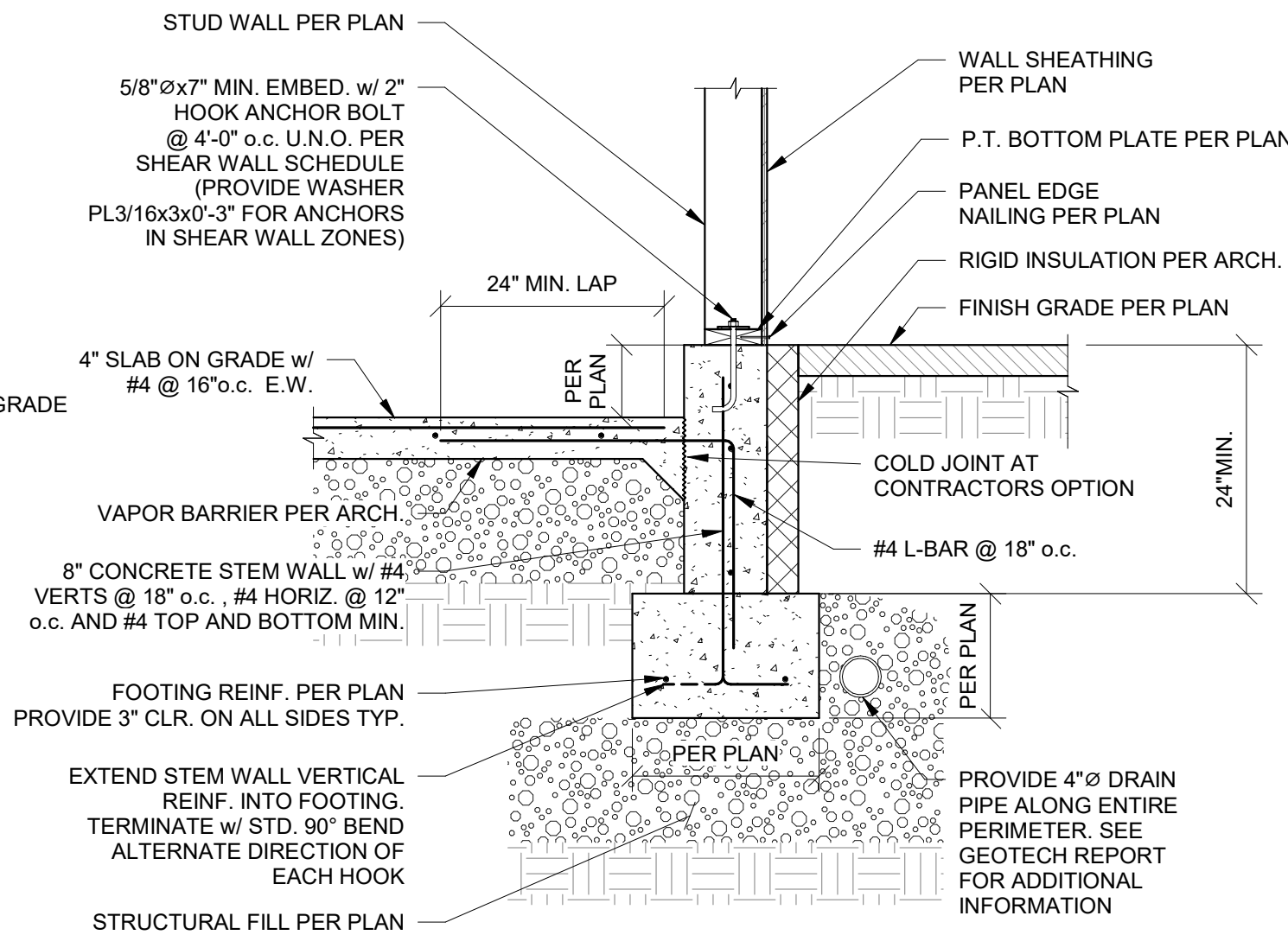
8 EXTERIOR HSS COLUMN BASE CONNECTION  
S402 3/4" = 1'-0"



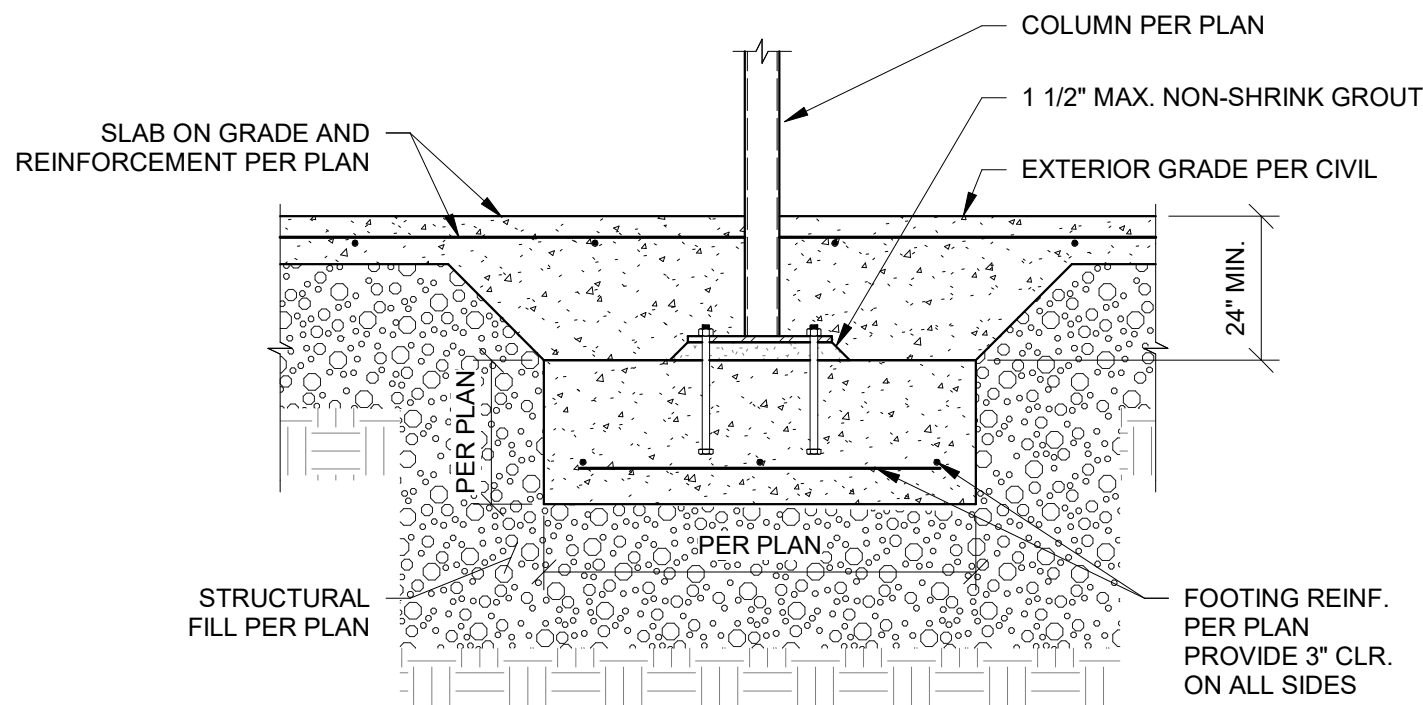
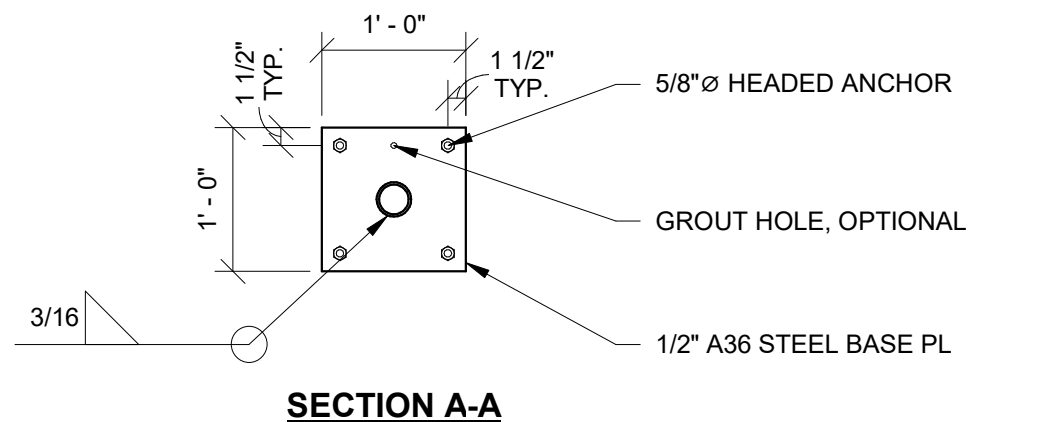
1 STOREFRONT HSS TO FOUNDATION  
S403 3/4" = 1'-0"



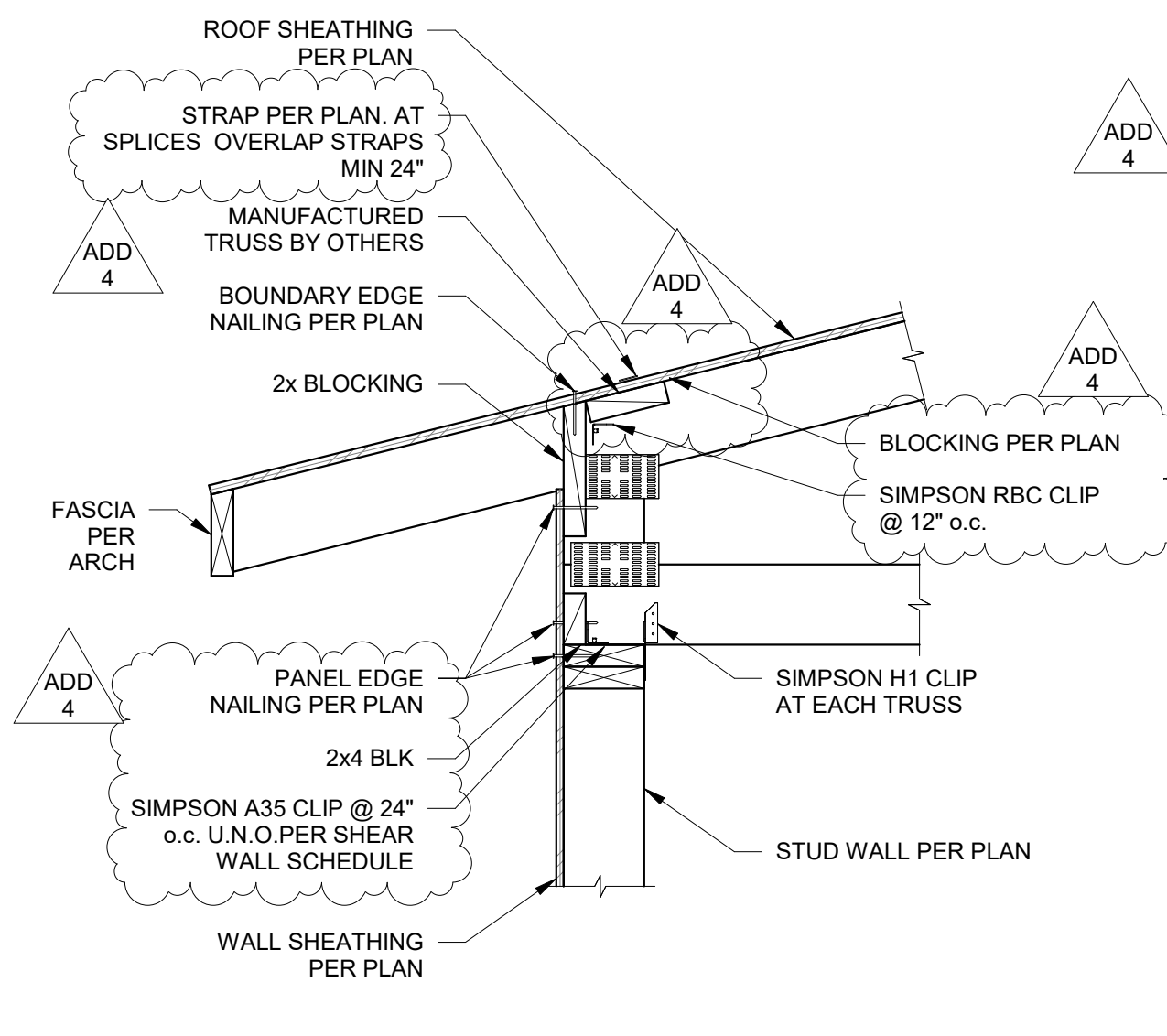
2 TYP. STEP IN SLAB ON GRADE AT FOOTING  
S403 3/4" = 1'-0"



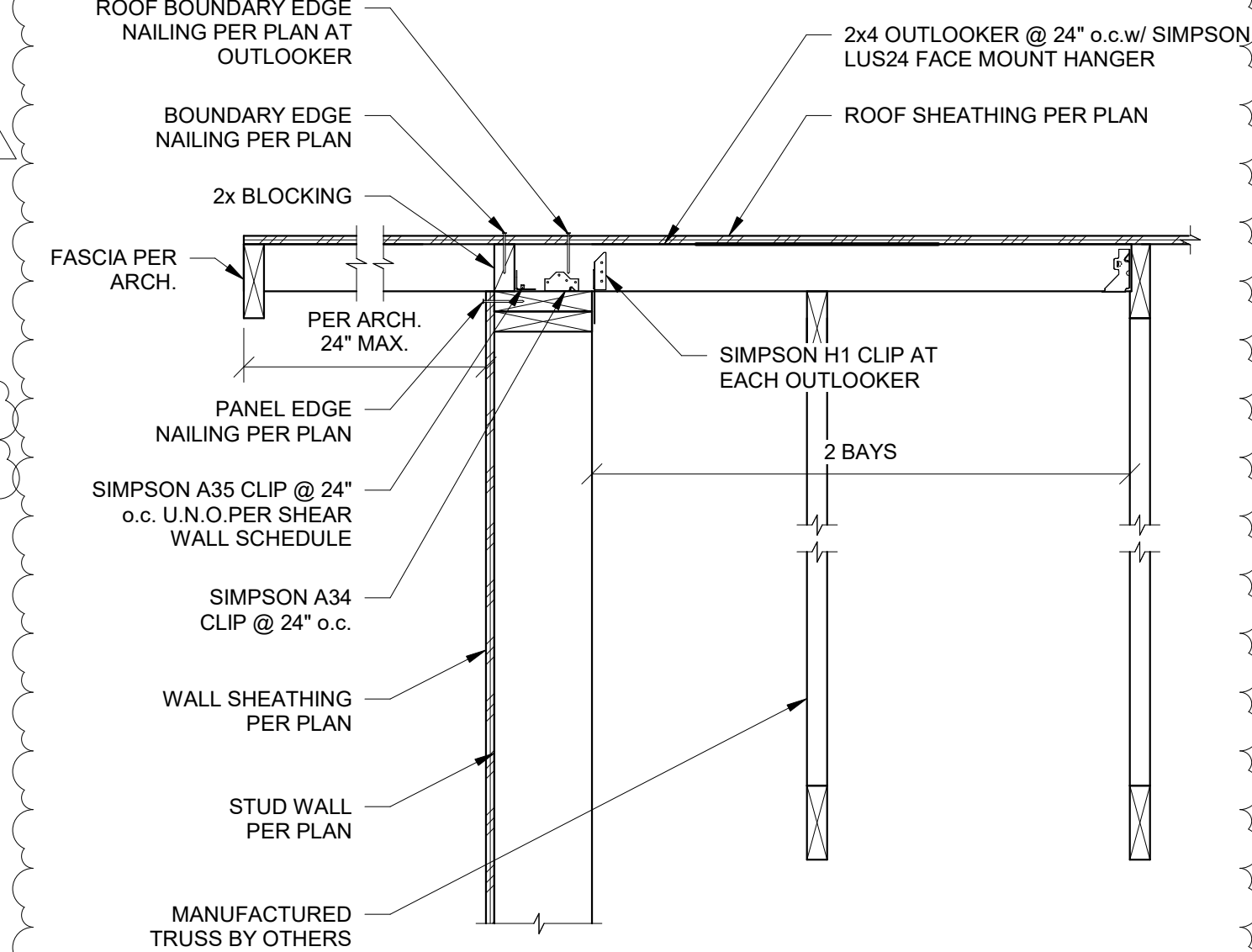
3 STEP IN SLAB ON GRADE AT EXT. FTG  
S403 3/4" = 1'-0"



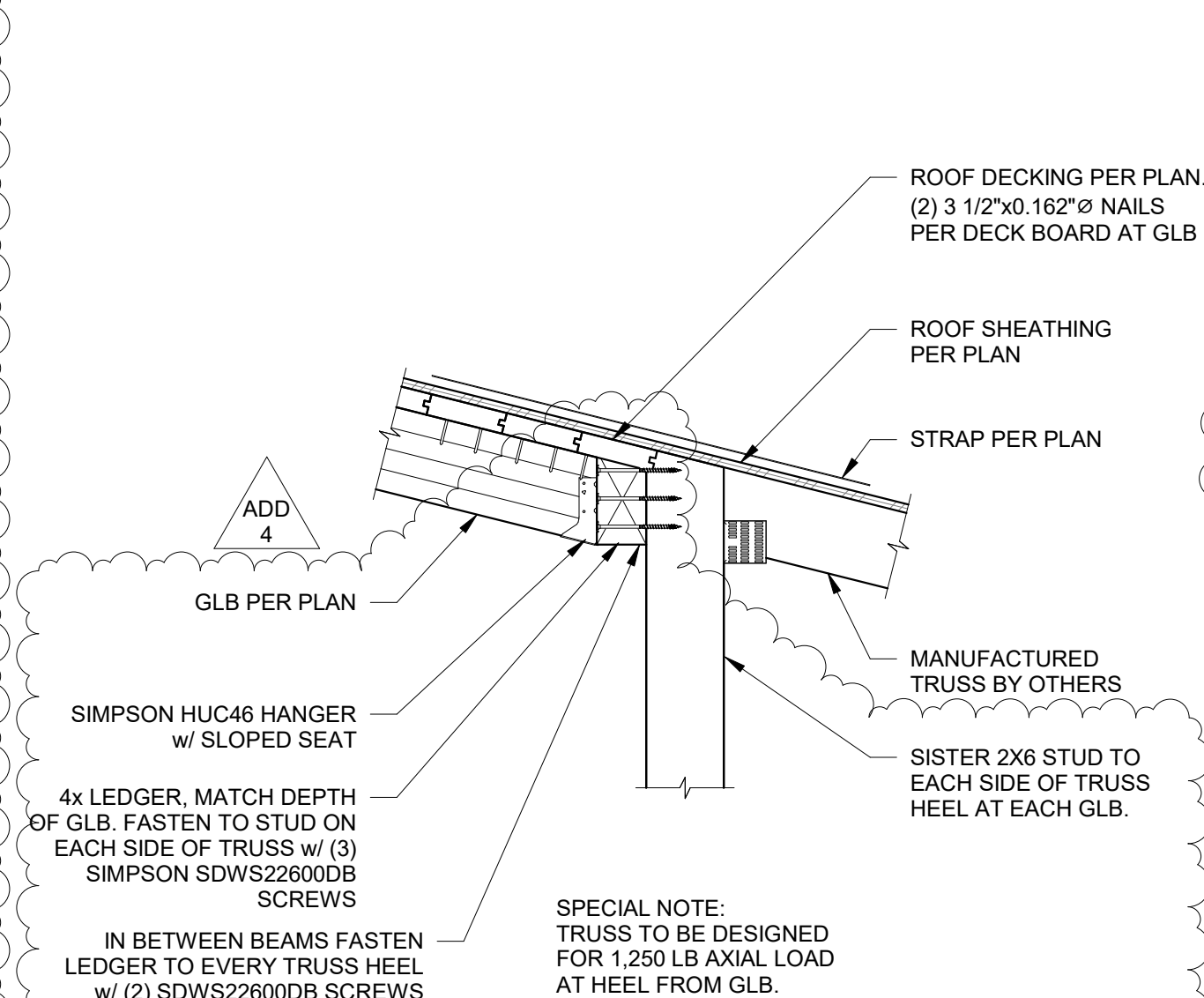
4 EXTERIOR PIPE COLUMN BASE CONNECTION  
S403 3/4" = 1'-0"



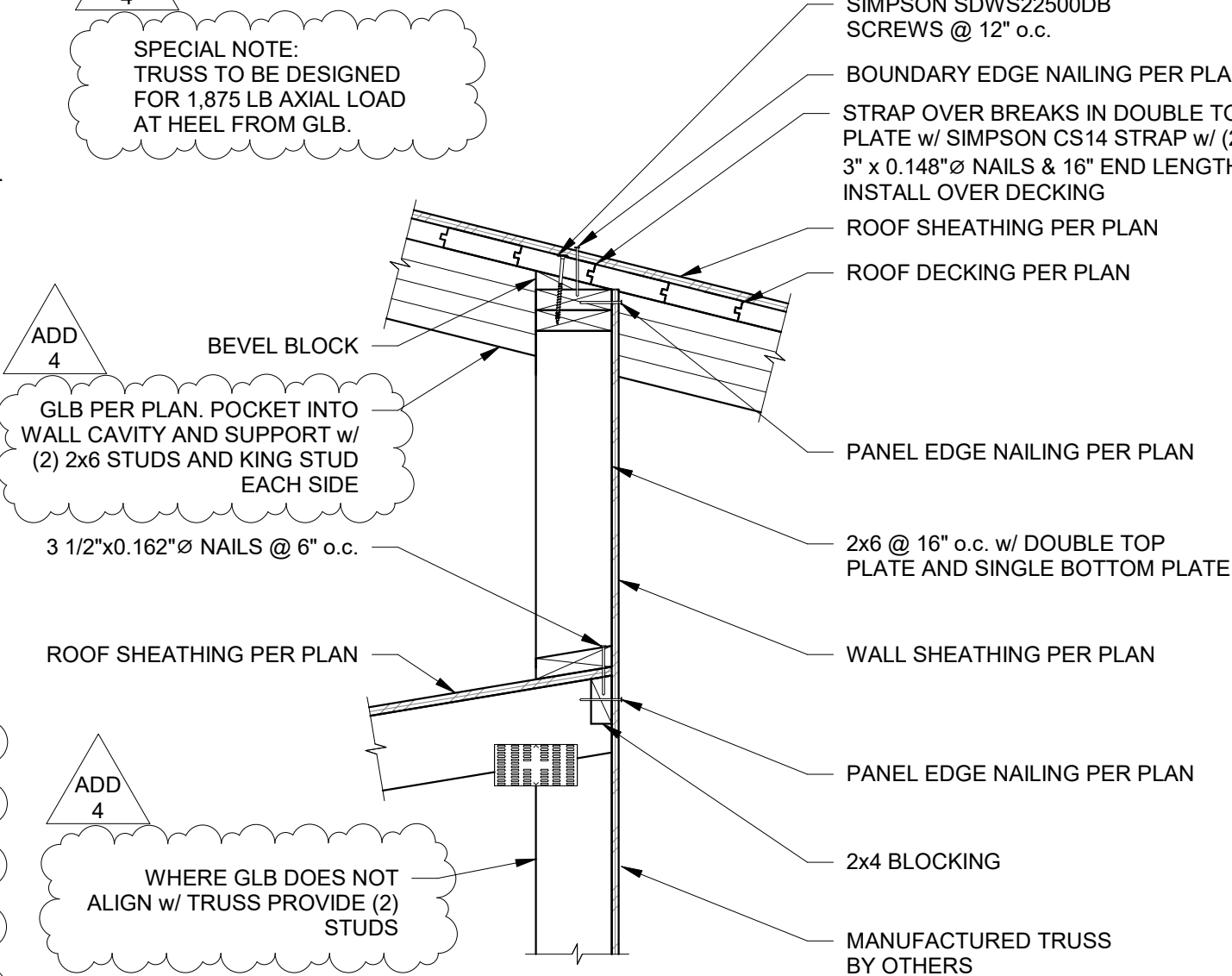
1 TYP. HEEL TRUSS AT EXTERIOR SHEAR WALL  
S501 1" = 1'-0"



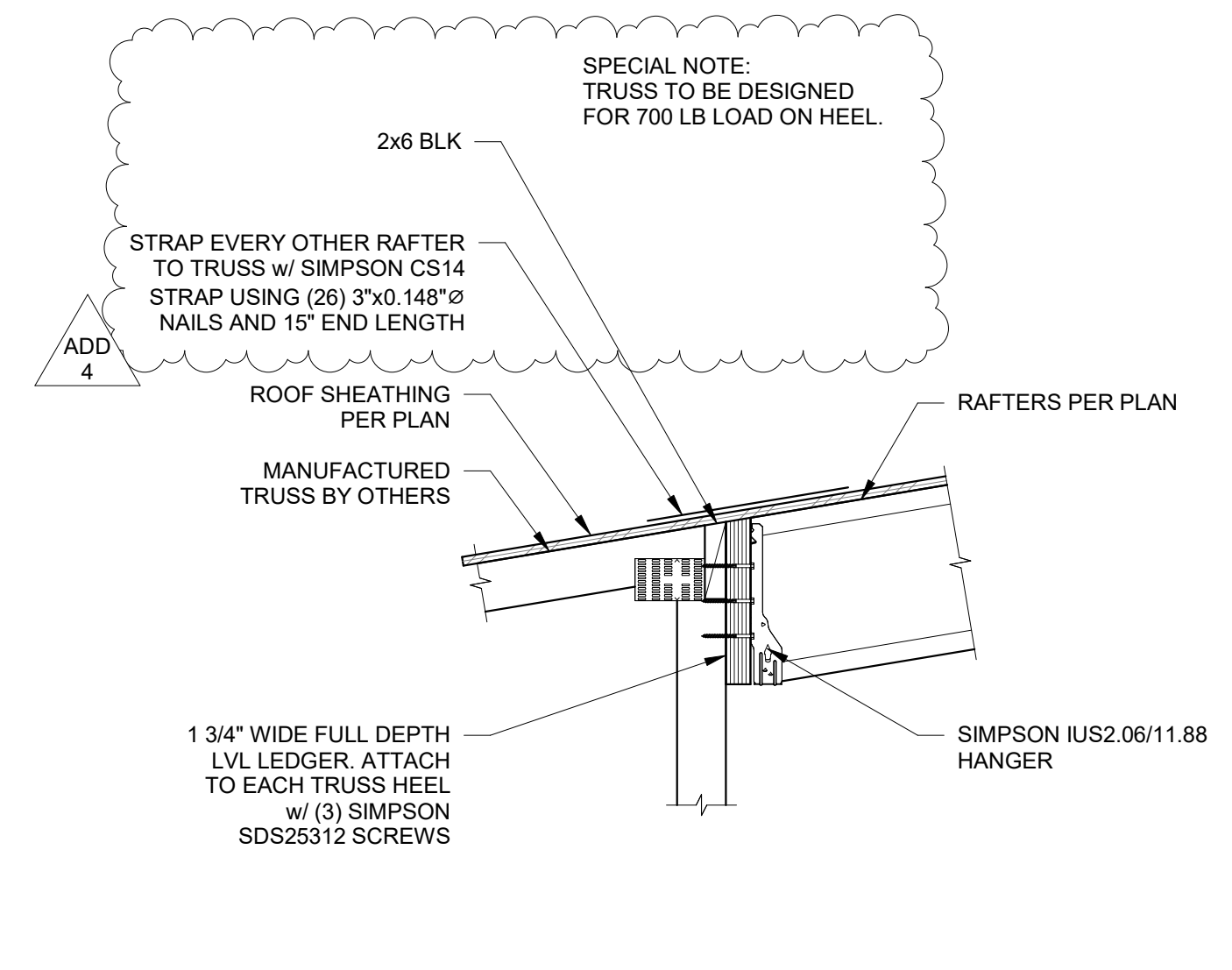
2 RAKE WALL GABLE END w/ OUTLOOKERS  
S501 1" = 1'-0"



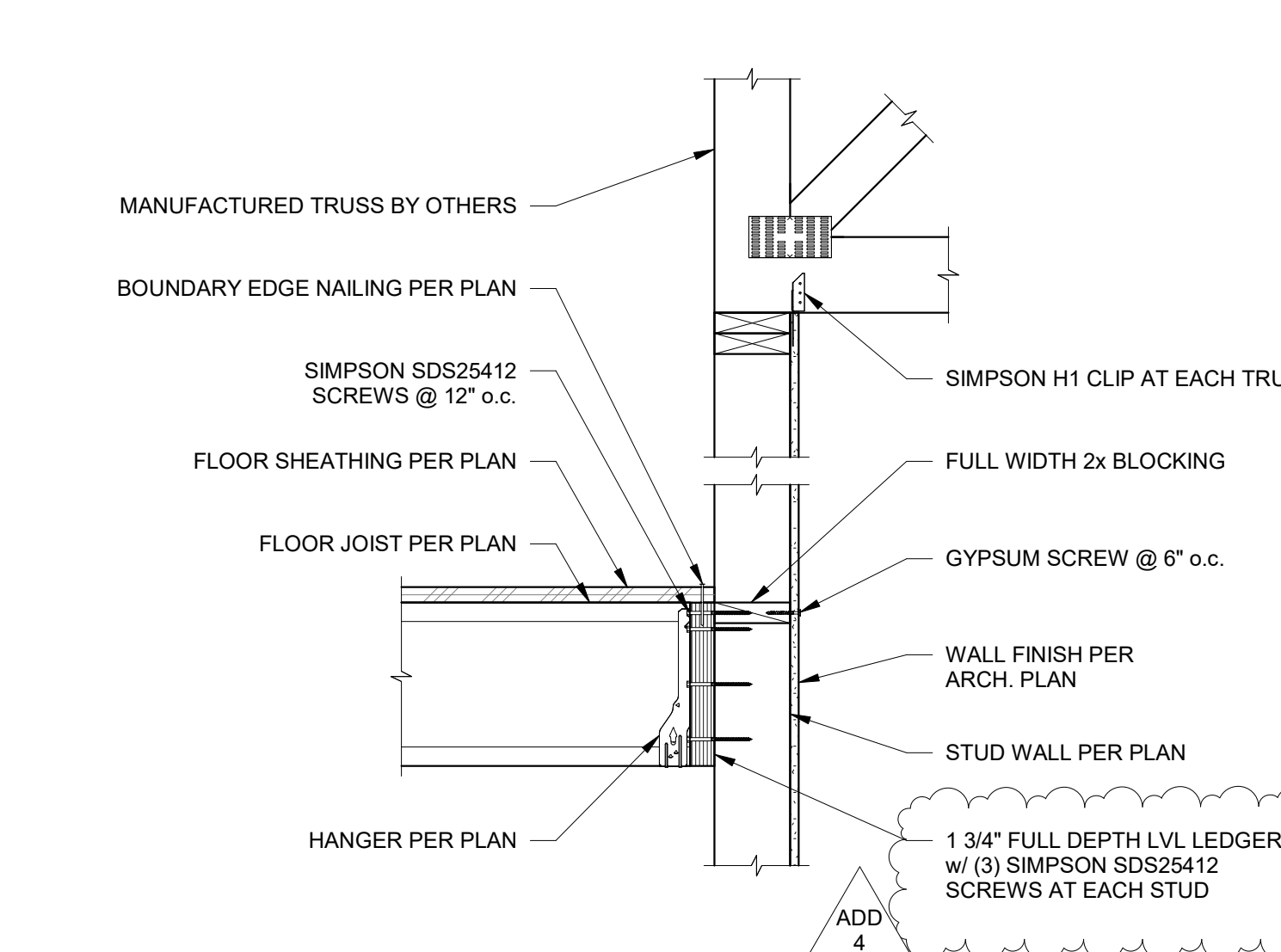
3 HALLWAY GLB TO TRUSS HEEL  
S501 1" = 1'-0"



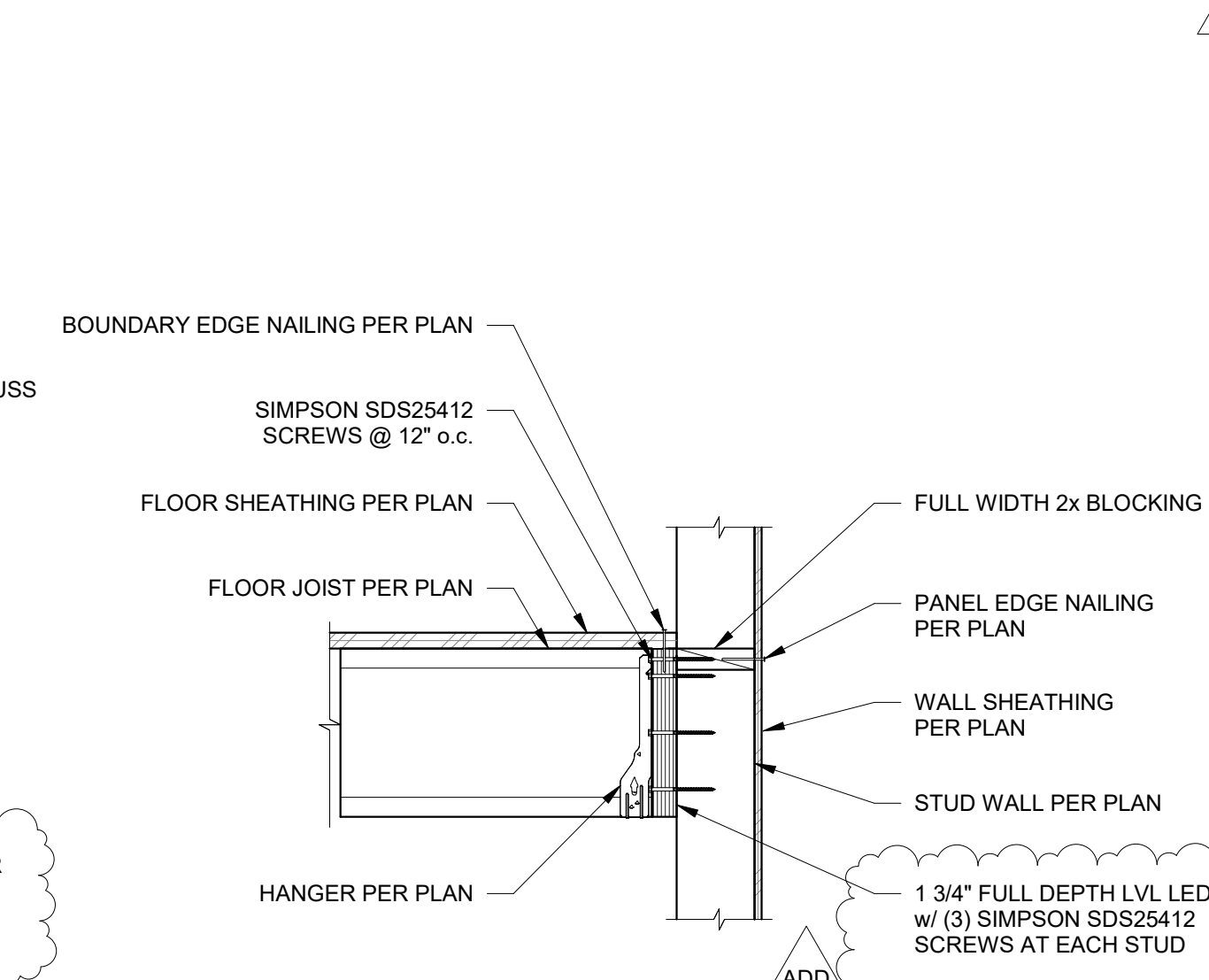
4 STUD WALL TO PERPENDICULAR GLB  
S501 1" = 1'-0"



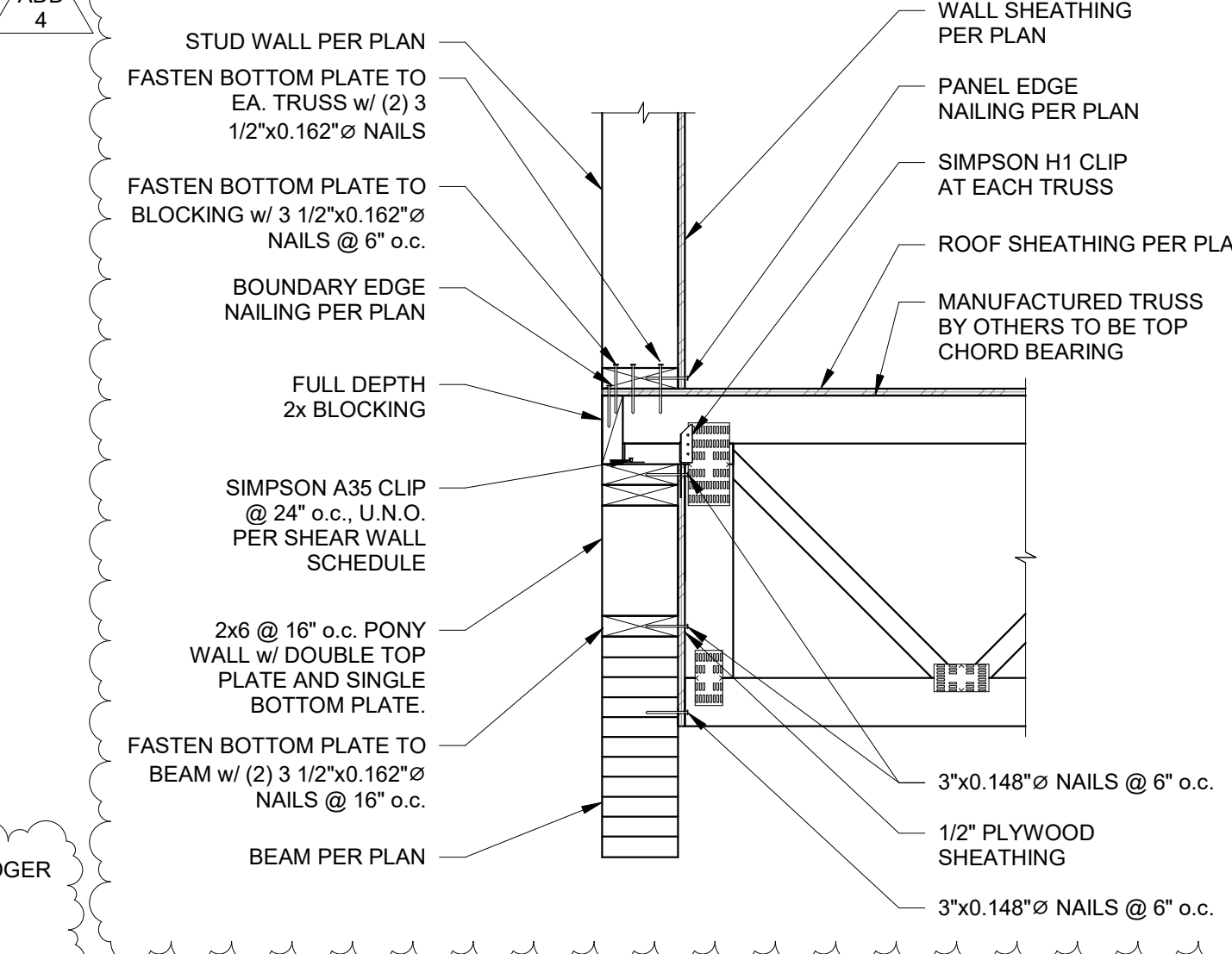
5 RAFTERS TO PARALLEL TRUSS  
S501 1" = 1'-0"



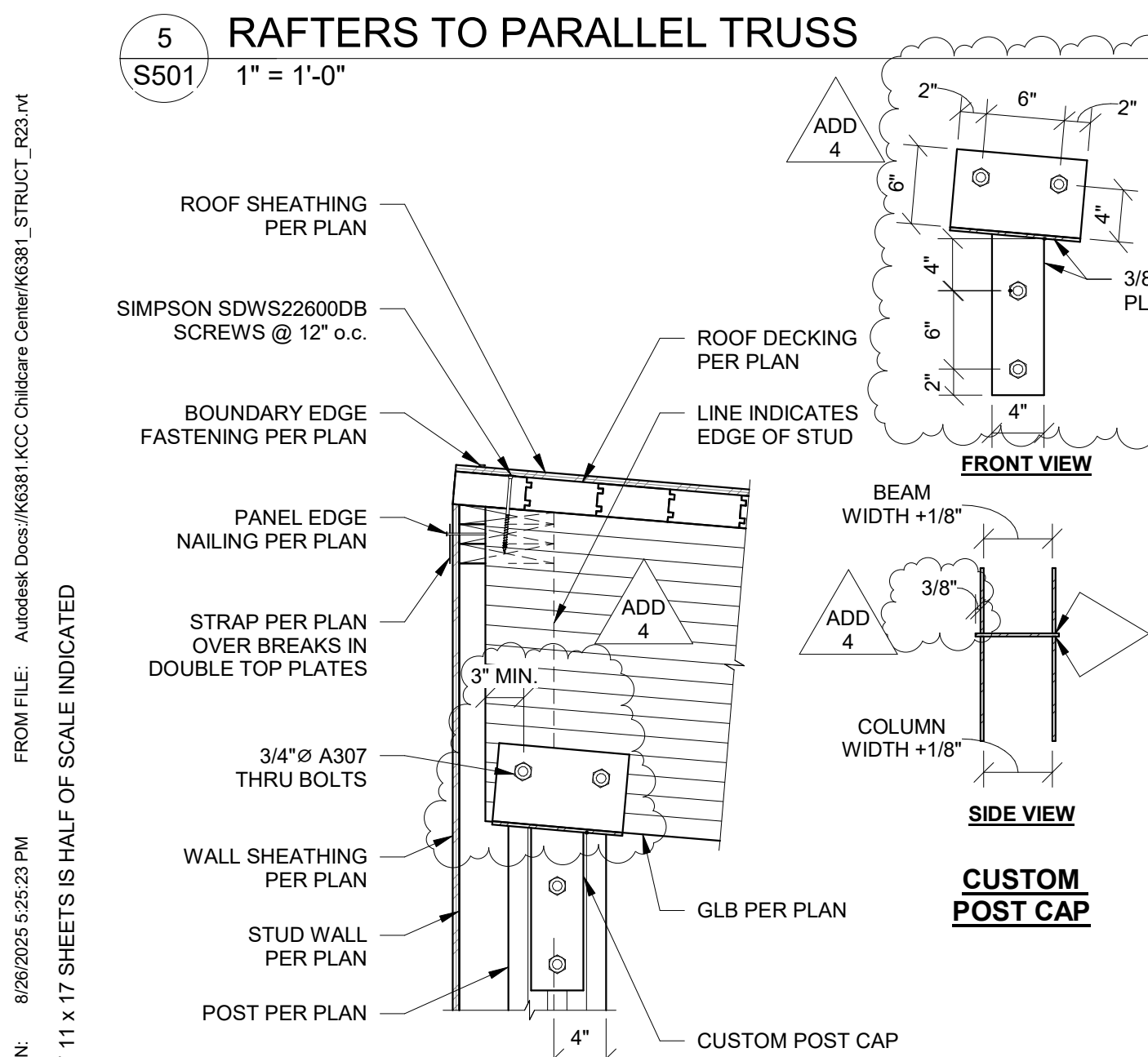
6 MEZZANINE JOIST TO STUD WALL  
S501 1" = 1'-0"



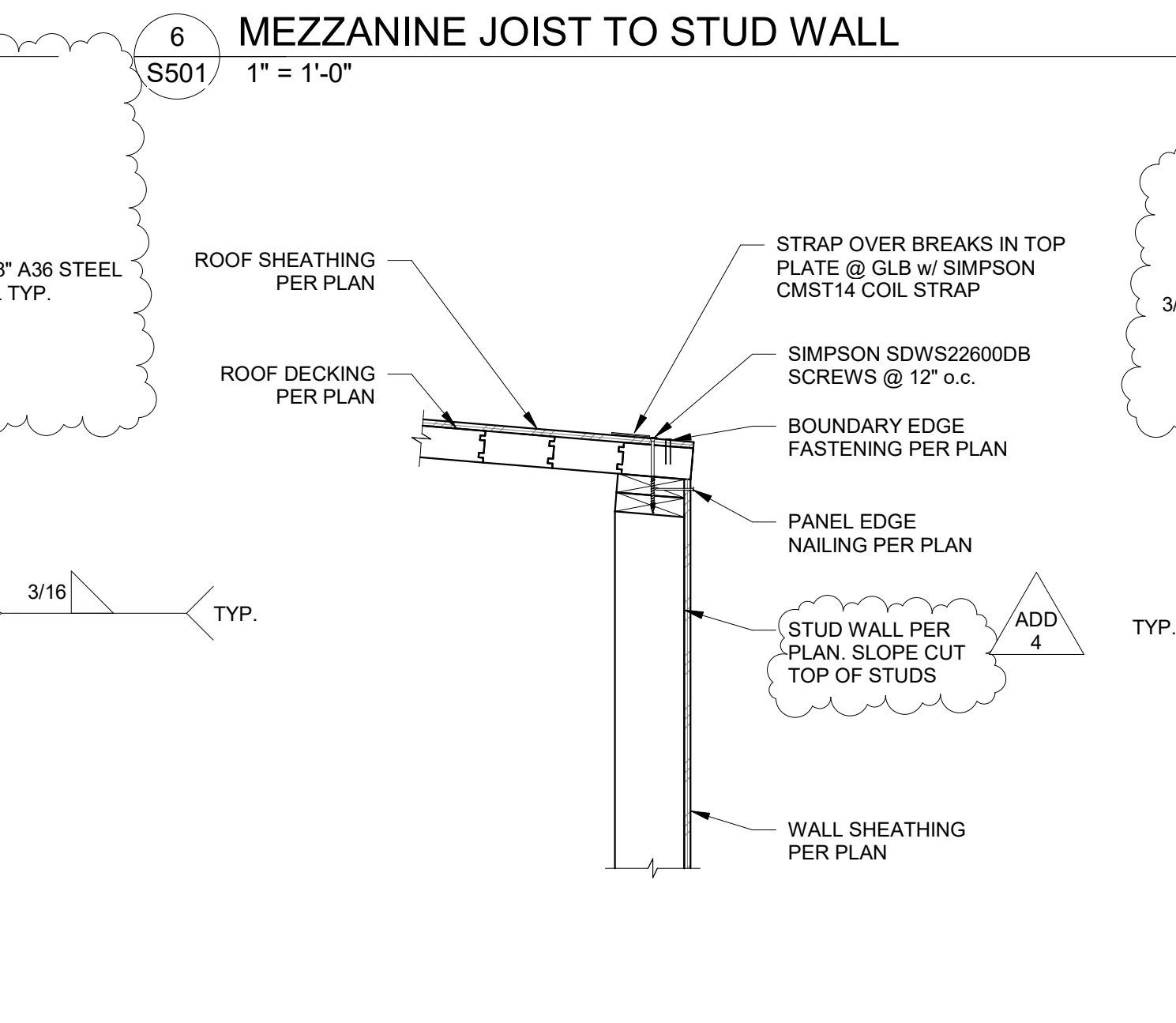
7 MEZZ. JOIST TO STUD WALL AT SHEAR WALL  
S501 1" = 1'-0"



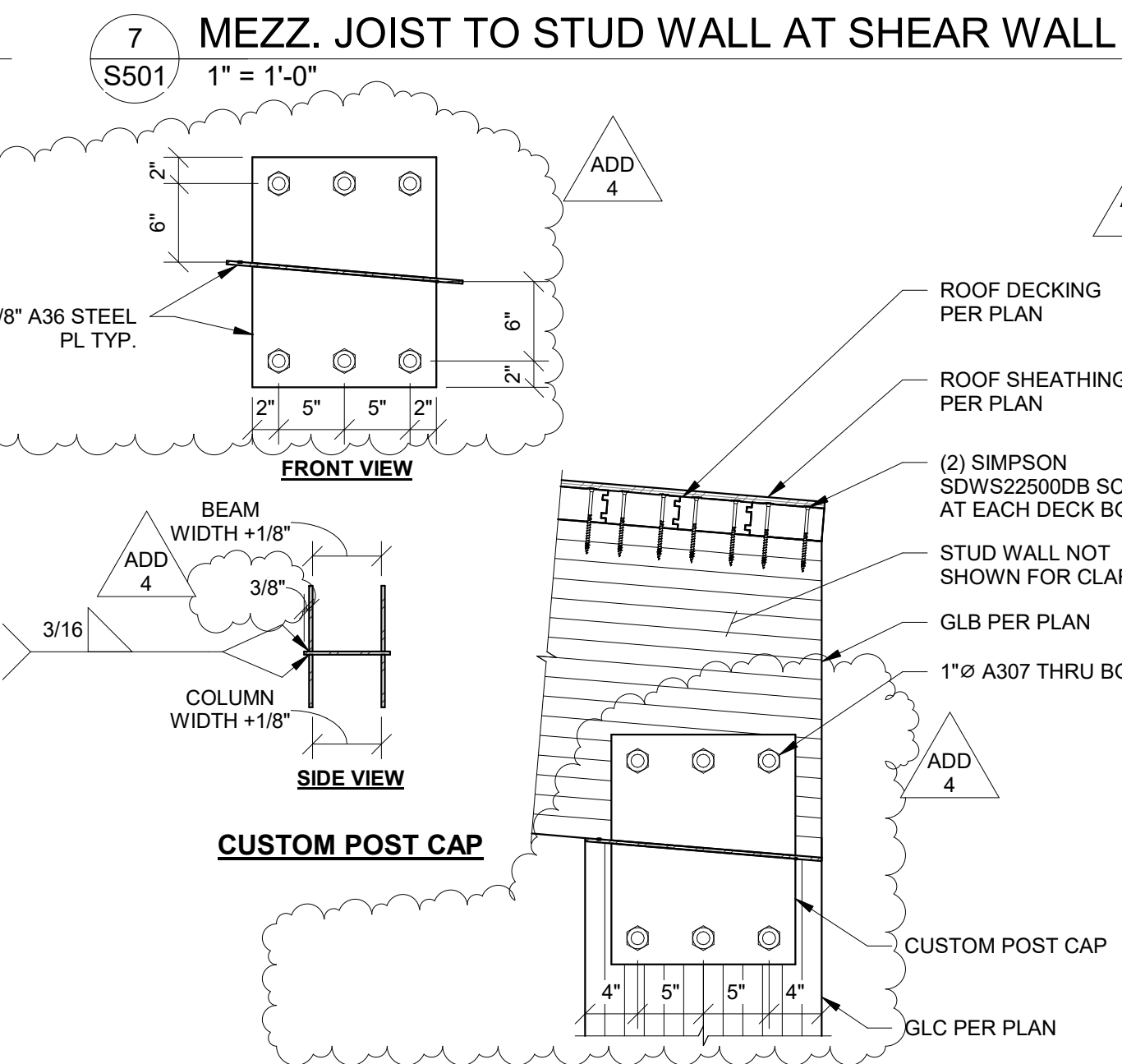
8 BEAM TO COLUMN CONNECTION AT LOW ROOF  
S501 1" = 1'-0"



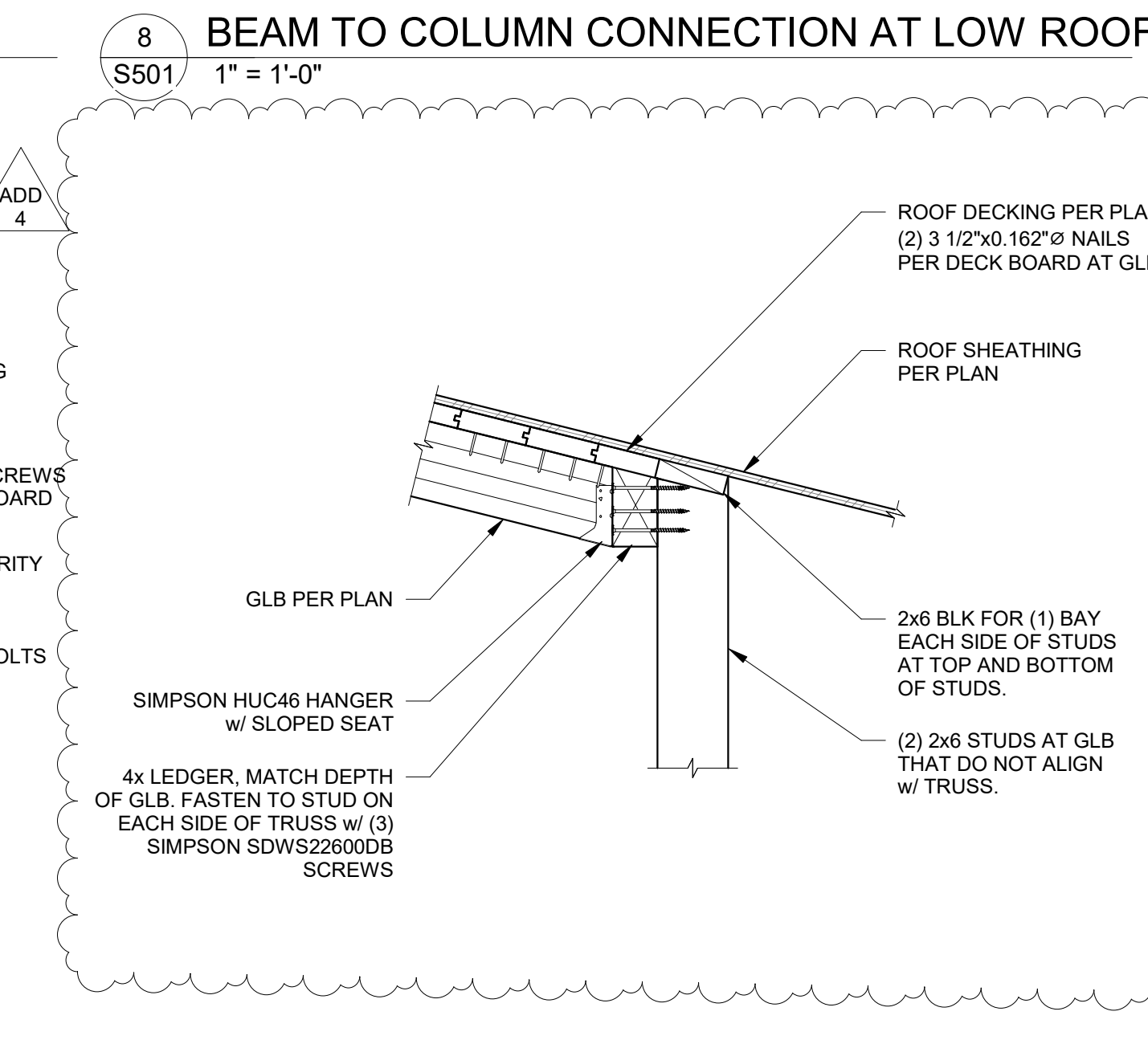
9 ROOF TO BEAM CONNECTION  
S501 1" = 1'-0"



10 ROOF DECK TO SHEAR WALL  
S501 1" = 1'-0"

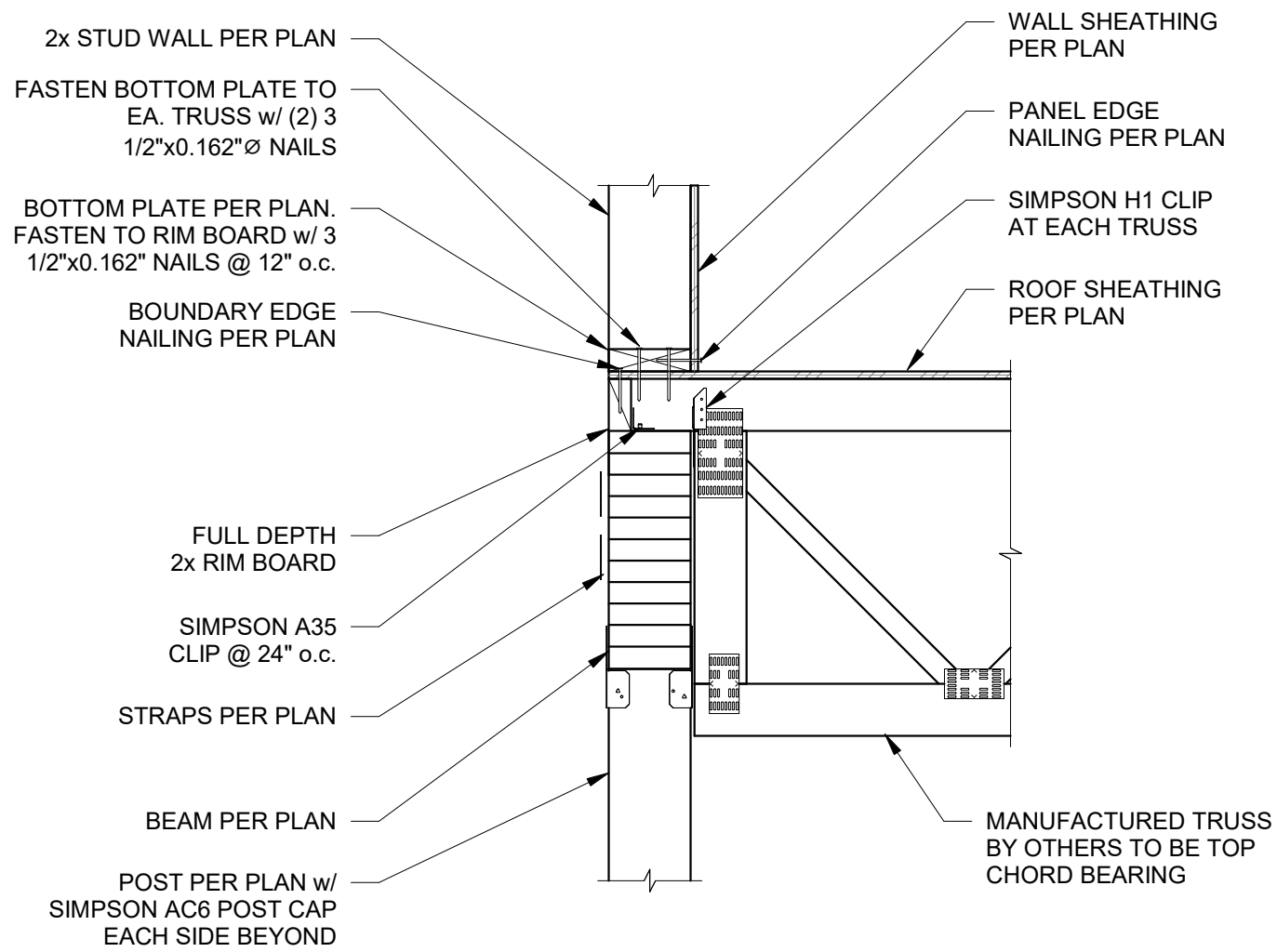


11 ROOF TO BEAM CONNECTION  
S501 1" = 1'-0"

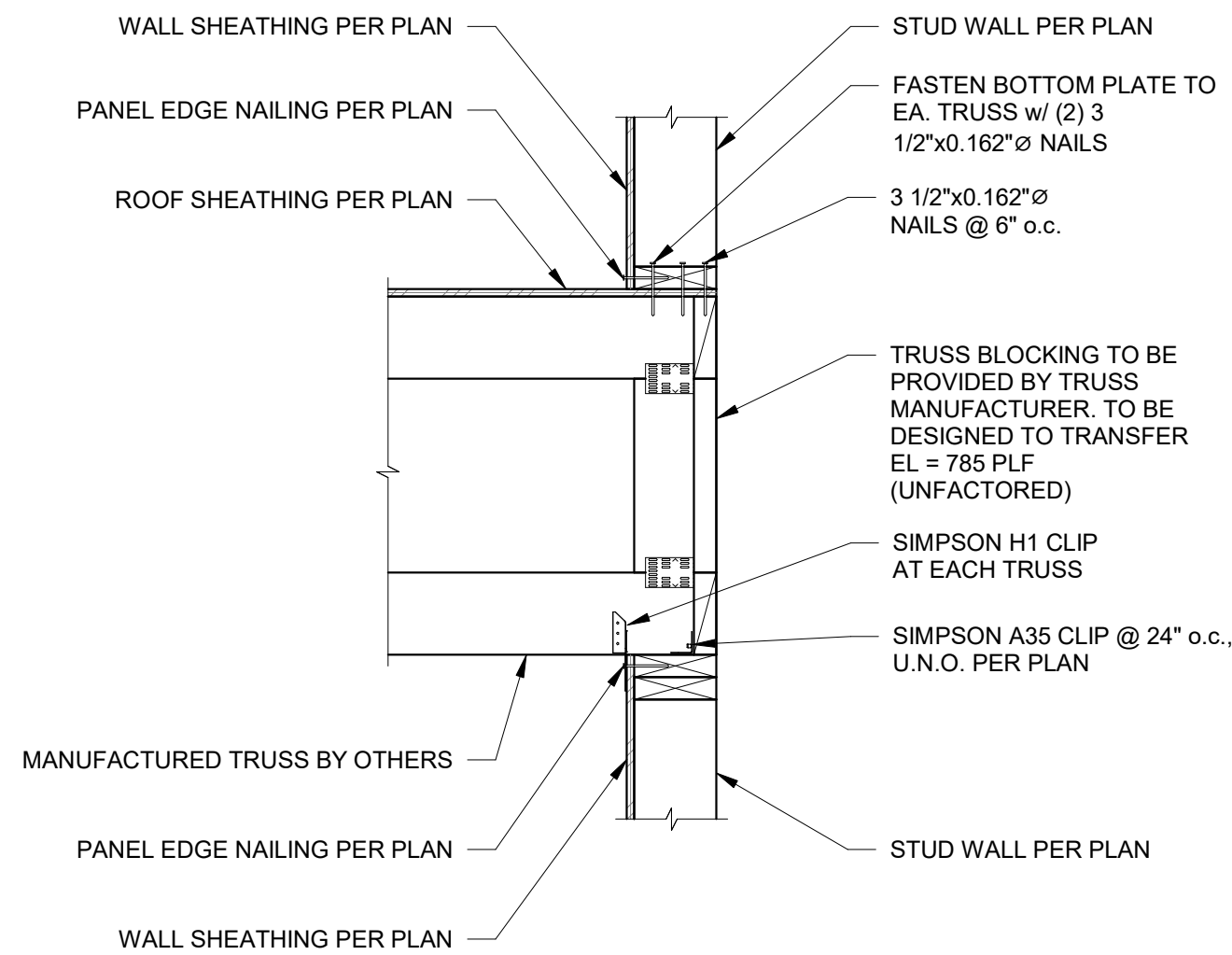


12 HALLWAY GLB OFFSET w/ TRUSS  
S501 1" = 1'-0"

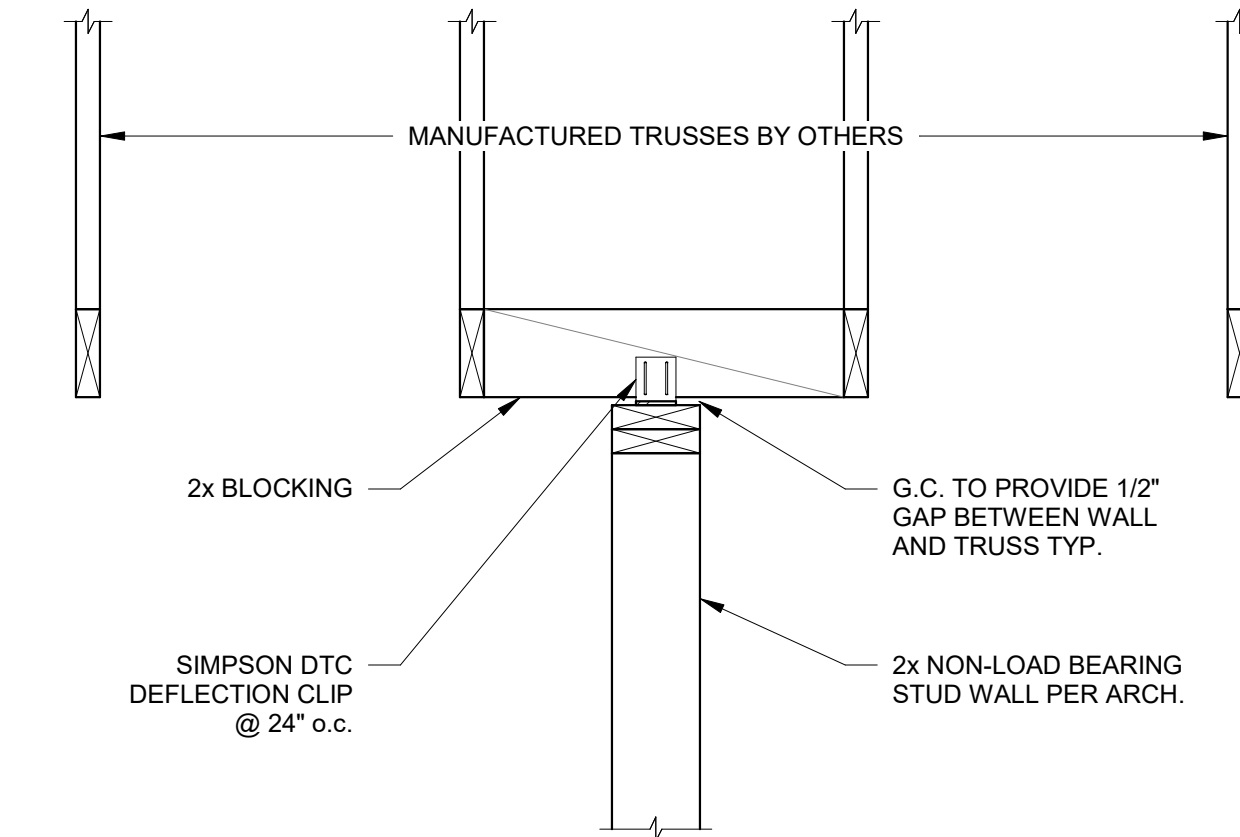




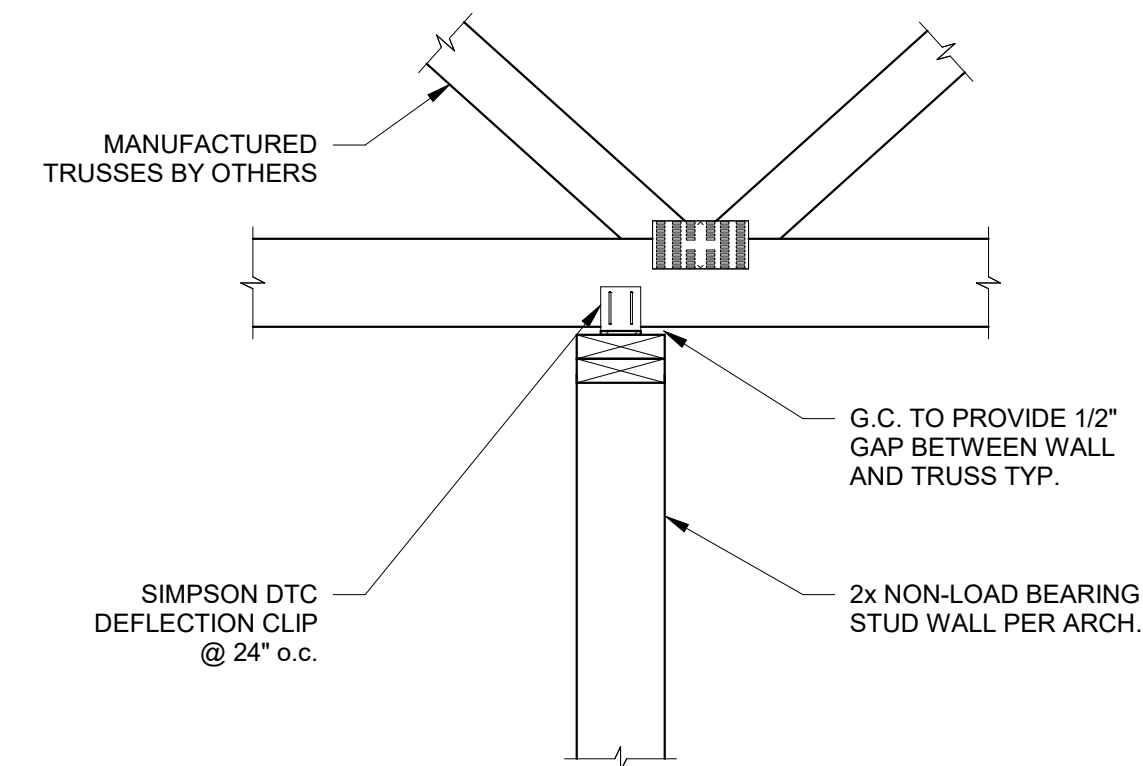
1 TRUSS TO BEAM CONNECTION  
S502 1" = 1'-0"



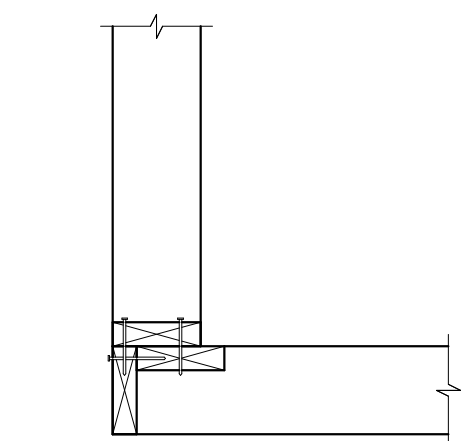
2 LOW ROOF TRUSS TO WALL CONNECTION  
S502 1" = 1'-0"



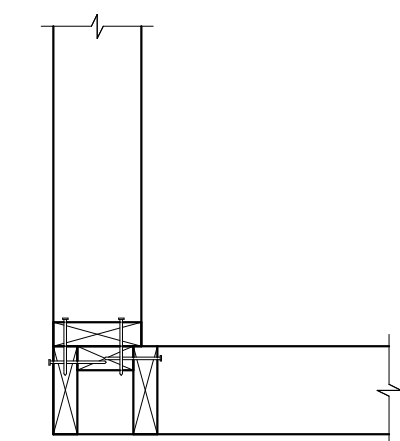
3 TRUSS PARALLEL TO PARTITION  
S502 1" = 1'-0"



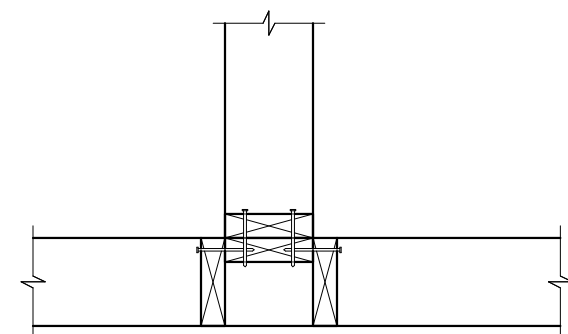
4 TRUSS PERP. TO PARTITION  
S502 1" = 1'-0"



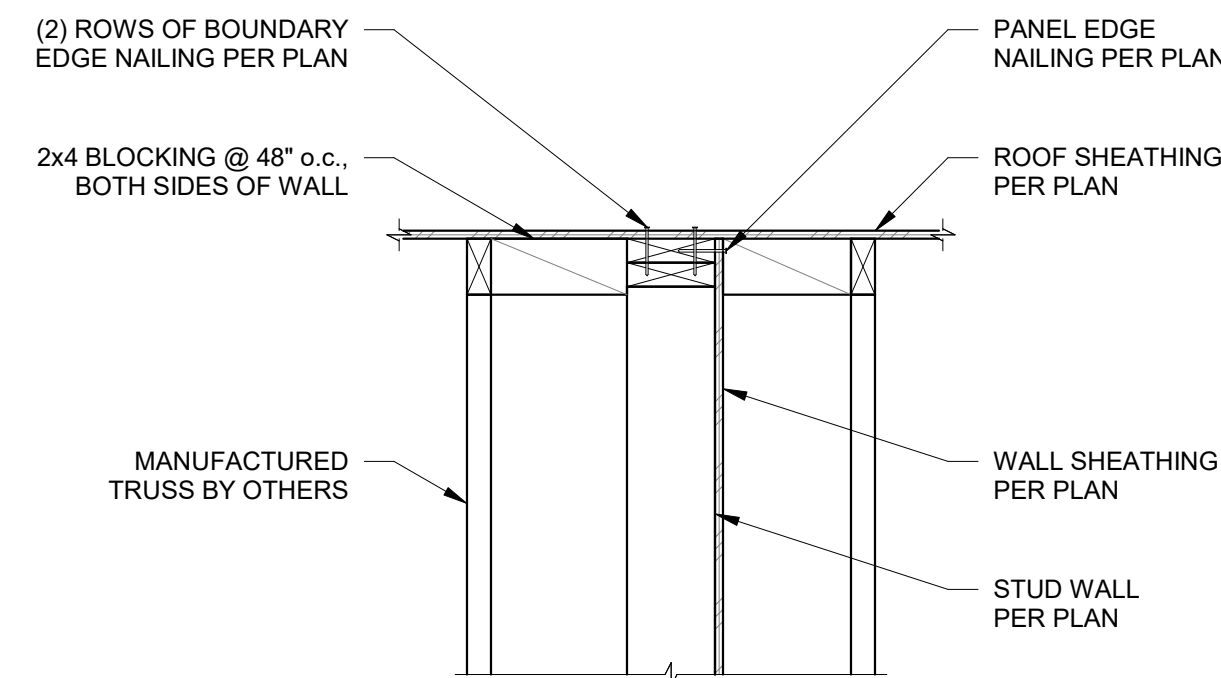
3 STUD CORNER DETAIL



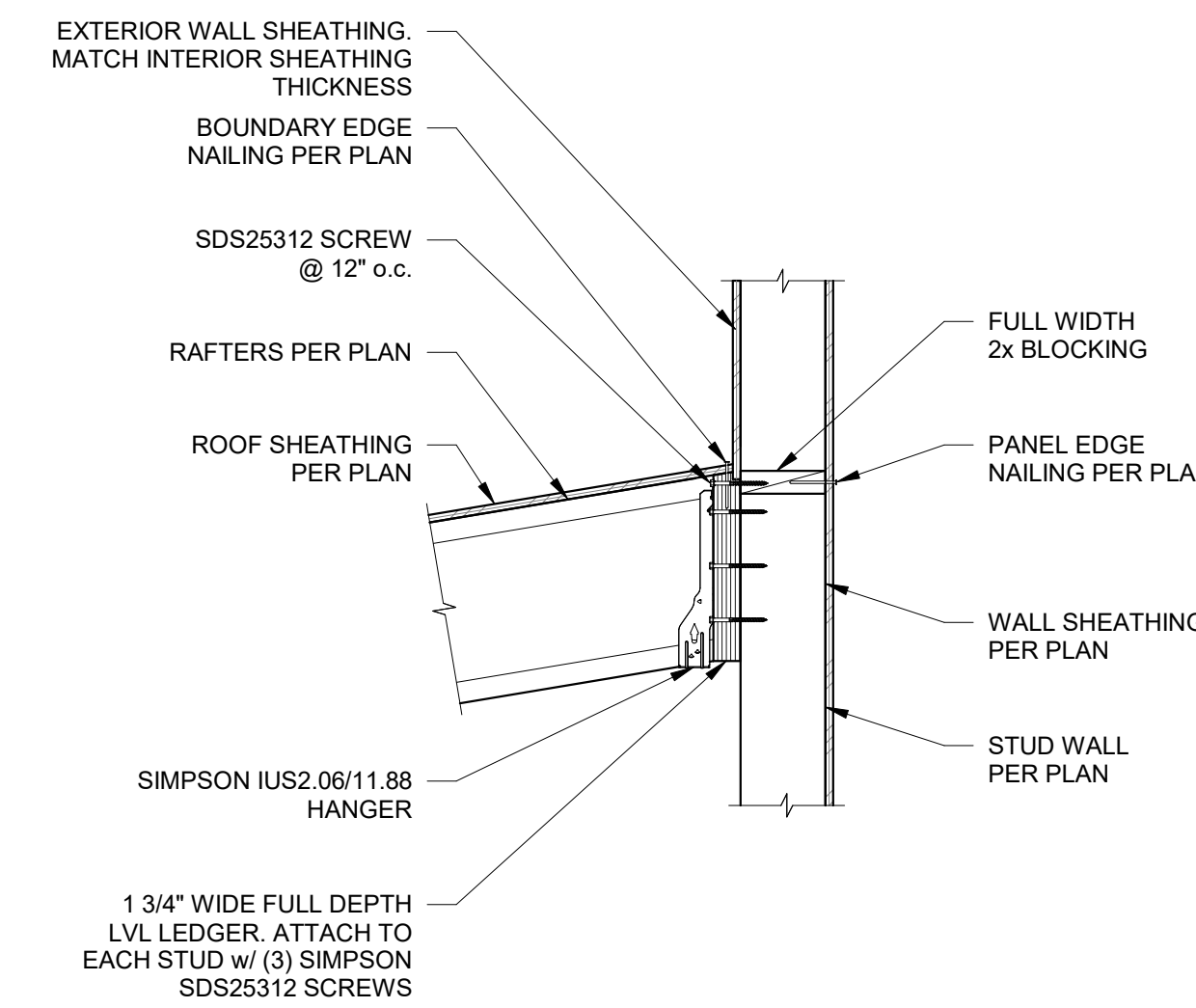
4 STUD U-CORNER DETAIL



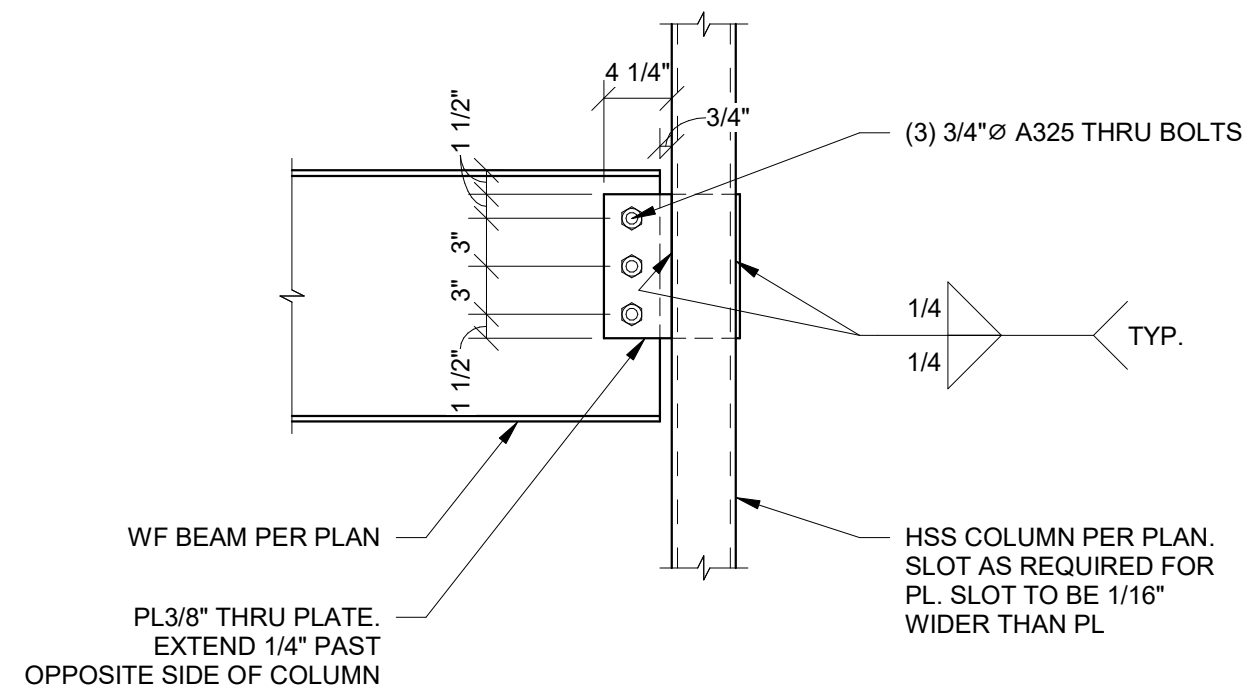
TYP. CHANNEL



6 RAKE SHEAR WALL TO ROOF SHEATHING  
S502 1" = 1'-0"

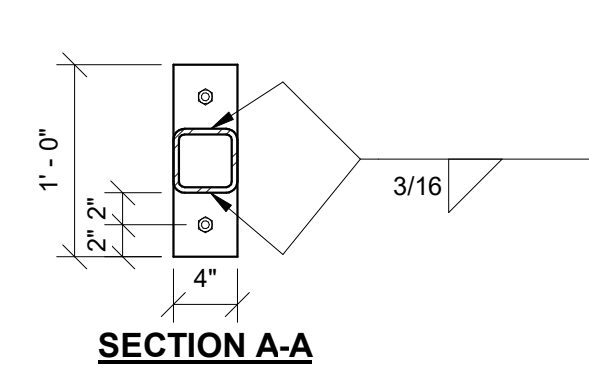


7 RAFTER TO STUD WALL  
S502 1" = 1'-0"

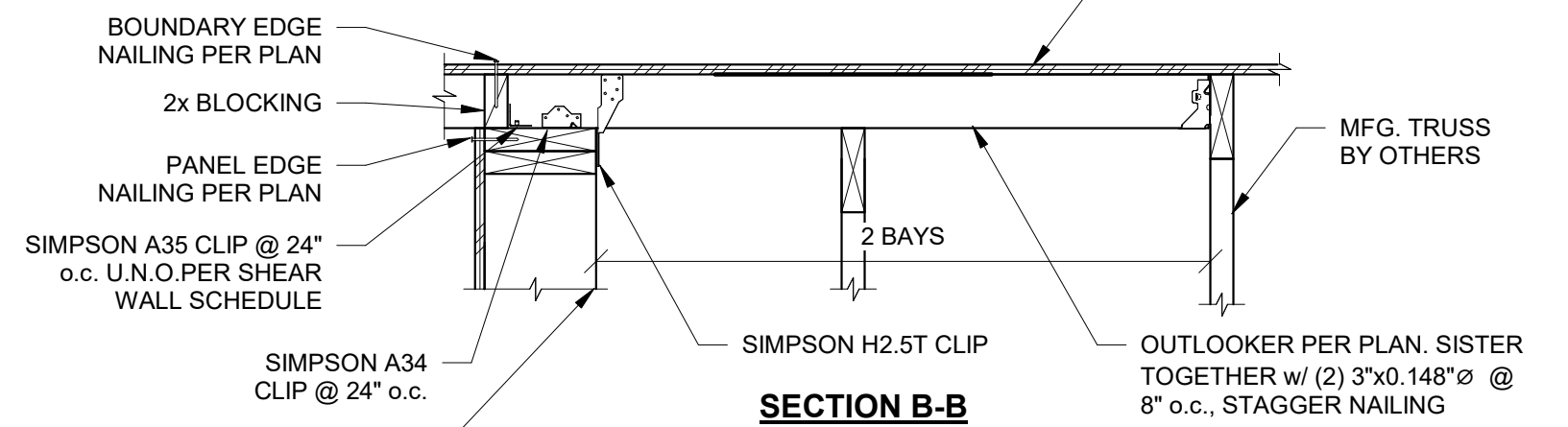


8 WF BEAM TO HSS COLUMN  
S502 1" = 1'-0"

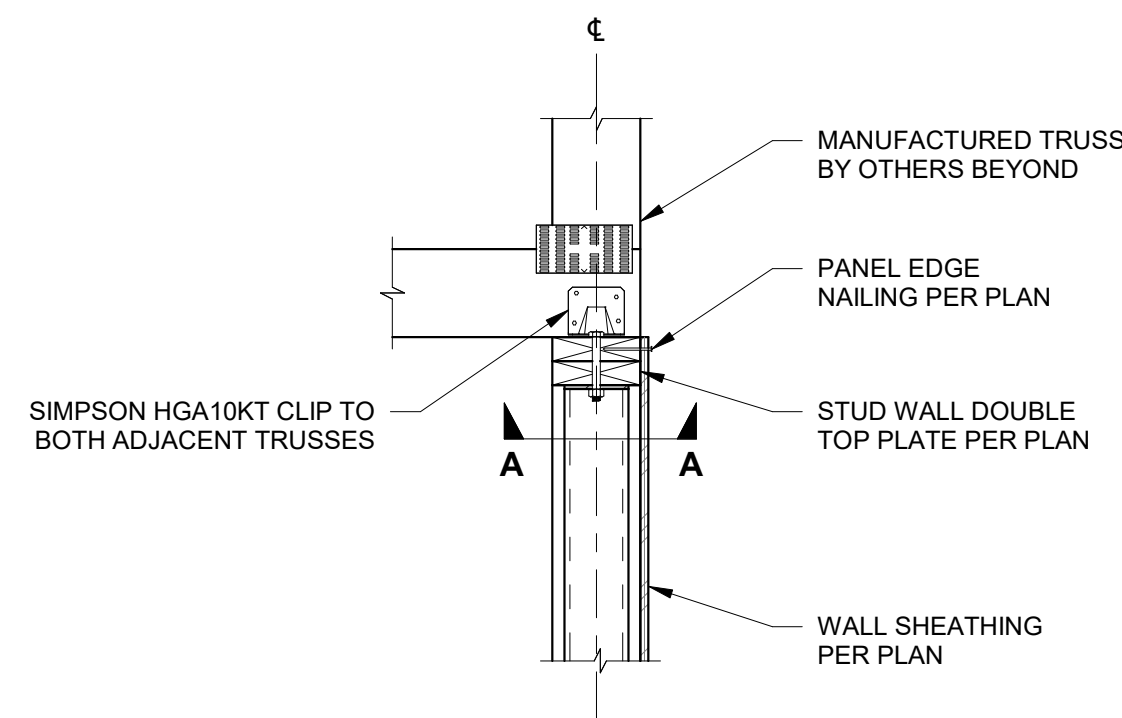
5 TYP. WALL CORNERS  
S502 1" = 1'-0"



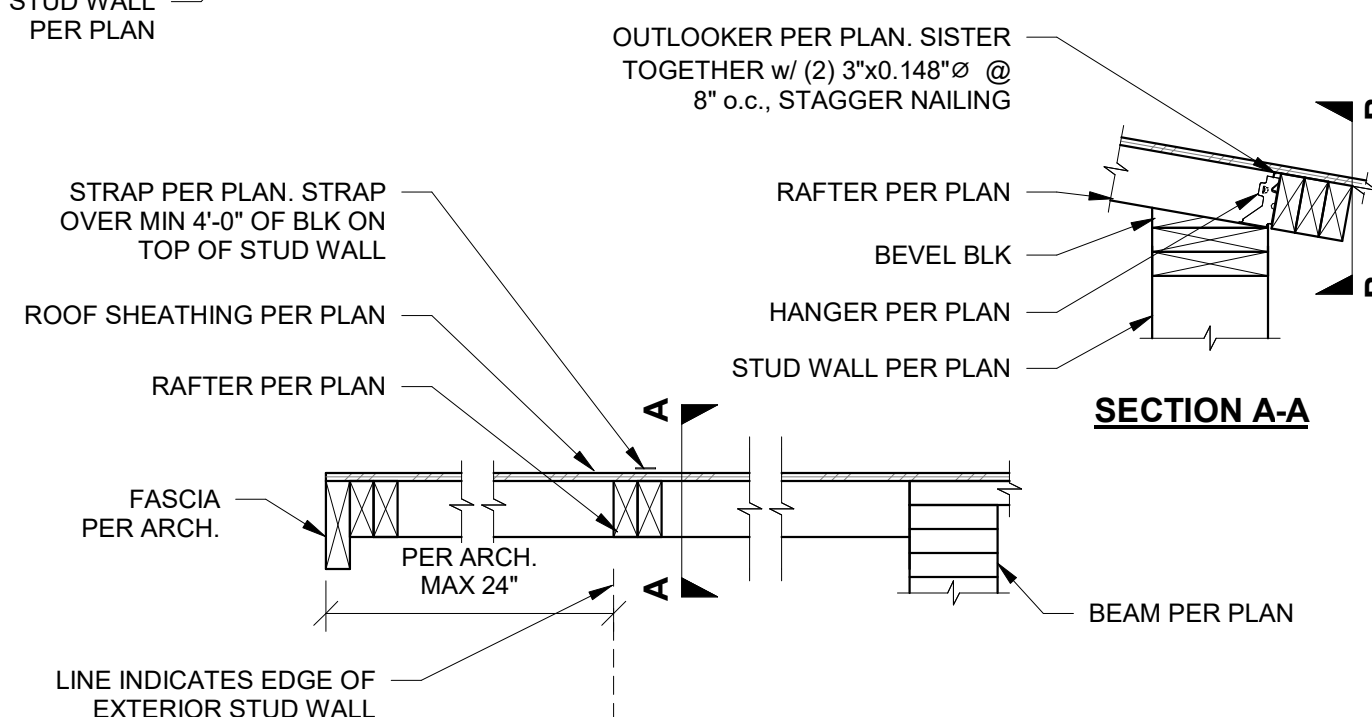
SECTION A-A



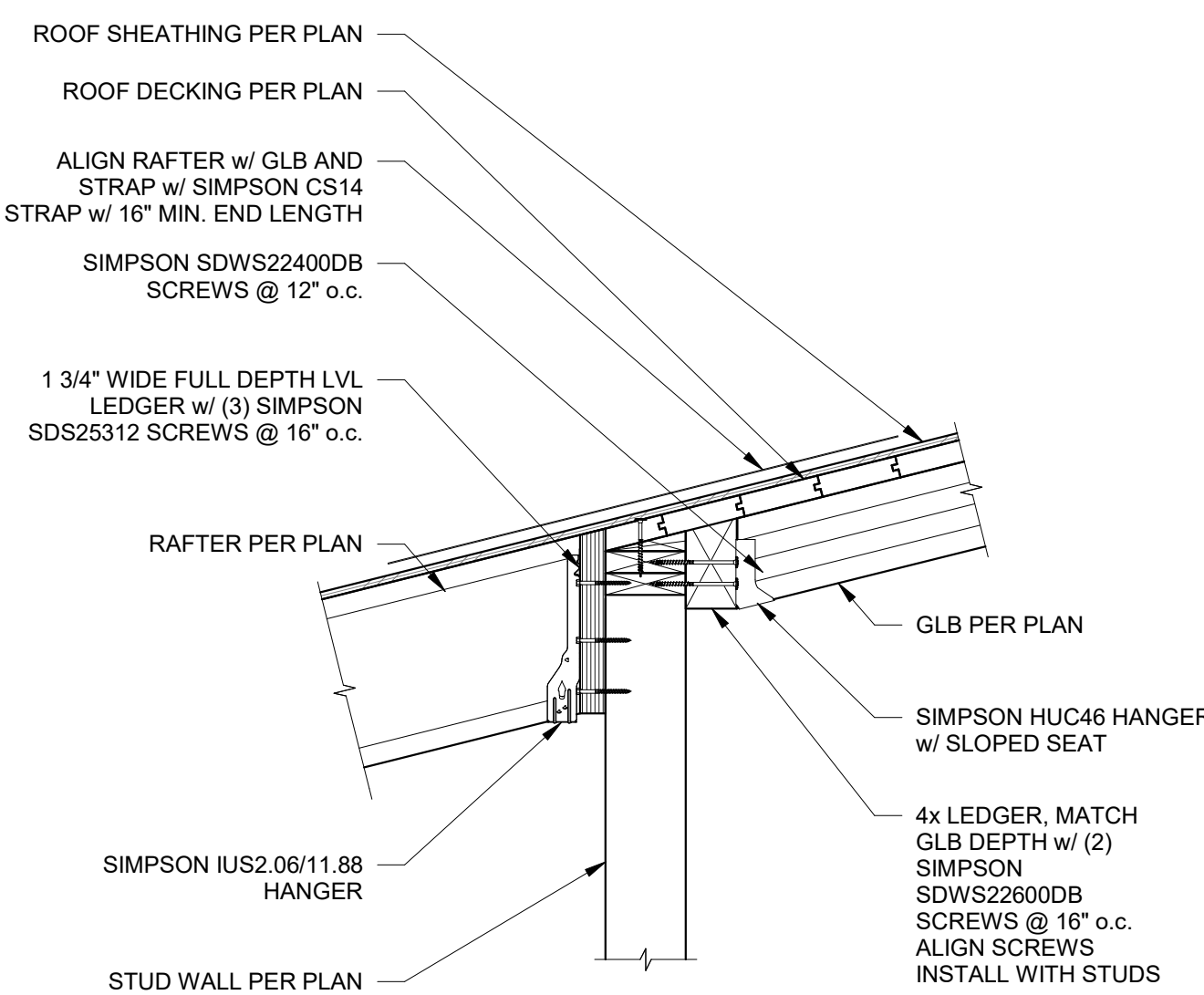
SECTION B-B



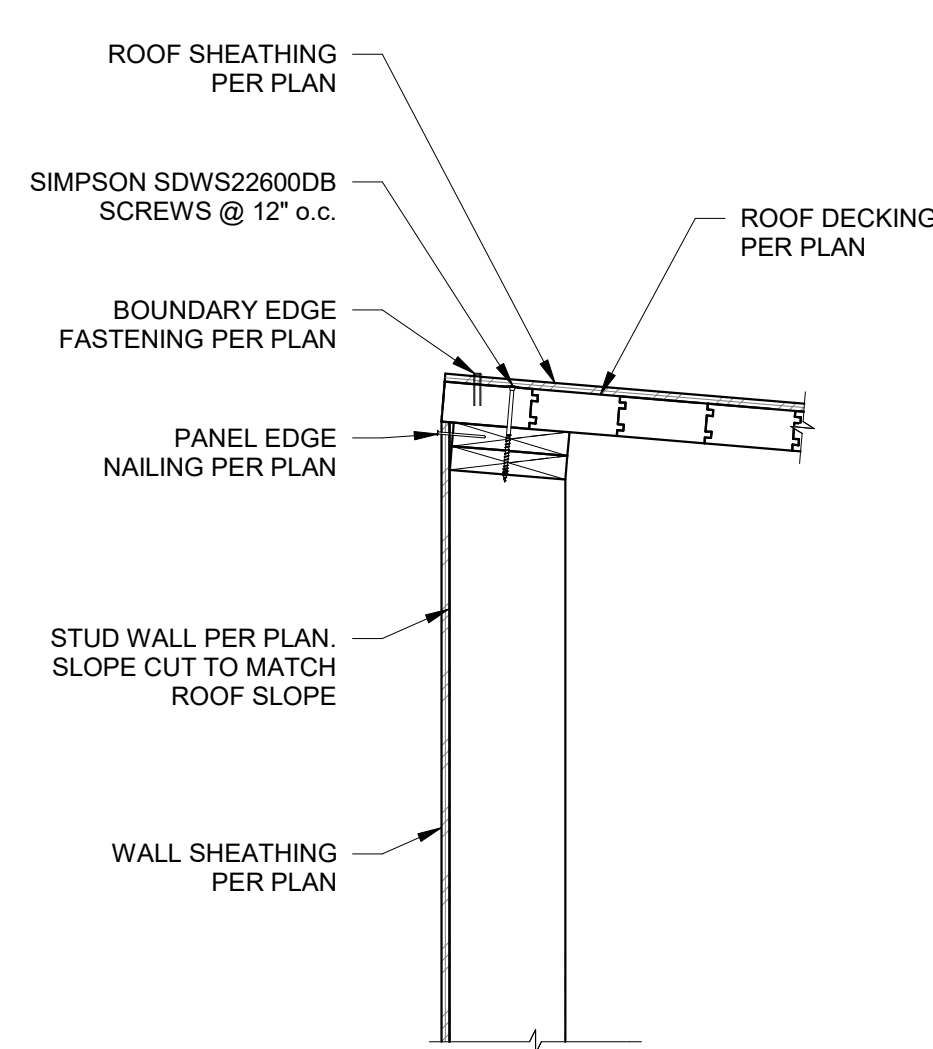
9 PERIMETER HSS TO ROOF FRAMING  
S502 1" = 1'-0"



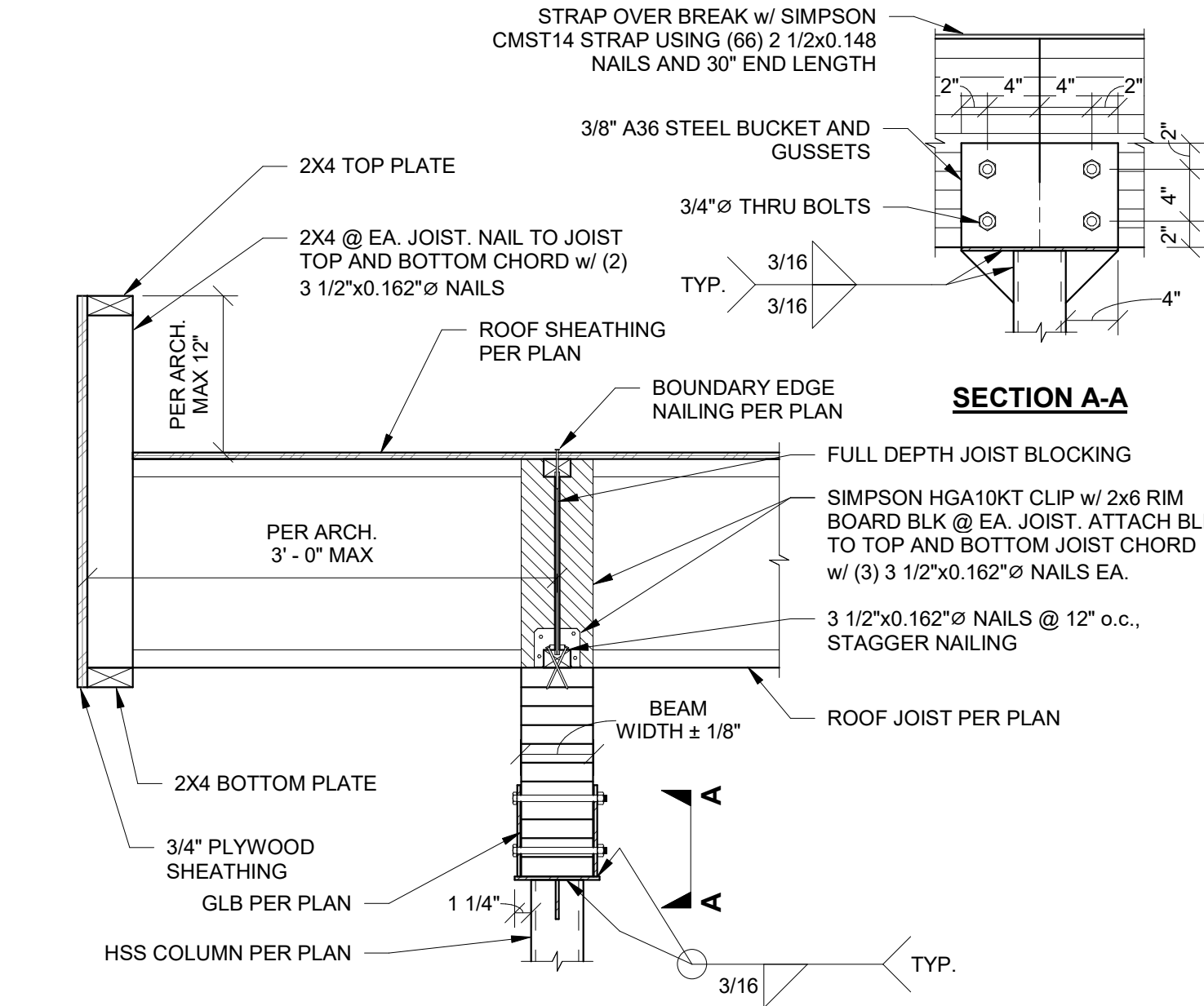
10 HALLWAY OVERHANG  
S502 1" = 1'-0"



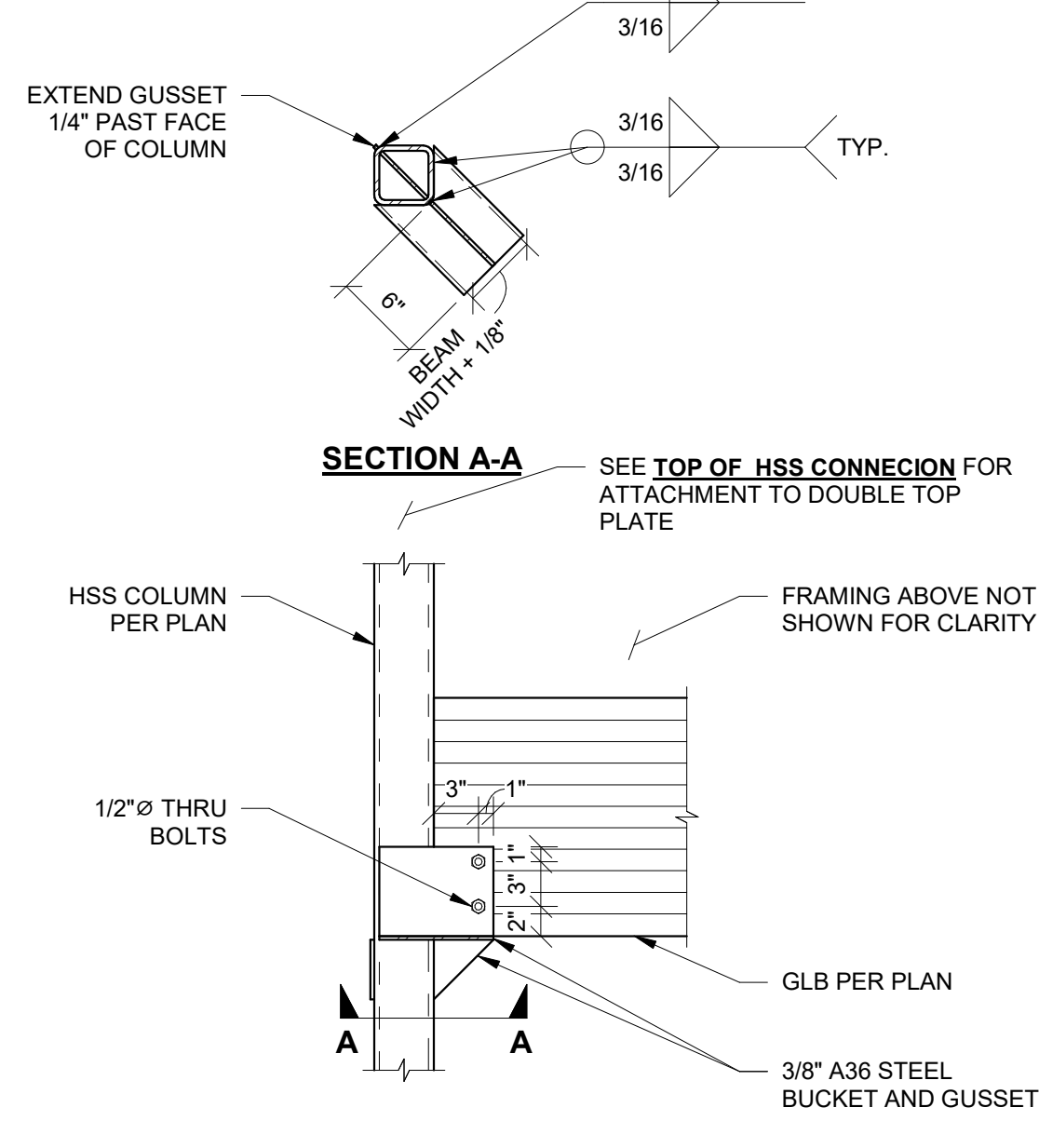
11 HALLWAY AND RAFTER TO STUD WALL  
S502 1" = 1'-0"



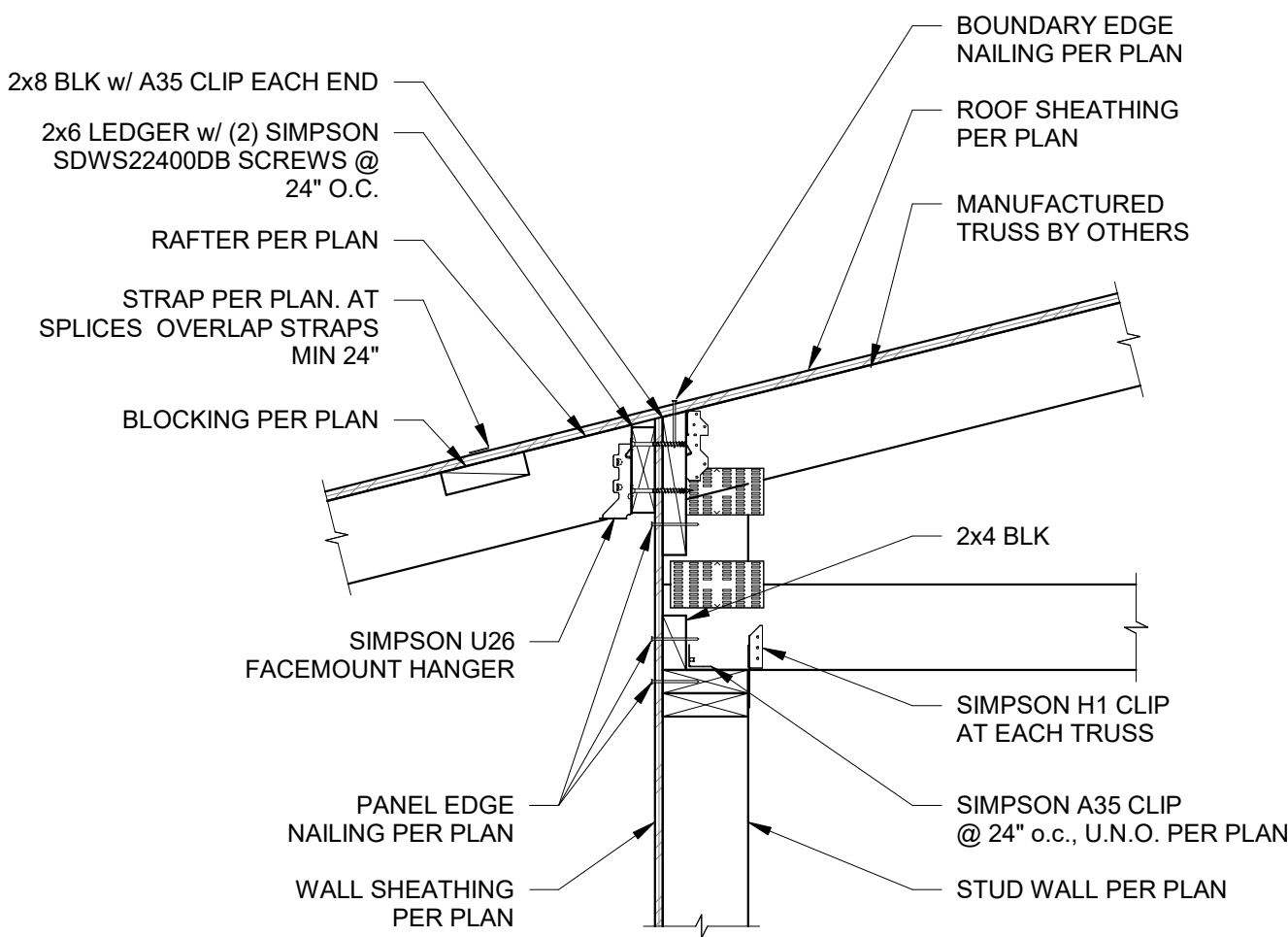
12 TOP OF WALL CONN. TO DECKING  
S502 1" = 1'-0"



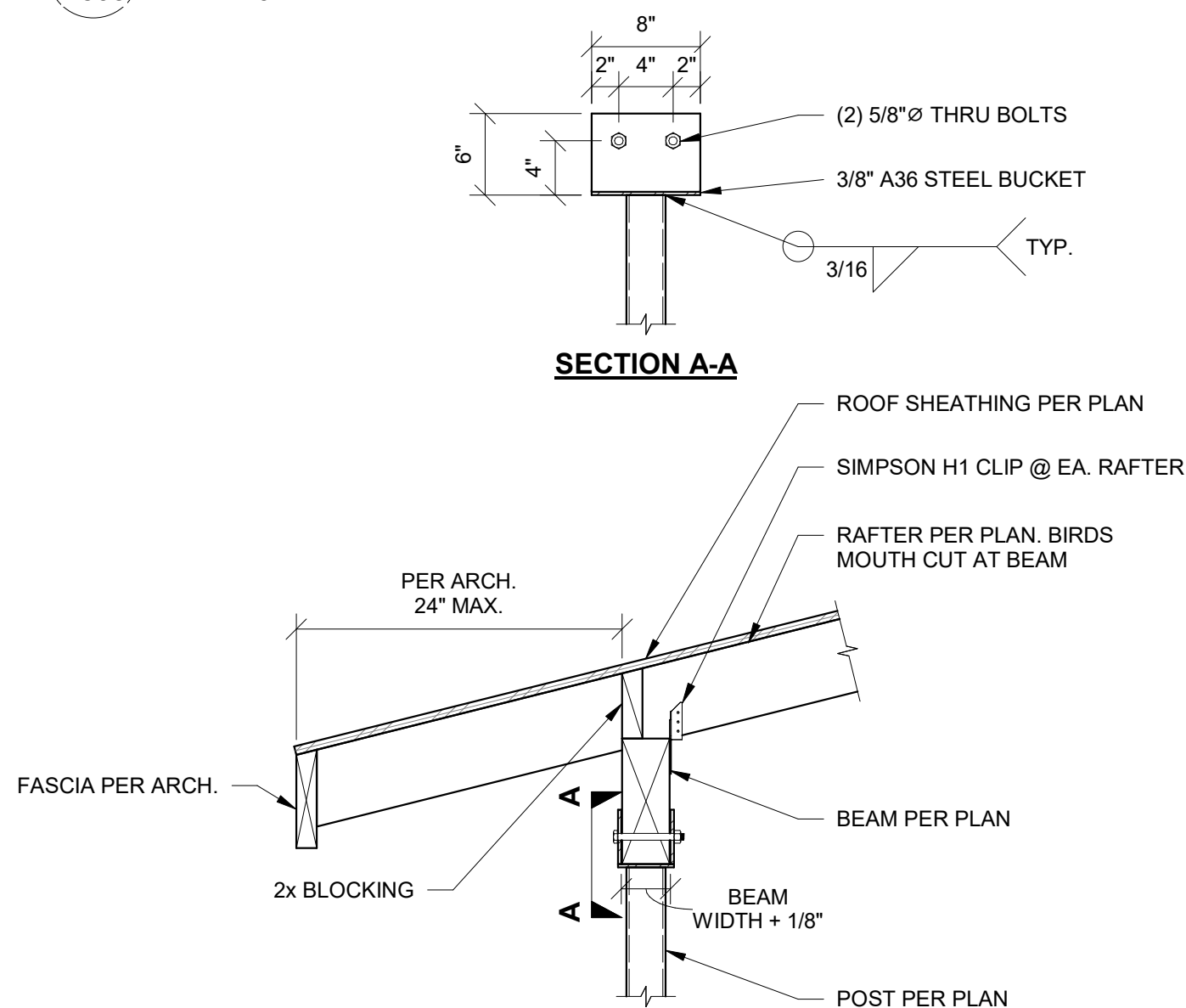
1  
S503 CANOPY EAVE CONNECTION  
1" = 1'-0"



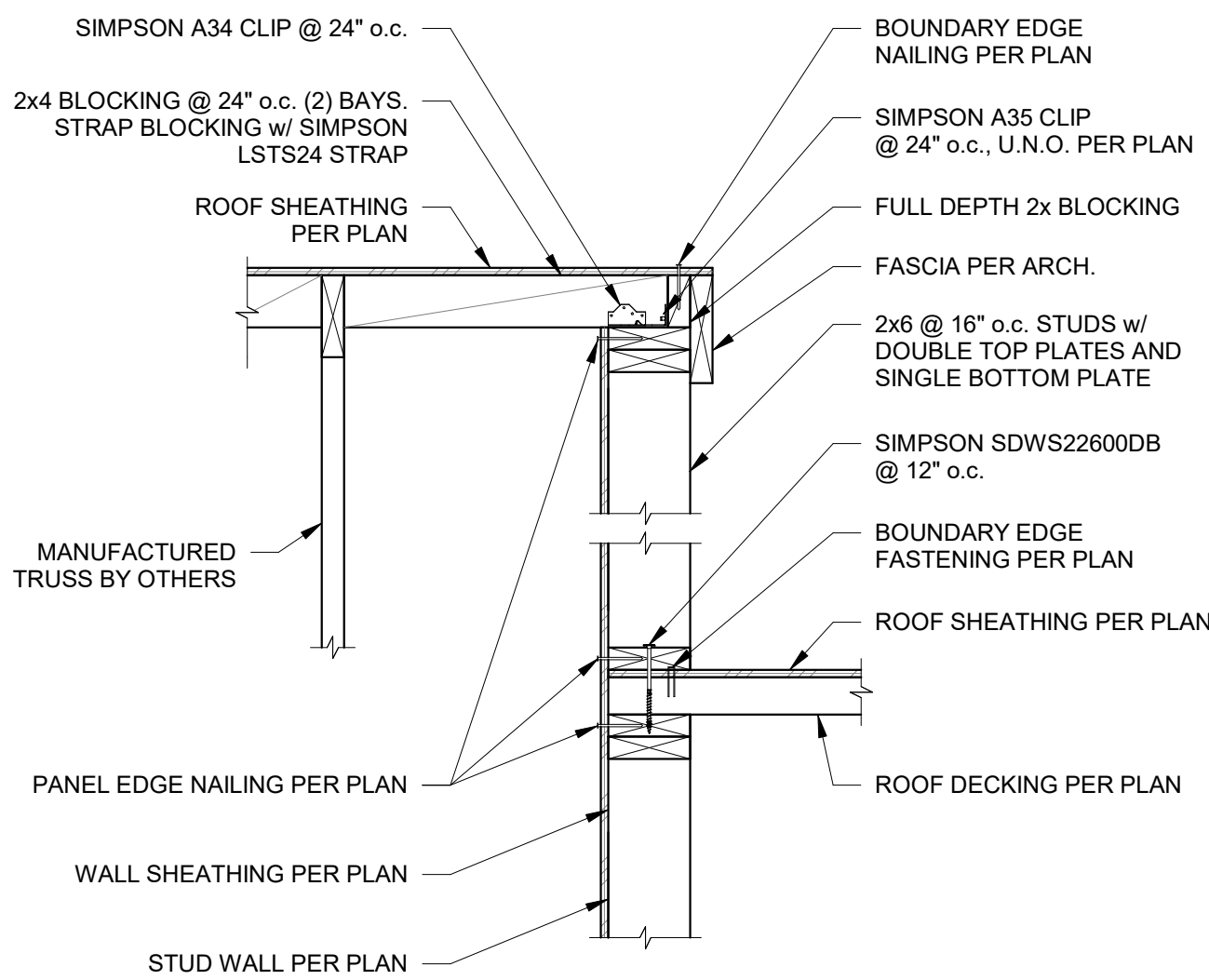
2  
S503 CANOPY BEAM TO HSS CONNECTION  
1" = 1'-0"



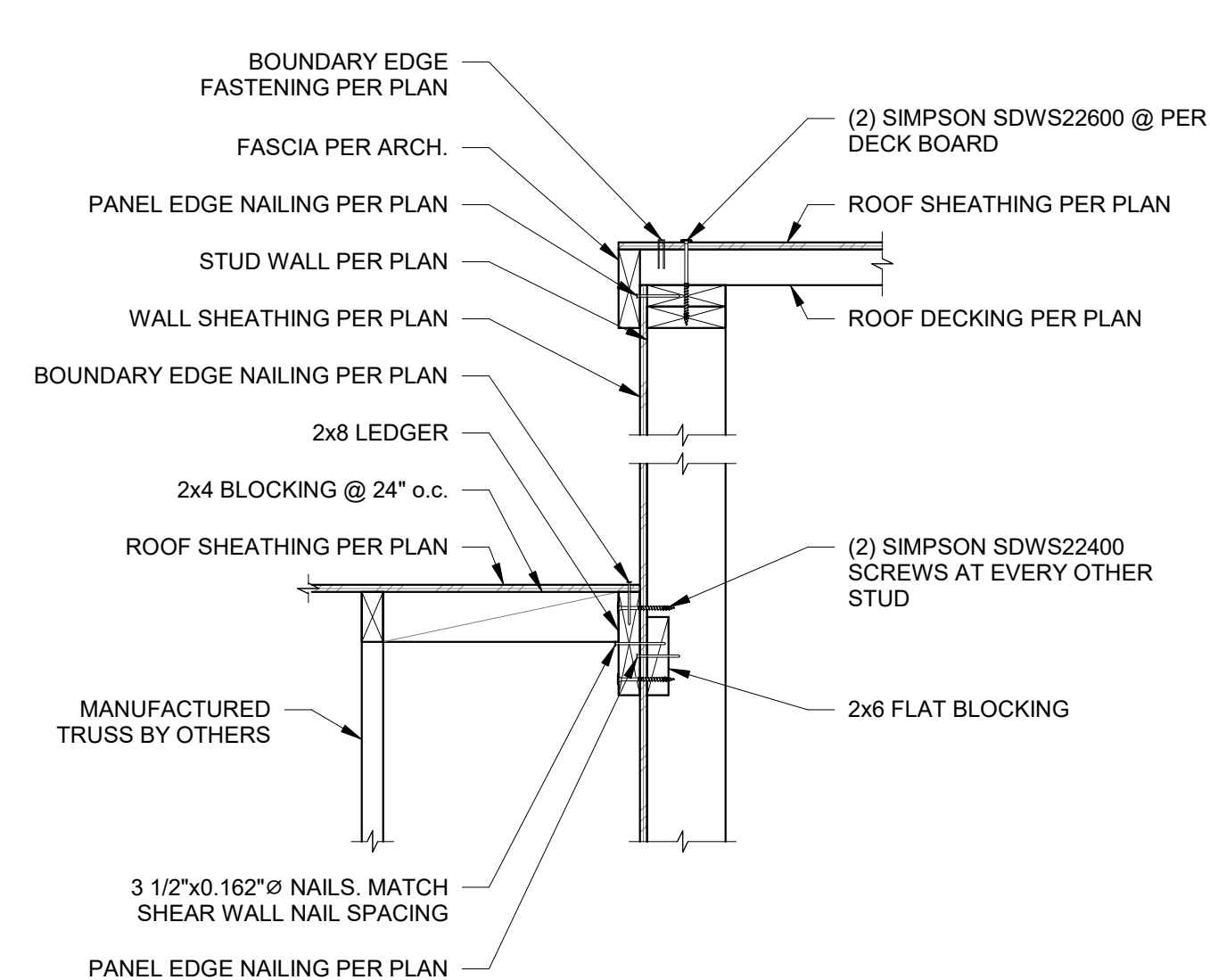
4  
S503 RAFTER AT EXTERIOR WALL  
1" = 1'-0"



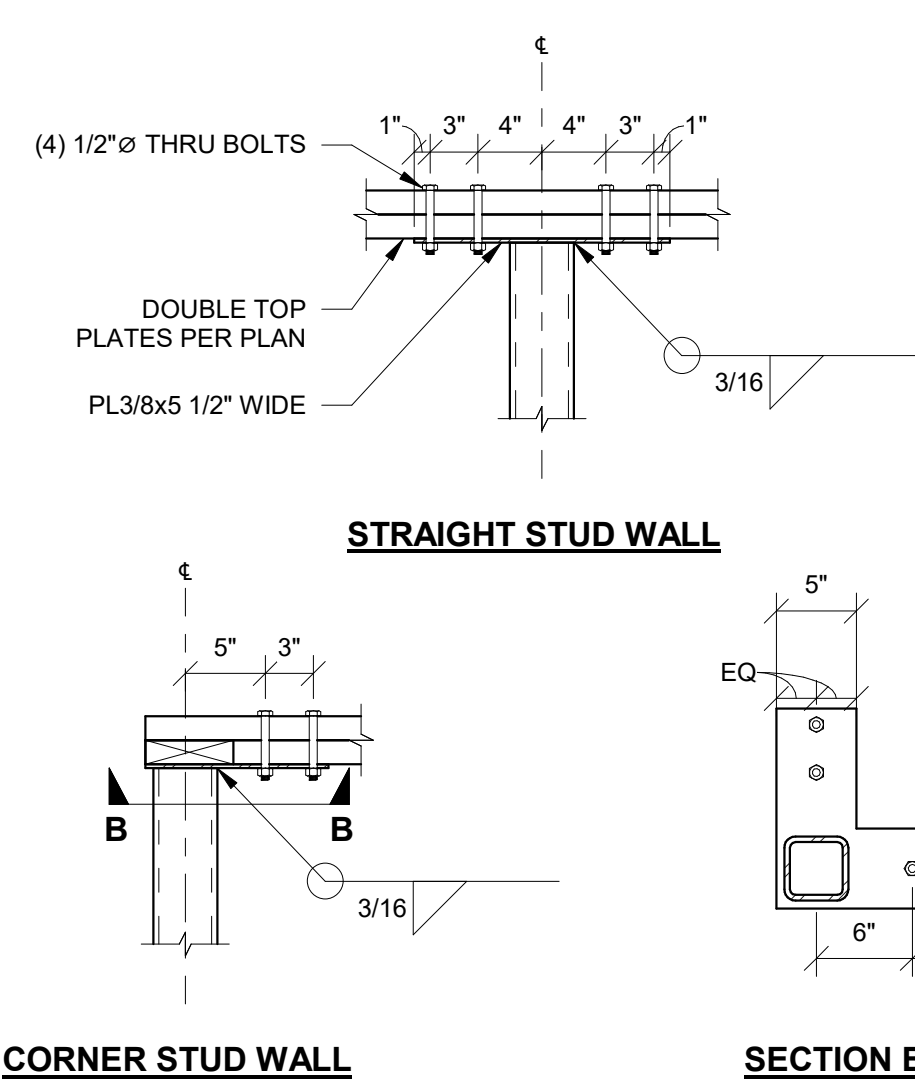
5  
S503 RAFTER TO BEAM  
1" = 1'-0"



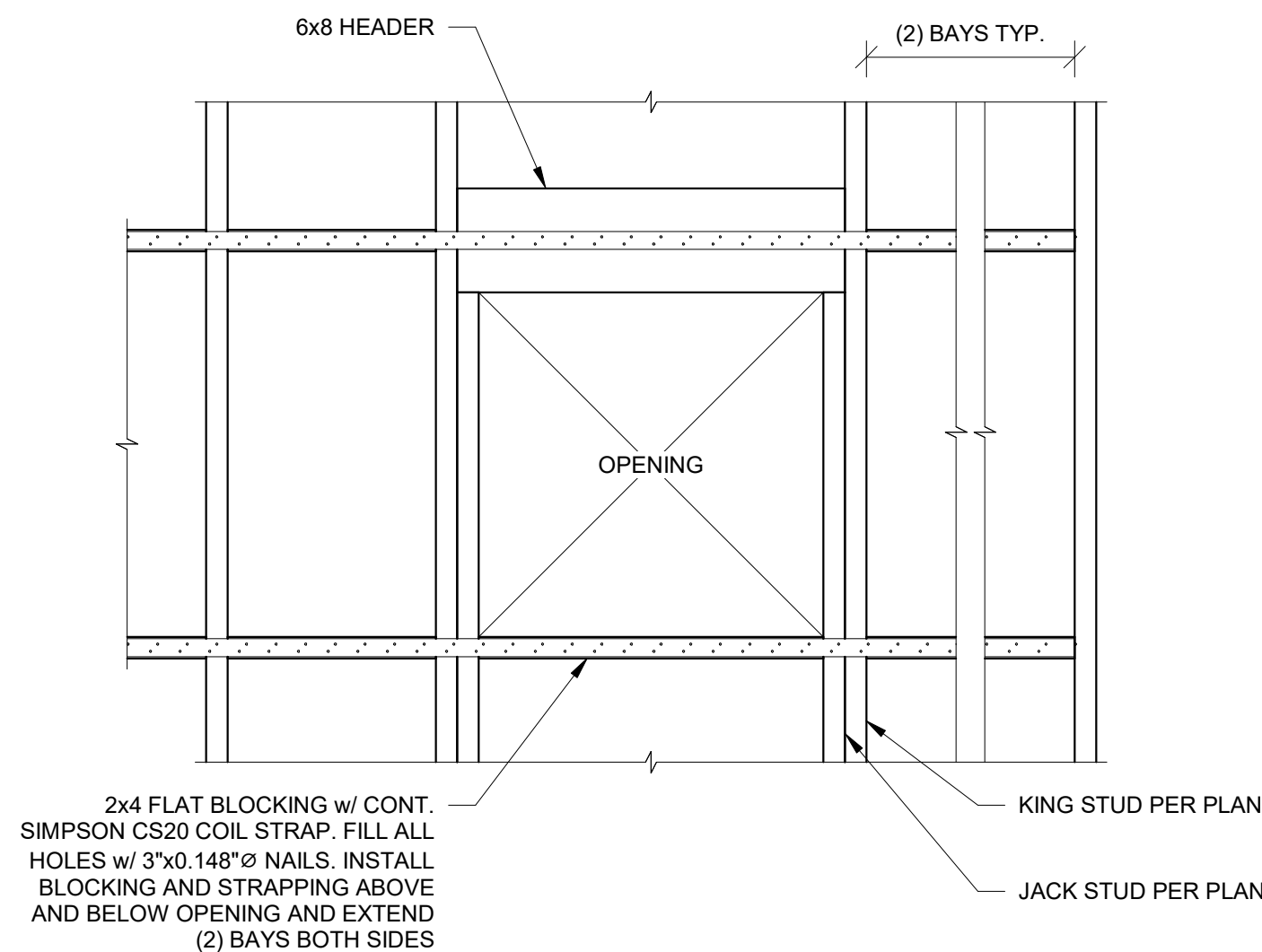
8  
S503 RAKE WALL GABLE END AT DECKING  
1" = 1'-0"



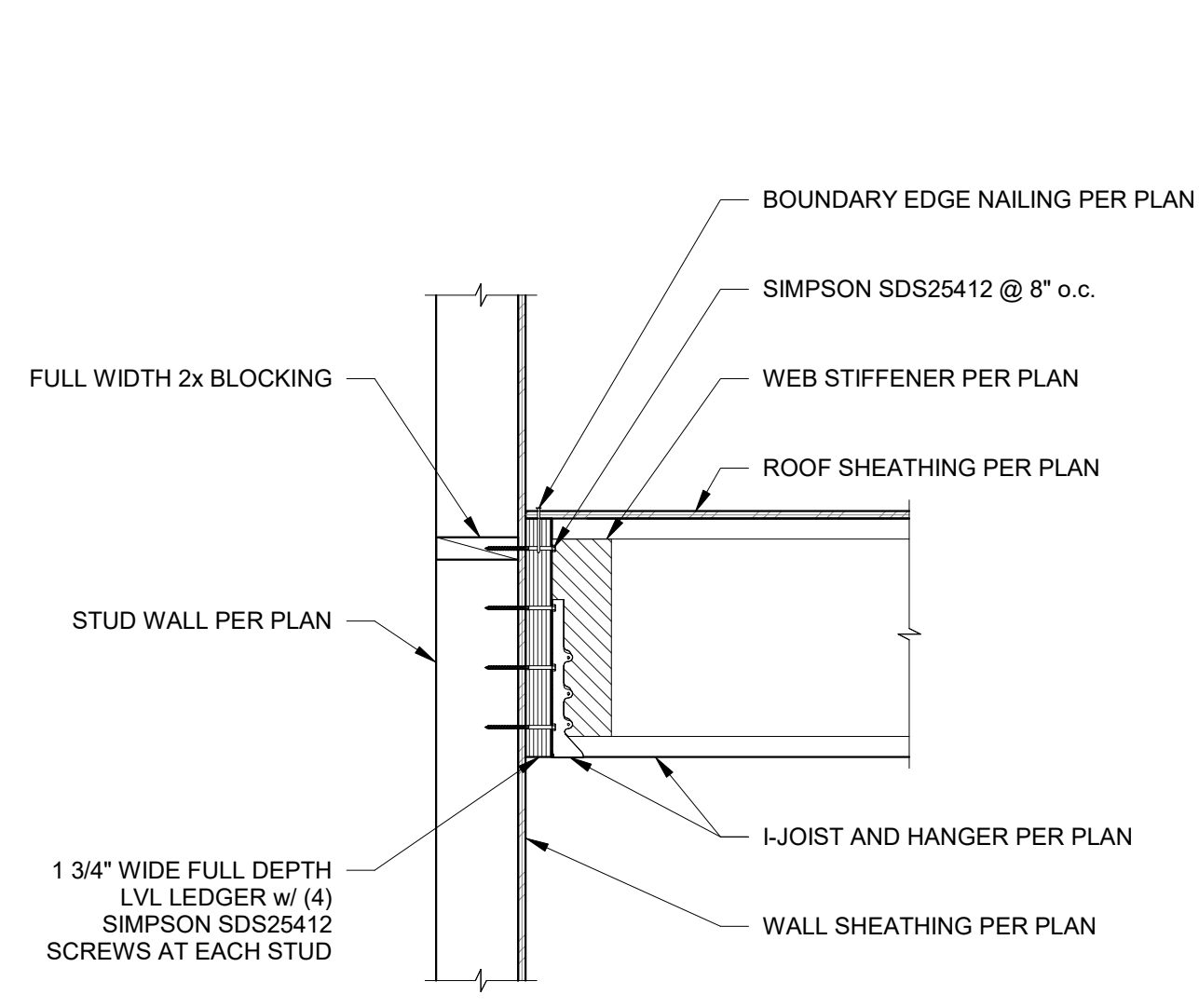
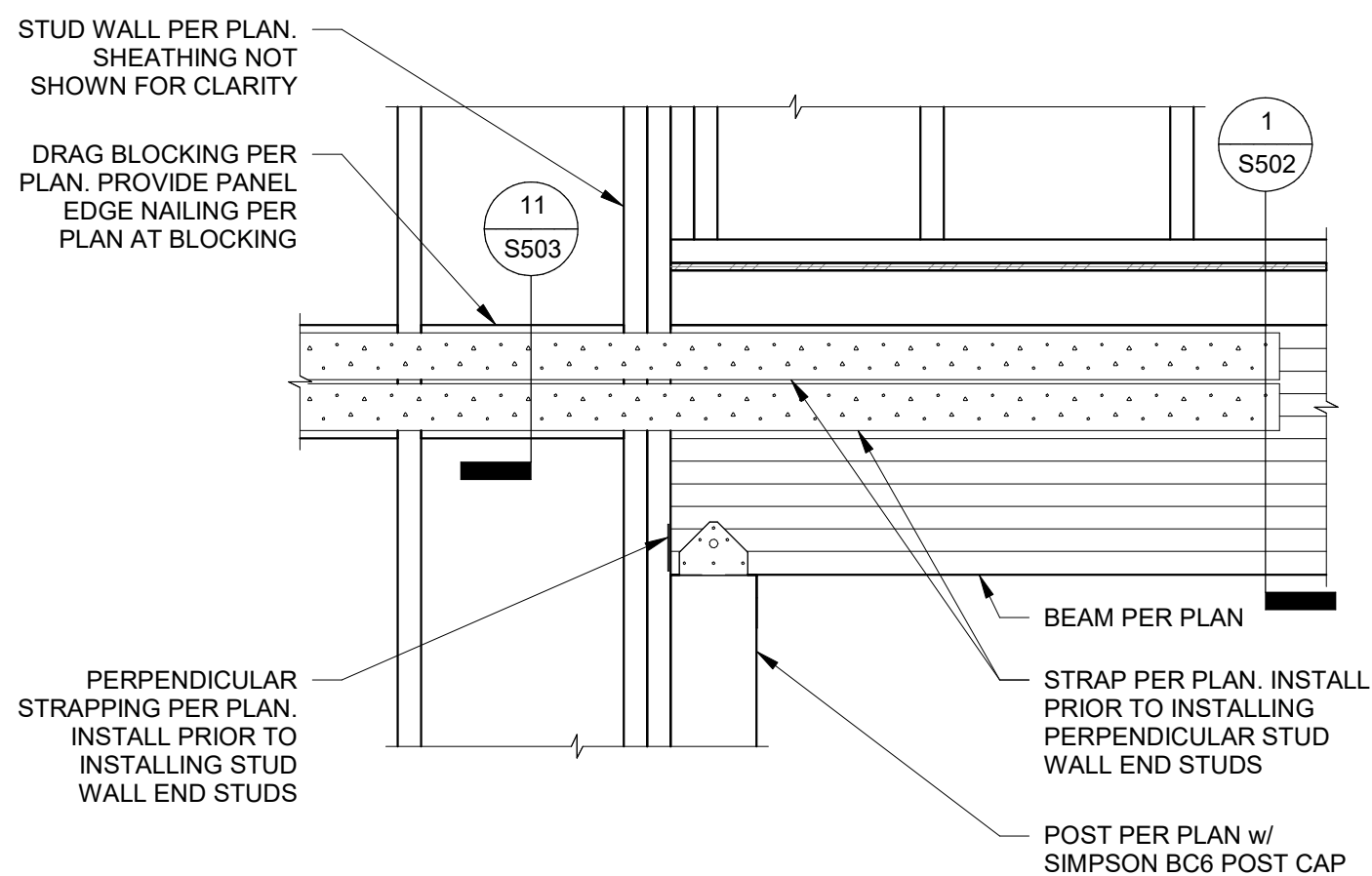
9  
S503 RAKE WALL GABLE END AT DECKING  
1" = 1'-0"



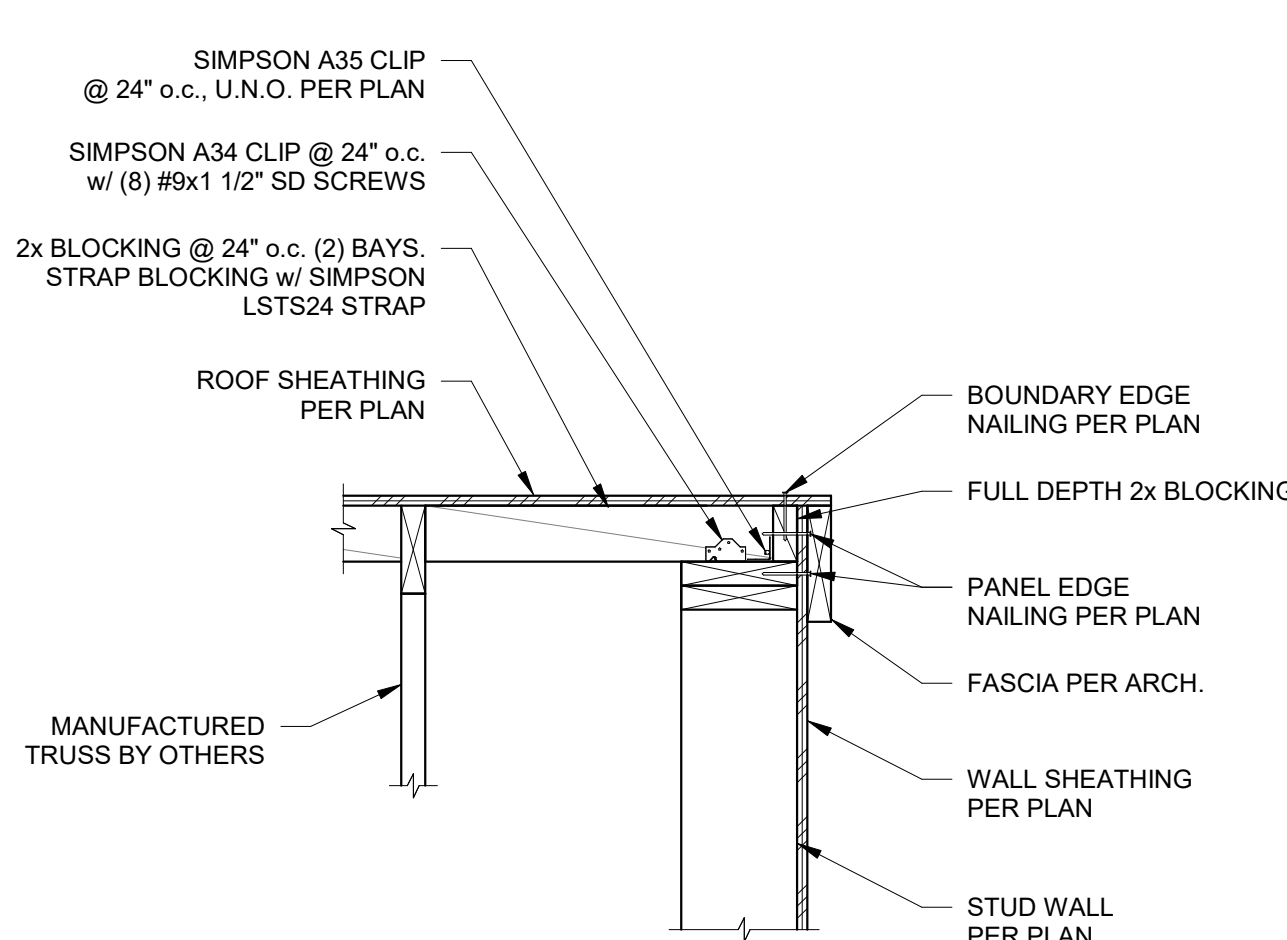
10  
S503 SHEAR WALL DRAG BLOCKING  
1" = 1'-0"



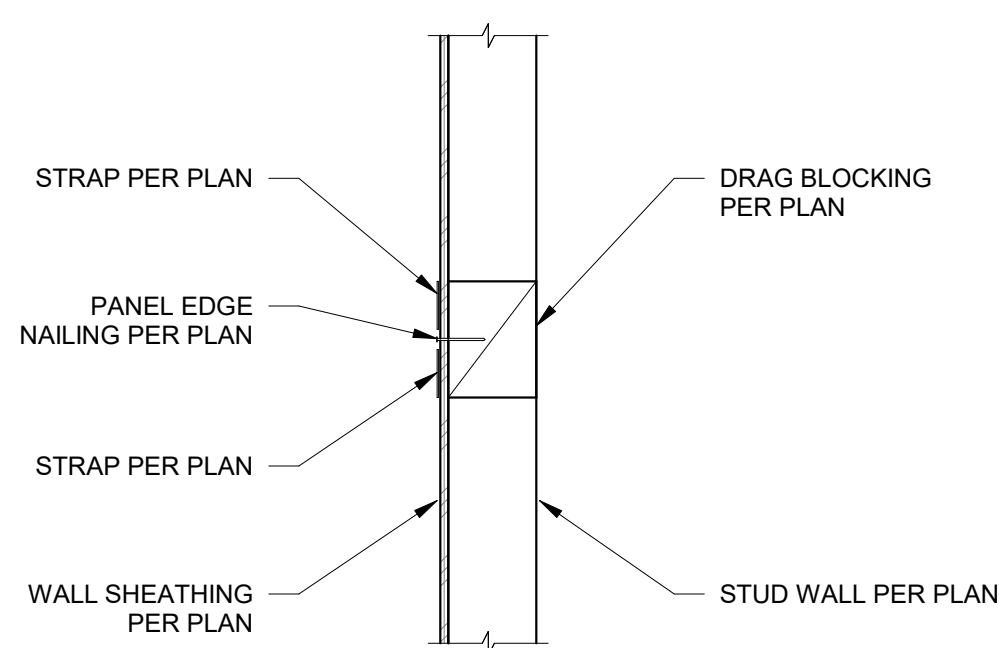
6  
S503 TYP. DUCT PENETRATION THRU RAKE SHEAR WALL  
1" = 1'-0"



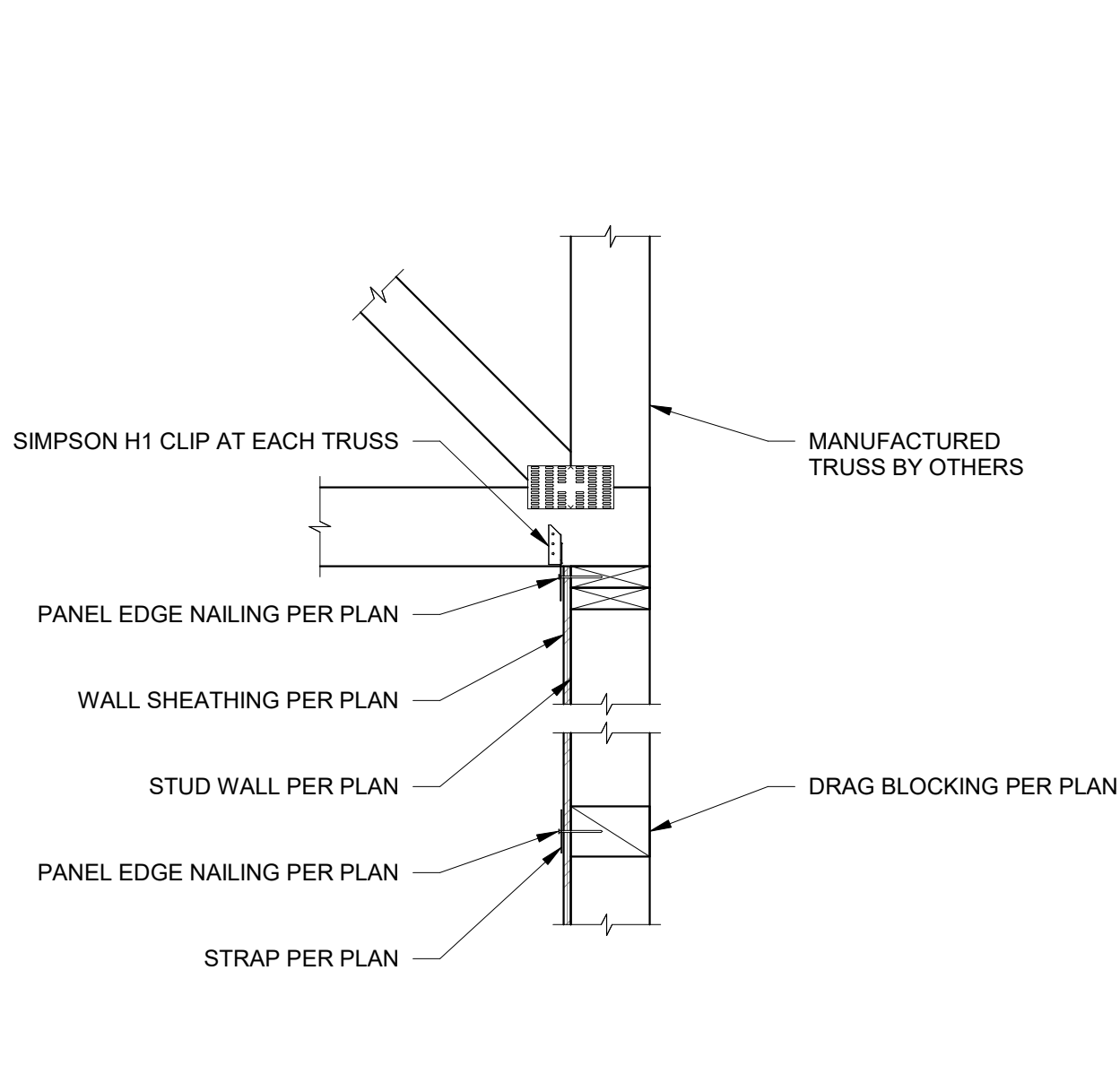
3  
S503 EXTERIOR WALL AT LOW ROOF  
1" = 1'-0"



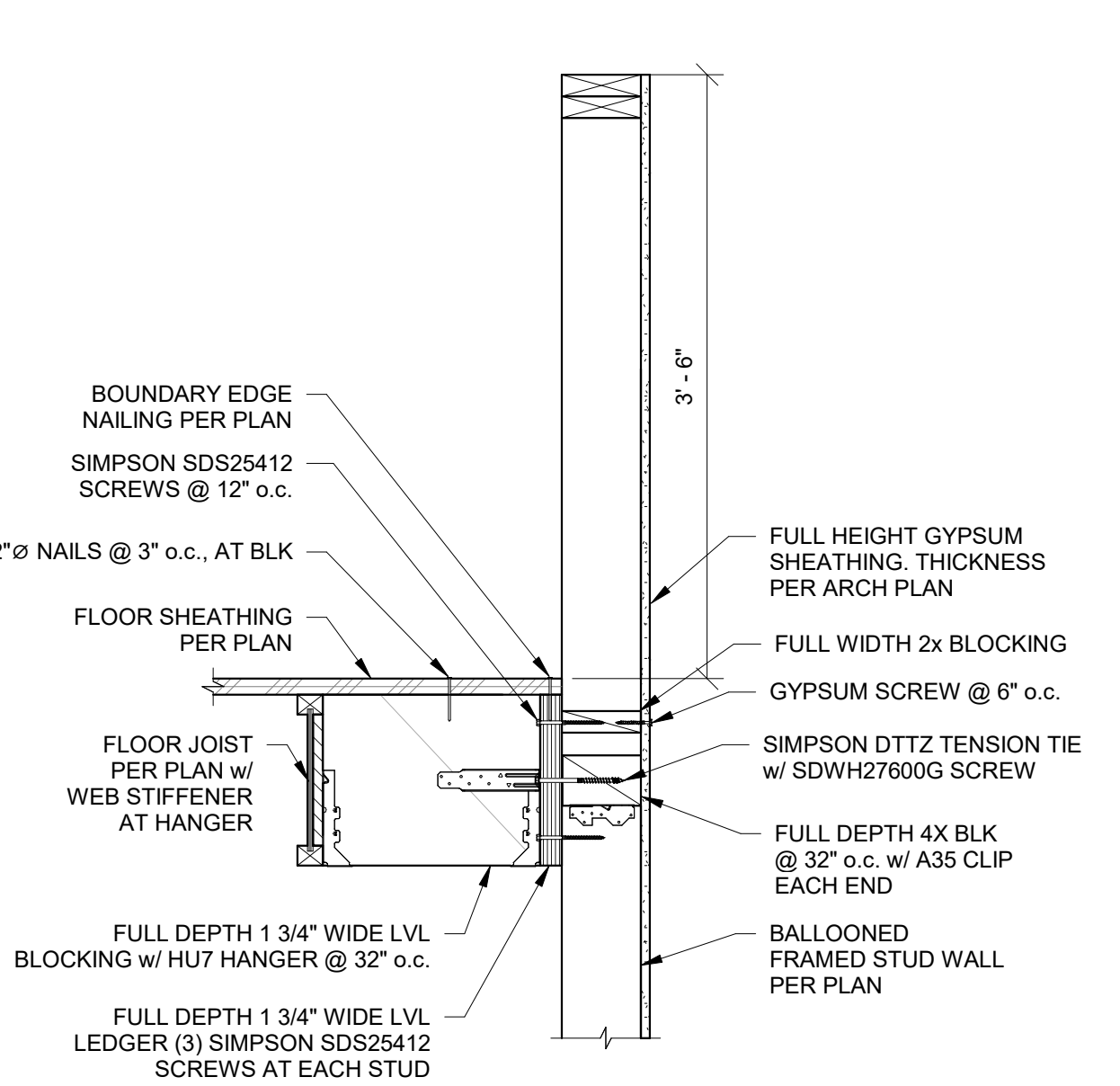
7  
S503 GABLE END WALL w/ NO OUTLOOKERS  
1" = 1'-0"



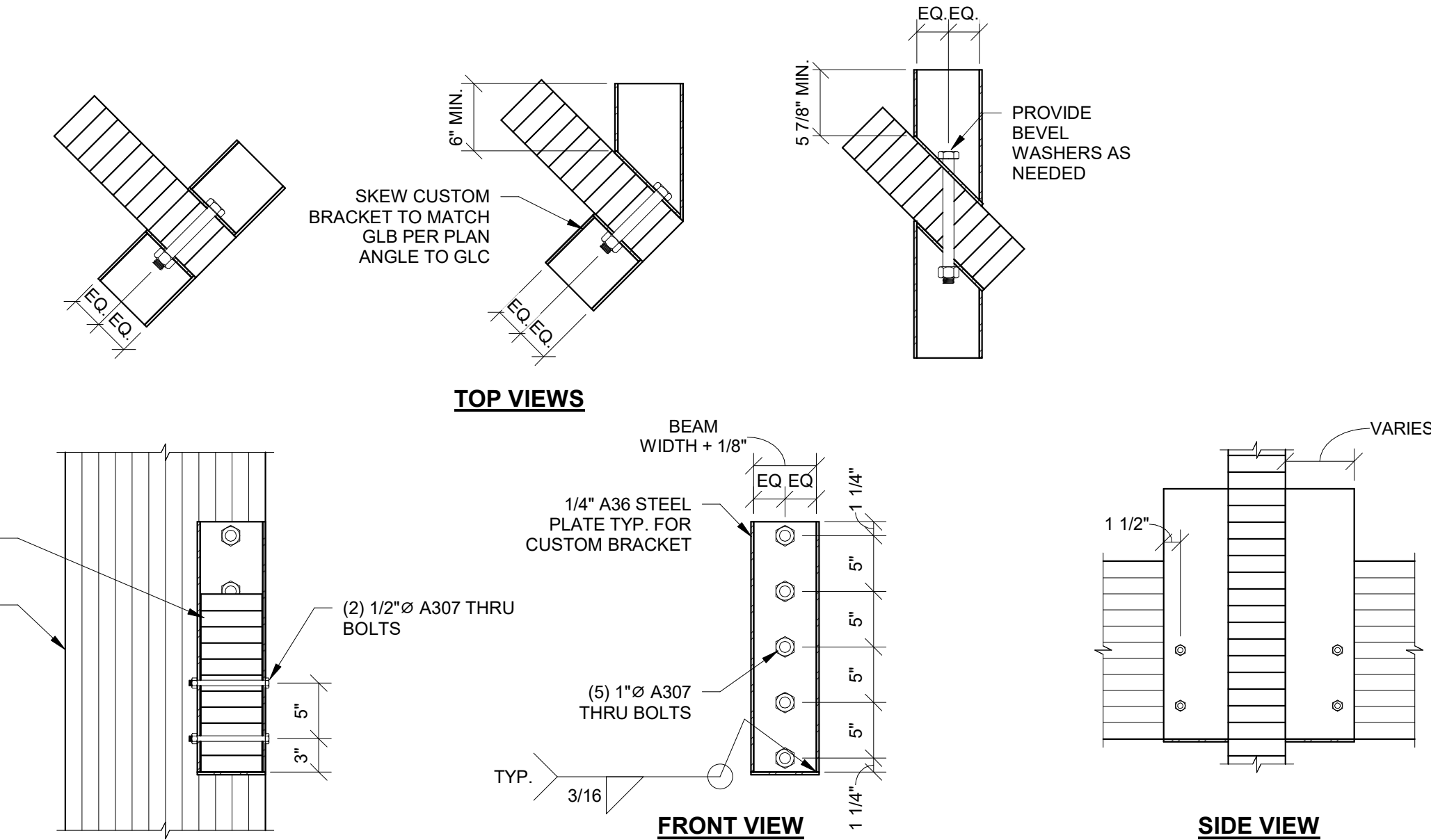
11  
S503 SHEAR WALL DRAG BLOCKING  
1" = 1'-0"



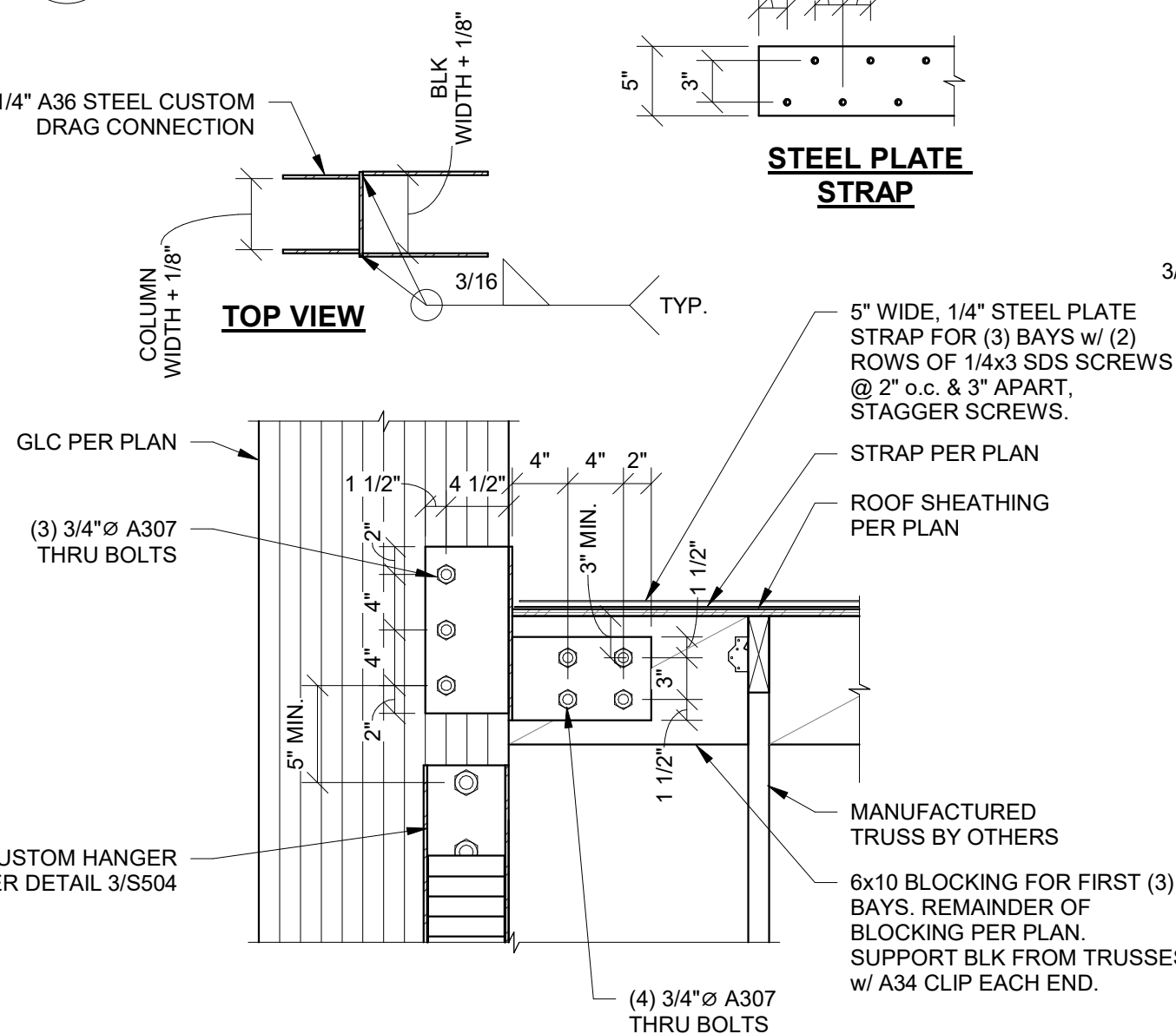
1  
S504  
1" = 1'-0"



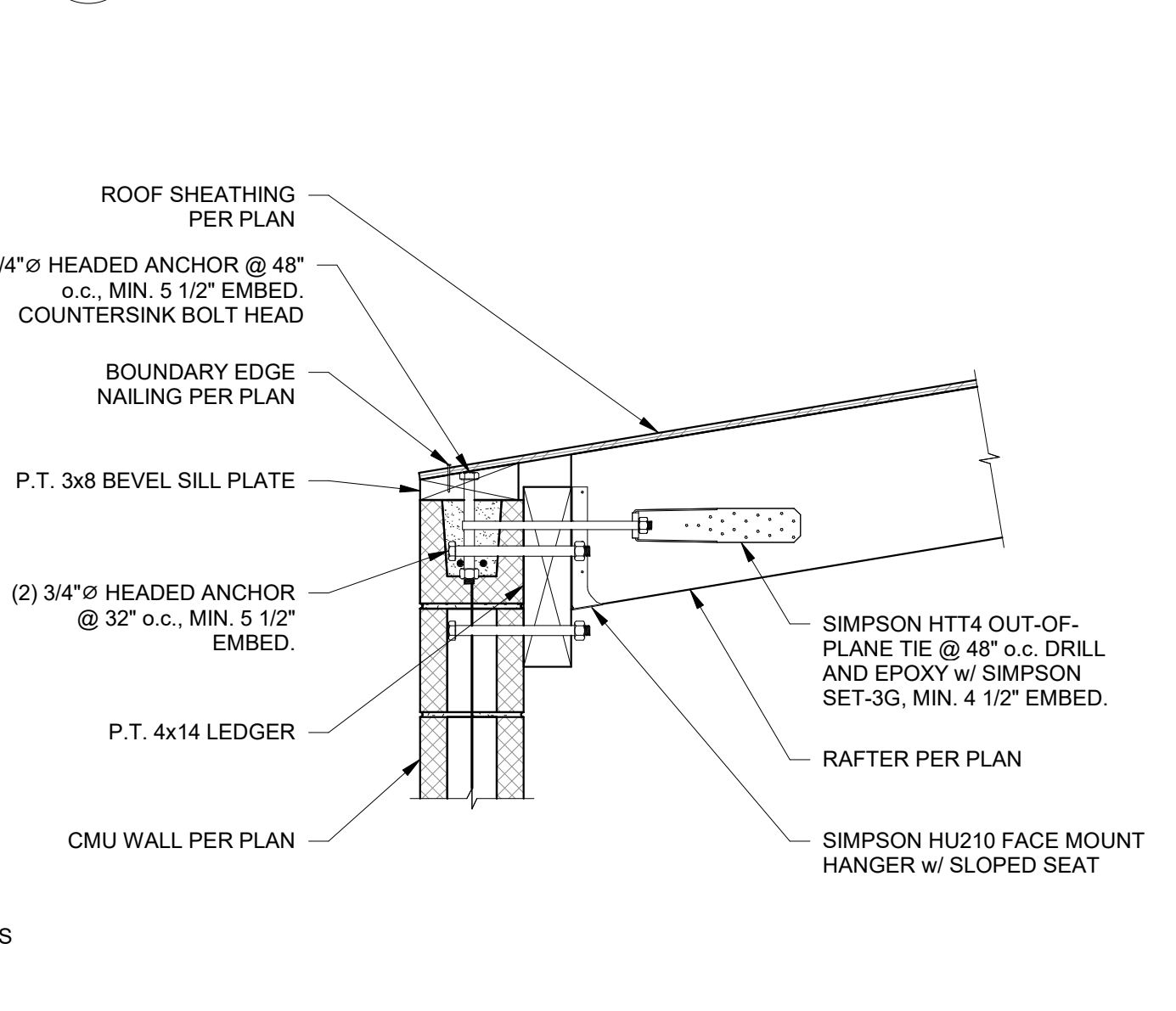
2  
S504  
1" = 1'-0"



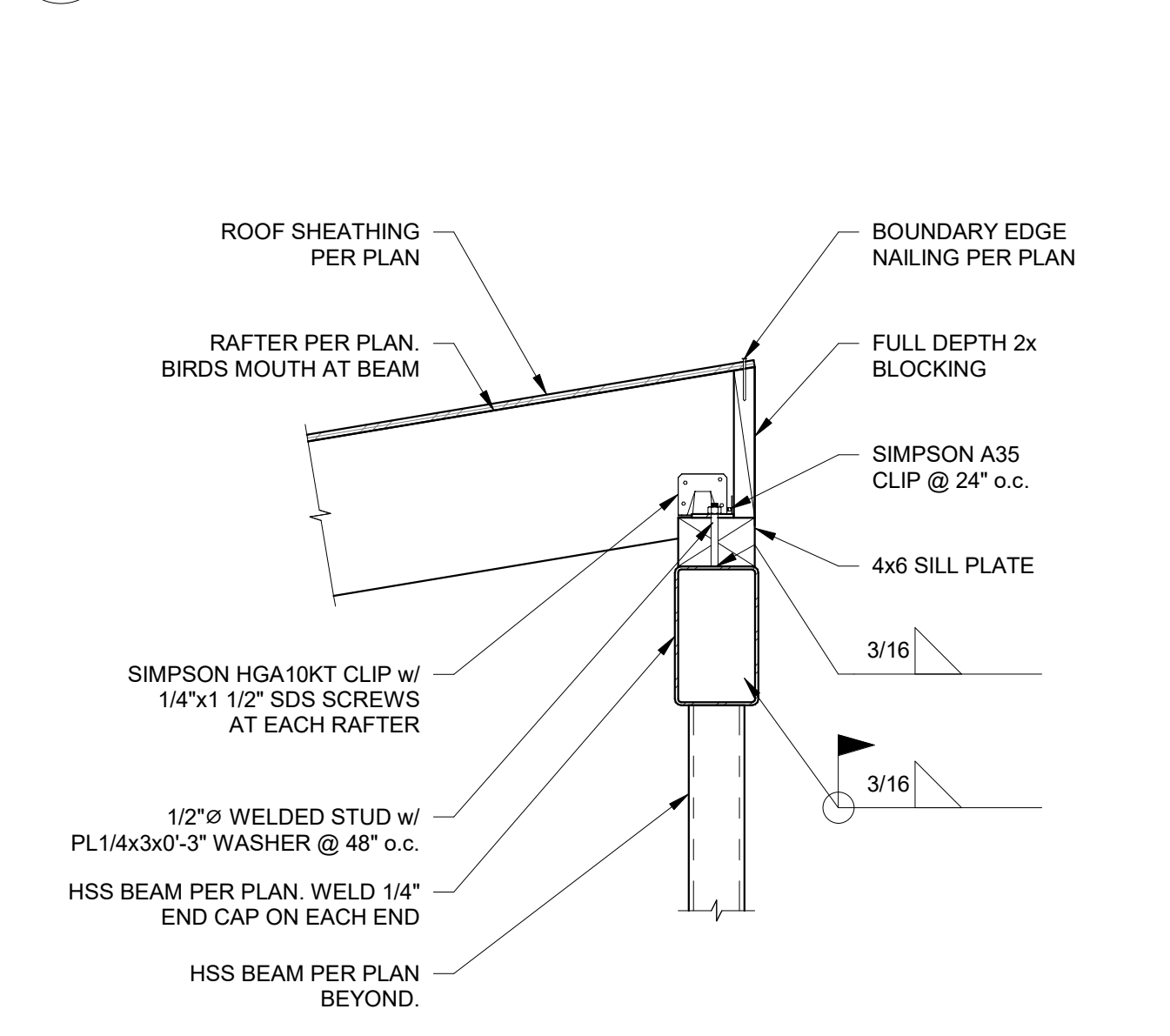
3  
S504  
1" = 1'-0"



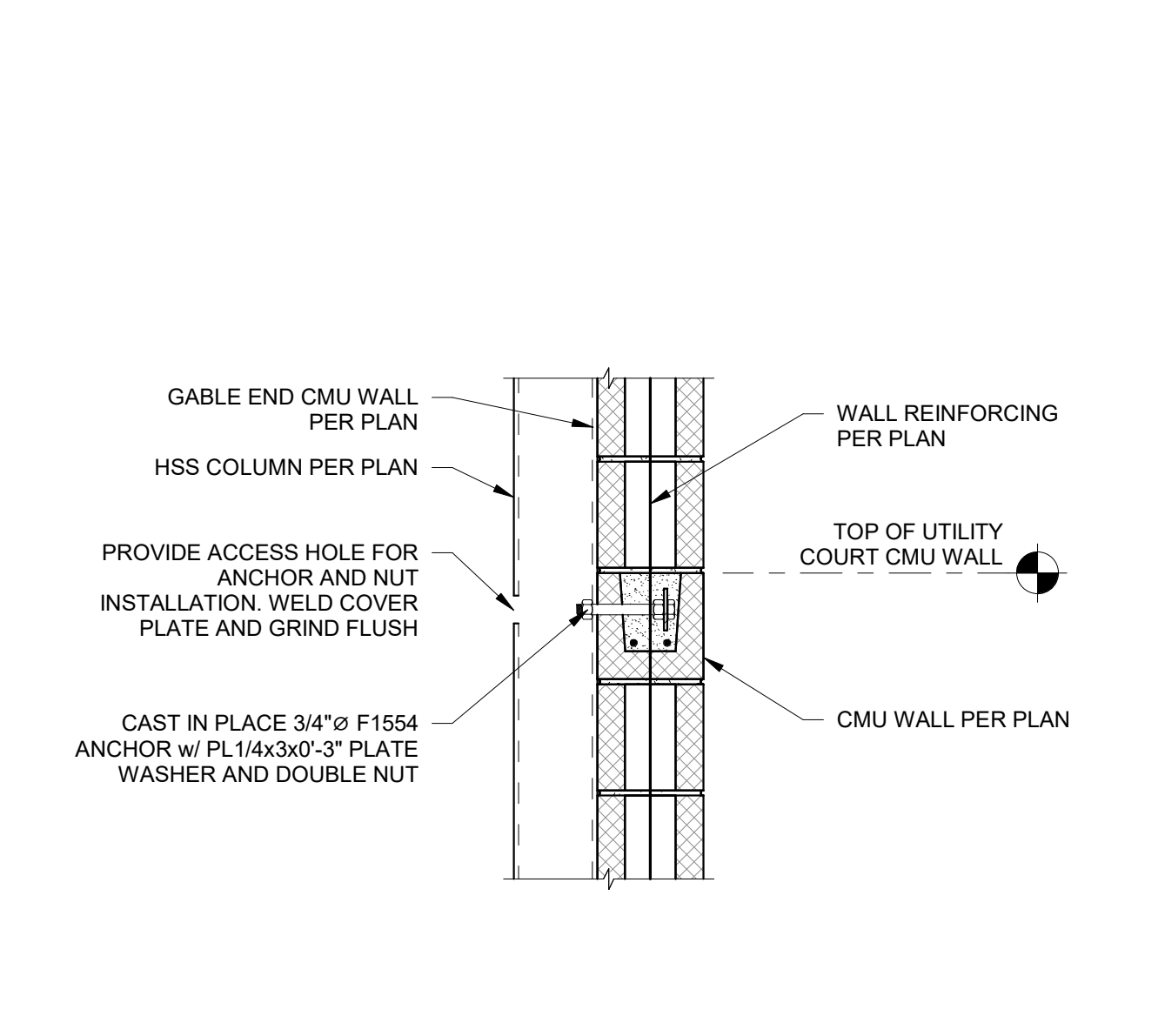
4  
S504  
1" = 1'-0"



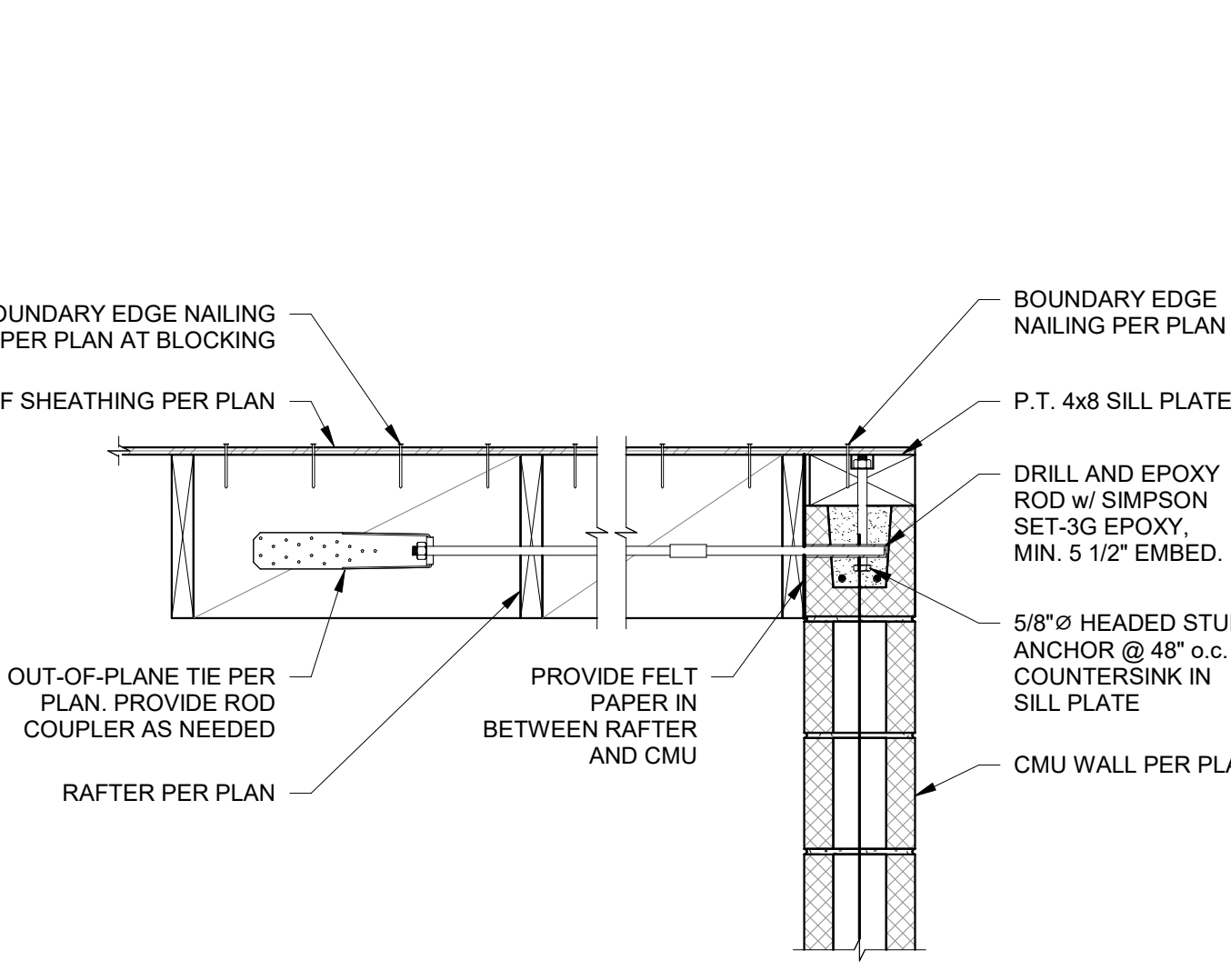
5  
S504  
1" = 1'-0"



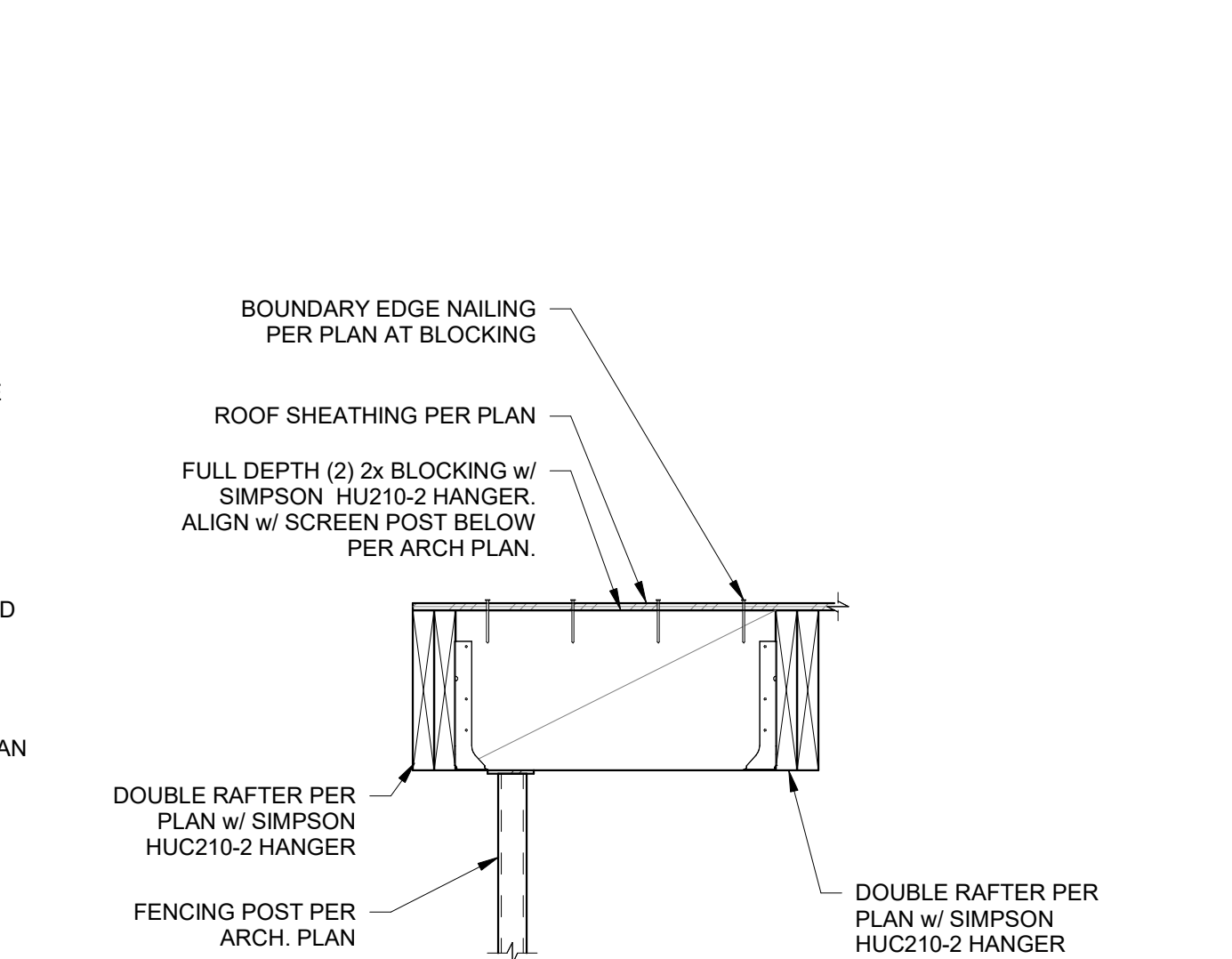
6  
S504  
1" = 1'-0"



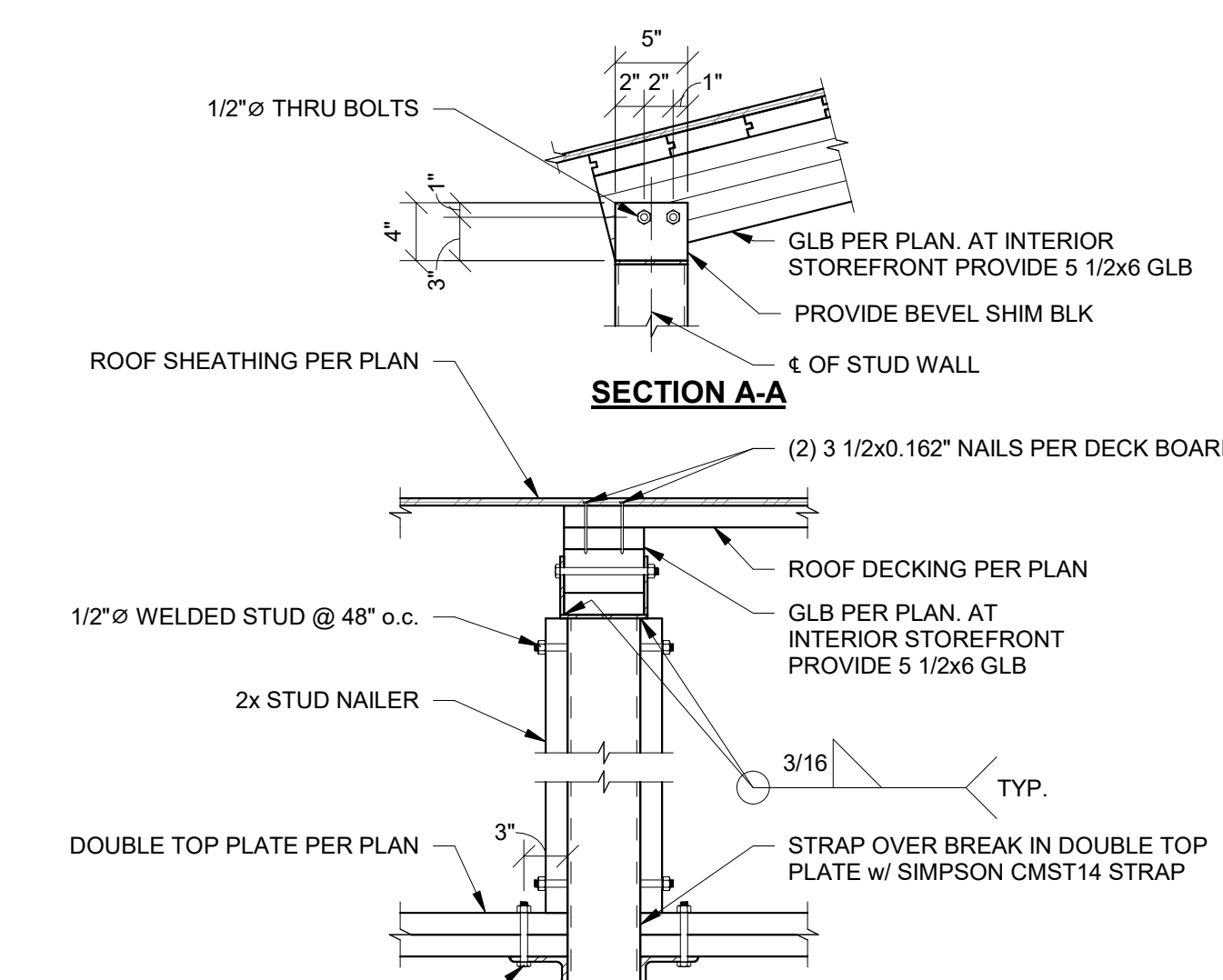
7  
S504  
1" = 1'-0"



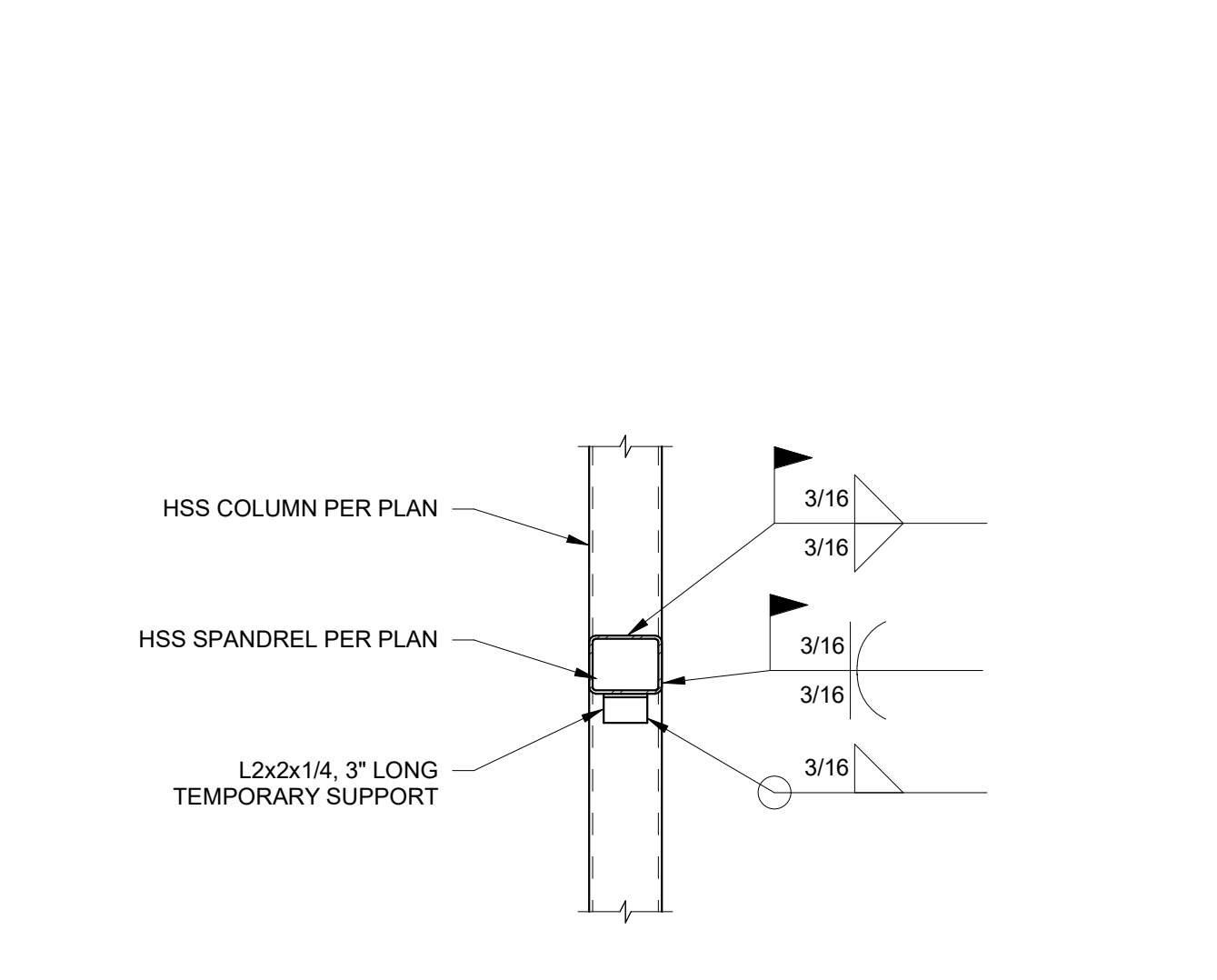
8  
S504  
1" = 1'-0"



9  
S504  
1" = 1'-0"

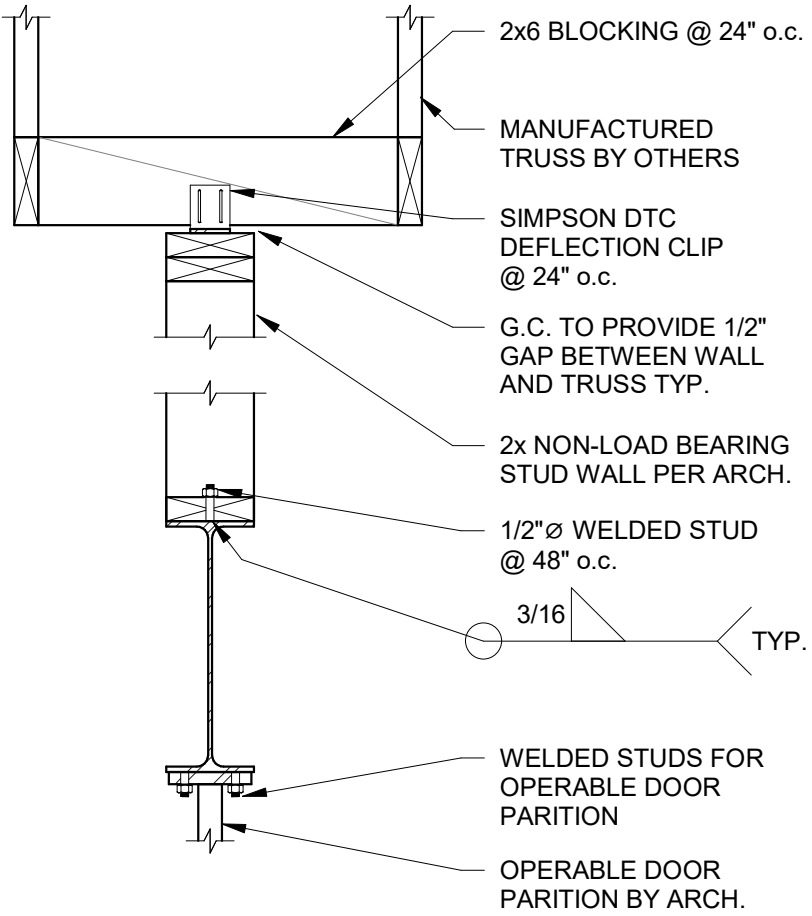


10  
S504  
1" = 1'-0"

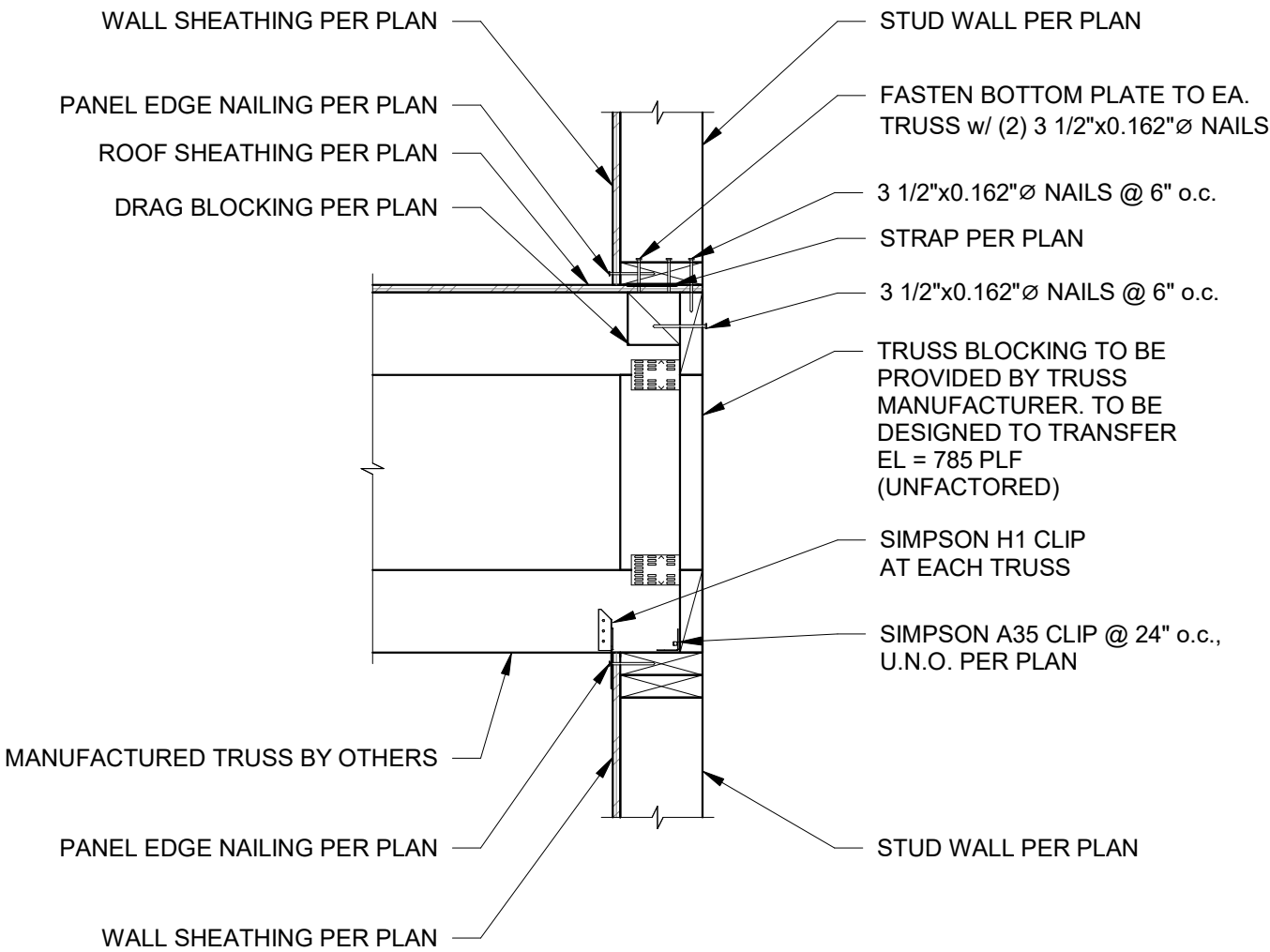


11  
S504  
1" = 1'-0"

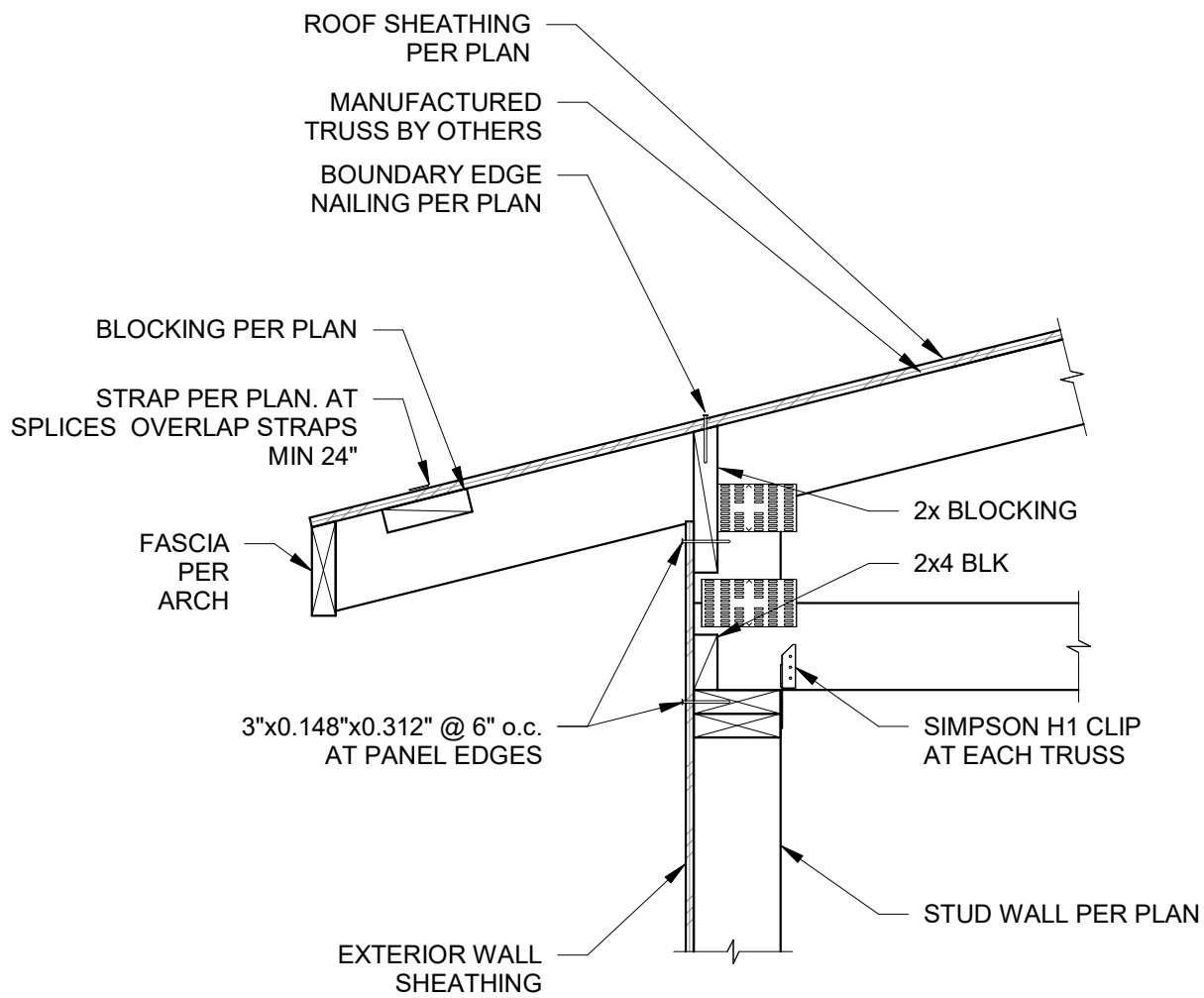




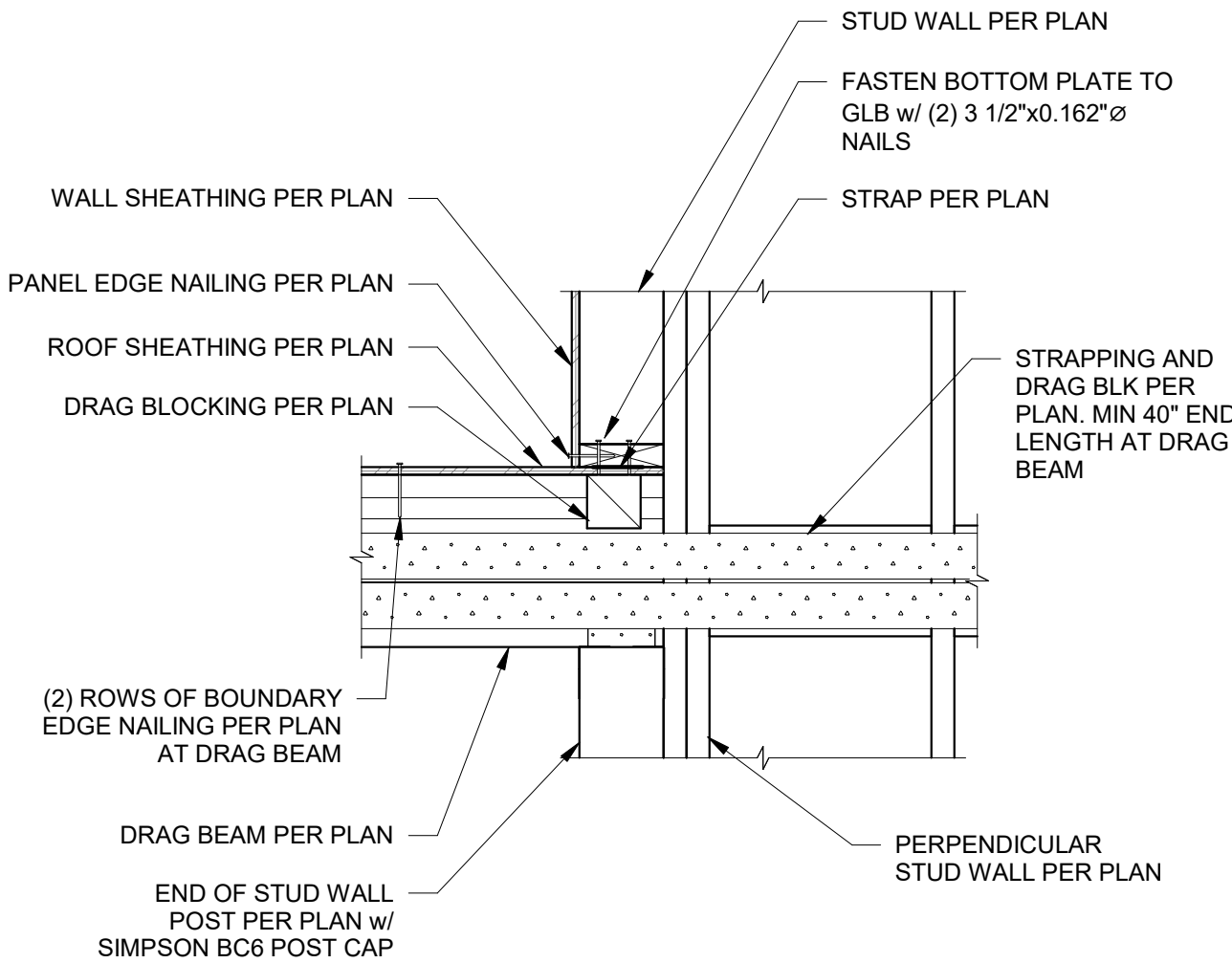
2 FOLDING PARTITION WALL CONN.  
S505 1" = 1'-0"



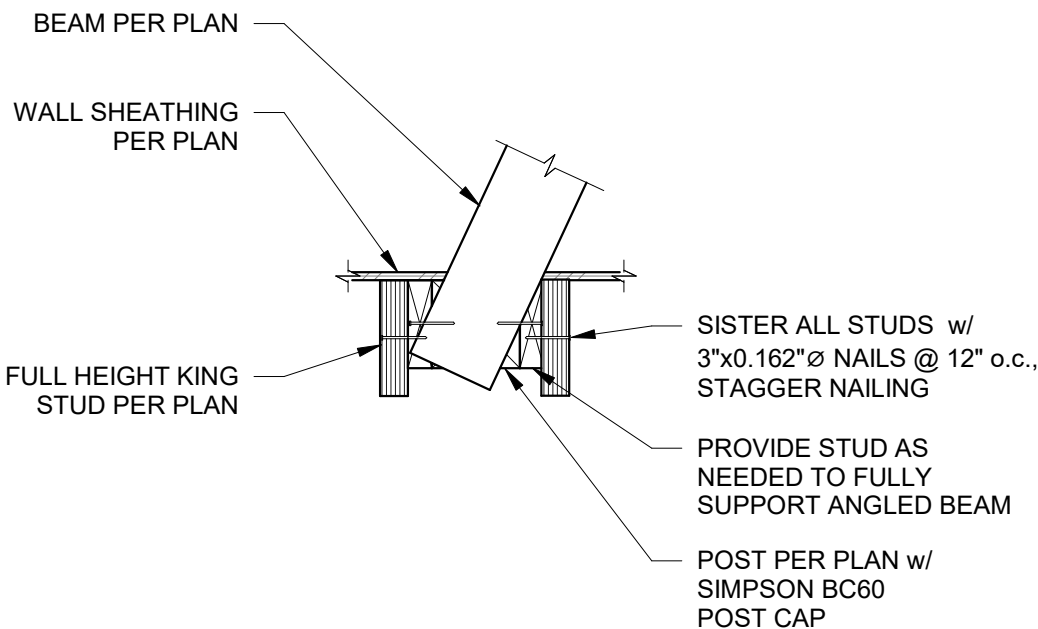
3 LOW ROOF TRUSS TO WALL CONNECTION  
S505 1" = 1'-0"



4 HEEL TRUSS AT EXTERIOR WALL  
S505 1" = 1'-0"



5 LOW ROOF GLB TO WALL CONNECTION  
S505 1" = 1'-0"



6 BEAM POCKET  
S505 1" = 1'-0"